

AD/A-007 244

HEAVY LIFT HELICOPTER - CARGO HANDLING  
ATC PROGRAM. VOLUME II. FABRICATION  
OF TEST HARDWARE AND FIXTURES (INTE-  
GRATED TEST RIG)

Joseph Shefrin, et al

Boeing Vertol Company

Prepared for:

Army Air Mobility Research and Development  
Laboratory

December 1974

DISTRIBUTED BY:

**NTIS**

National Technical Information Service  
U. S. DEPARTMENT OF COMMERCE

UNCLASSIFIED

SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

REPORT DOCUMENTATION PAGE		READ INSTRUCTIONS BEFORE COMPLETING FORM
1. REPORT NUMBER USAAMRDL-TR-74-97B	2. GOVT ACCESSION NO.	3. RECIPIENT'S CATALOG NUMBER <b>AD/A-007244</b>
4. TITLE (and Subtitle) HEAVY LIFT HELICOPTER - CARGO HANDLING ATC PROGRAM, VOLUME II FABRICATION OF TEST HARDWARE AND FIXTURES (INTEGRATED TEST RIG)		5. TYPE OF REPORT & PERIOD COVERED Final Report June 1971 - June 1974
7. AUTHOR(s) Joseph Shefrin Wendell F. Hill (Wichita Division)		6. PERFORMING ORG. REPORT NUMBER
9. PERFORMING ORGANIZATION NAME AND ADDRESS Boeing Vertol Company (A Division of the Boeing Company) Philadelphia, Pa. 19142		8. CONTRACT OR GRANT NUMBER(s) Contract DAA 701-71-C-0840 (P6A)
11. CONTROLLING OFFICE NAME AND ADDRESS U. S. Army Aviation Systems Command P. O. Box 209, St. Louis, Missouri 63166 Attn: AMSAV-PDAC		10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS Para. F.4d. (3) (a)
14. MONITORING AGENCY NAME & ADDRESS (if different from Controlling Office) Eustis Directorate U.S. Army Air Mobility R&D Laboratory Fort Eustis, Va. 23604		12. REPORT DATE December 1974
		13. NUMBER OF PAGES 24
		15. SECURITY CLASS. (of this report) Unclassified
		15a. DECLASSIFICATION/DOWNGRADING SCHEDULE
16. DISTRIBUTION STATEMENT (of this Report)  Approved for public release; distribution unlimited.		
17. DISTRIBUTION STATEMENT (of the abstract entered in Block 20, if different from Report)		
18. SUPPLEMENTARY NOTES  NATIONAL TECHNICAL INFORMATION SERVICE		
19. KEY WORDS (Continue on reverse side if necessary and identify by block number) Helicopters      Test Equipment:      Container Handling Heavy Lift Helicopters      Air Cargo Handling Cargo Handling      Hoist Systems Cargo System Test Rig      Pneumatic Systems Integrated Test Rig      Suspension Systems		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number) This report formally documents the efforts and results of the cargo handling system segment of the Heavy Lift Helicopter (HLH) Advanced Technology Component (ATC) development program.  The purpose of the HLH/ATC was to minimize technical, cost and schedule risks associated with future HLH system research.		

DD FORM 1 JAN 73 1473

EDITION OF 1 NOV 65 IS OBSOLETE

UNCLASSIFIED

SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

PRICES SUBJECT TO CHANGE

UNCLASSIFIED

SECURITY CLASSIFICATION OF THIS PAGE(When Data Entered)

20. Abstract (continued)

development, test and evaluation (RDTE) and production programs. This was achieved by design, fabrication, and testing of specific ATC hardware in three critical air vehicle subsystems:

- a. Rotor/Drive System
- b. Flight Control System
- c. Cargo Handling System

This report covers only the cargo handling system and consists of three volumes:

Volume I - Detail Design, Structural and Weights Analysis, and Static and Dynamic Load Analysis

Volume II - Fabrication of Test Hardware and Fixtures (Integrated Test Rig)

Volume III - Results of Tests, Inspections and Evaluations

Volume II contains the design criteria, physical description, stress analysis, fabrication and supporting data for the Integrated Test Rig which was used to conduct system testing of the full-scale cargo-handling ATC hardware.

UNCLASSIFIED

SECURITY CLASSIFICATION OF THIS PAGE(When Data Entered)

## TABLE OF CONTENTS

	<u>Page</u>
LIST OF ILLUSTRATIONS . . . . .	v
LIST OF TABLES. . . . .	ix
INTRODUCTION. . . . .	1
GENERAL DESCRIPTION OF INTEGRATED TEST RIG. . . . .	2
Major Elements of Test Fixture . . . . .	2
Full-Scale Static Simulation - HLH Installation. . . . .	2
Rig Location . . . . .	6
DESIGN REQUIREMENTS AND CRITERIA. . . . .	6
Hardware Compatibility . . . . .	6
Suspension and Hoist Span Arrangement. . . . .	6
Design Loads . . . . .	7
Failure Criteria . . . . .	7
Load Size and Access . . . . .	9
Load Lift. . . . .	9
Hoisting Speed . . . . .	9
Utilities and Communication. . . . .	9
Control Room . . . . .	10
Safety Considerations. . . . .	10
Transportability Considerations. . . . .	10
Pneumatic Power Generator. . . . .	10
Data Requirements. . . . .	10
<u>DETAIL DESIGN</u> . . . . .	10
Design Considerations. . . . .	10
Configuration Description. . . . .	12
Material Design Criteria . . . . .	13
Trade-Offs/Cost Reduction. . . . .	13
Member Sizing. . . . .	13
Tower Structures . . . . .	14
Overhead Structure . . . . .	14
Hoist Assembly Modules . . . . .	15
Control-Room Operator's Station. . . . .	15
Site Location and Foundation . . . . .	17
Stress Analysis. . . . .	17
Utilities and Communication. . . . .	17
Pneumatic Power Generator. . . . .	19
Instrumentation. . . . .	22
Design Layouts . . . . .	36

	<u>Page</u>
FABRICATION . . . . .	38
Structure Prefabrication . . . . .	38
Foundation . . . . .	38
Hoist Module Installation. . . . .	56
PPG and Air Supply Installation. . . . .	56
Instrumentation Installations. . . . .	56
Proof Loading. . . . .	70
SPECIMEN INSTALLATION AND REMOVAL . . . . .	70
USE OF THE INTEGRATED TEST RIG. . . . .	70
CONCLUSIONS AND RECOMMENDATIONS . . . . .	72
APPENDIXES:	
I Integrated Test Rig - Stress Analysis . . . . .	79
II Drawings - Design Layouts . . . . .	135
III Pneumatic Power Generator - Starting and Operating Procedure . . . . .	187
IV Instrumentation Calibration Procedure . . . . .	190

## LIST OF ILLUSTRATIONS

<u>Figure</u>		<u>Page</u>
1	HLH Cargo Handling System Integrated Test Rig (ITR) Hoisting - 29-Ton Load . . . . .	3
2	Single-Point Mode-Hoist Installation in HLH . . . . .	4
3	Dual-Point Mode -Hoist Installation in HLH . . . . .	5
4	Multi-Point and Single-Point Static Loading Schematic. . . . .	8
5	Test Rig, Showing Load-Controlling Crewman's View of Cargo. . . . .	16
6	Schematic - HLH/ATC Pneumatic Power Generator (PPG) and Generator Location . . . . .	20
7	PPG Gearing Arrangement. . . . .	21
8	Instrumentation Rack Console ST30855-1 . . . . .	27
9	Cable Angle Sensor Mechanism . . . . .	30
10	Cable Payout Sensor. . . . .	31
11	Galvo Input Networks . . . . .	34
12	Maximum Load Test Network. . . . .	37
13	Excavation for ITR Foundation. . . . .	39
14	Preparation of Footings and Foundation . . . . .	40
15	Three Ground Tiedown Points in Foundation. . . . .	41
16	Rig Components and Raising of Lower Main Frames . . . . .	42
17	Complete Tower Frames Erection . . . . .	43
18	Overhead and Stairway Installation . . . . .	44
19	End View of Aft Tower. . . . .	45
20	End View of Forward Tower. . . . .	46

<u>Figure</u>		<u>Page</u>
21	Under View of Overhead Structure and Hoist Module Location . . . . .	47
22	Hoist Modules (Inverted). . . . .	48
23	Control Room Area - Top of Forward Tower. . .	49
24	View of Overhead Section From LCC Position; Forward Tower . . . . .	50
25	Work Platform and Davit Looking Forward, Control Room Location . . . . .	51
26	Completed ITR Framework on Foundation . . . .	52
27	Control Room Enclosure Assembly . . . . .	53
28	Aerial View of Complete Structure and Hot Air Distribution Duct . . . . .	54
29	Hoist System Installed in ITR . . . . .	57
30	PPG Pad at Aft Tower. . . . .	58
31	Forward Hoist - Hoist and Signal Reel Air Supply. . . . .	59
32	Insulated Air Header and Riser. . . . .	60
33	Complete PPG Enclosure and Air Distribution Line. . . . .	61
34	Side View of PPG. . . . .	62
35	Three-Quarter View of Load Compressor and Engine Inlet. . . . .	63
36	Hoist Controls and Displays at LCC Station. .	64
37	Instrumentation, PPG and LCC Station. . . . .	65
38	PPG Control Console . . . . .	66
39	Aft Hoist Drive and Instrumentation . . . . .	67
40	Instrumentation Attachment for Weighing Ballasted MILVAN. . . . .	68
41	Static Load Test Instrumentation. . . . .	69

<u>Figure</u>		<u>Page</u>
42	Hoist System Test Installation . . . . .	71
43	Control Room During Test . . . . .	73
44	LCC View - Hoist and Couplings . . . . .	74
45	LCC View of Two Individual Loads Near Top of Lift Cycle . . . . .	75
46	29-Ton Multi-Point Test. MILVAN With Con- tainer Handling Device During 50-Foot Lift . .	76
47	Lifting a 29-Ton Load With Single-Point Adapter - Hoist Span Was 16 Feet . . . . .	77
48	Unloading MILVAN Through Base of Aft Tower . .	78
49	Structures Installation- HLH Hoist Test. . . .	137
50	HLH Hoist Tower Assembly . . . . .	139
51	HLH Overhead Assembly . . . . .	145
52	Site Plan - Paving and Utilities, Plan and Sections . . . . .	149
53	Foundations - Plans, Sections and Details. . .	151
54	Pneumatic Power Generator Shelter HLH/ATC Cargo Handling System . . . . .	153
55	Control Room Plans, Elevations and Section . .	155
56	Control Room Section and Details . . . . .	157
57	Removable Handrail Details . . . . .	159
58	Piping Arrangement, Test Tower Top Section . .	161
59	Piping Arrangement, Test Tower Base . . . . .	163
60	Fuel Piping Arrangement - PPG Unit . . . . .	165
61	Electrical Single Line Diagram . . . . .	167
62	Electrical HLH/ATC Cargo Handling Test Rig . .	169
63	Control Room Electrical Layout . . . . .	171



<u>Figure</u>		<u>Page</u>
64	Load-Controlling Crewman Platform - Integrated Test Rig . . . . .	173
65	System Test - Drawing Tree . . . . .	175
66	Lifting Sling, Hoist/Module . . . . .	177
67	Hoist Lifting Fixture . . . . .	179
68	Integrated Test Rig - System Wiring . . . . .	183
69	Instrumentation Drawing Tree . . . . .	185

## LIST OF TABLES

<u>Table</u>		<u>Page</u>
I	ITR - Test Data and Instrumentation Requirements . . . . .	11
II	Component Listing, Instrumentation System - HLH Cargo Handling System . . . . .	23
III	ITR Instrumentation System - Recorded Data Format. . . . .	28
IV	ATM Shaft Speed Related to Cable Payout Speed by System Gearing . . . . .	33
V	Integrated Test Rig Weight Breakdown. . . . .	55
VI	Tower Assembly - Fore-Aft Frame Loads (KIPS). . . . .	107
VII	Tower Assembly - Inboard/Outboard Frame . . . . .	116
VIII	Reaction Loads - 70-Ton Failure Condition . . . . .	130
IX	Reaction Loads - 70-Ton Failure Condition . . . . .	130
X	Reaction Loads - Impact of Load Container on Tower . . . . .	131
XI	Deadweights . . . . .	132
XII	Normal PPG Operating Limits . . . . .	189

## INTRODUCTION

Final design ATC hardware was based on subsystem design development testing. Verification of the cargo handling system ATC design goals, accomplished in advance of an aircraft installation, required a test fixture with which system performance could be demonstrated.

Since no suitable evaluation fixture existed, the ATC program included preparation of such a fixture. This fixture, the integrated test rig (ITR), was designed to permit: operation of the ATC developed hardware through all its functions; verification of performance and reliability characteristics, failure modes and effects, and maintenance characteristics; demonstration of 1800 hoisting cycles using both suspension modes at design load and speed and demonstration of maximum static load.

Although the ITR was erected on Boeing Vertol property, its design provided for dismantlement and reerection at another site. To support use at another location, appendixes to this document include foundation loads, descriptive drawings, air supply operating instructions, and an instrumentation calibration procedure.

## GENERAL DESCRIPTION OF INTEGRATED TEST RIG (ITR)

### MAJOR ELEMENTS OF TEST FIXTURE

The integrated test rig is shown in Figure 1. It consists of two steel I-beam towers, 14x14 feet square, supporting a pair of horizontal internally braced 30-inch-deep wide-flange I-beams with a 40-foot span and a 70-foot vertical clearance. Lateral outriggers buttress the towers.

The major elements of the ITR are:

1. Main structure consisting of towers and overhead section.
2. Footings and foundation with provisions for three ground tiedown points ("dead men").
3. Pneumatic power generation (hoist drive air supply) including fuel supply.
4. Utility power and communication.
5. Control room and enclosure.
6. Instrumentation and data recording system.
7. Stairway to control room and work platform.
8. Provisions for utility hoist.
9. Site improvements including access roads.

### FULL-SCALE STATIC SIMULATION - HLH INSTALLATION

The test rig simulates a full-scale installation of the cargo handling system (CHS) in the Heavy Lift Helicopter (HLH), Figures 2 and 3, in the following respects:

1. Two steel frames simulate the a/c mounting structure for the hoists and signal conductor reels. These are removable to permit complete ground buildup of individual hoists and associated assemblies including the wiring to form a "hoist module". The assembled "modules" are raised with the overhead utility hoist and installed between the main overhead and auxiliary horizontal I-beams. Each hoist module also mounts the span positioning equipment and has space provisions for hoist separations of 16, 22 and 26 feet.

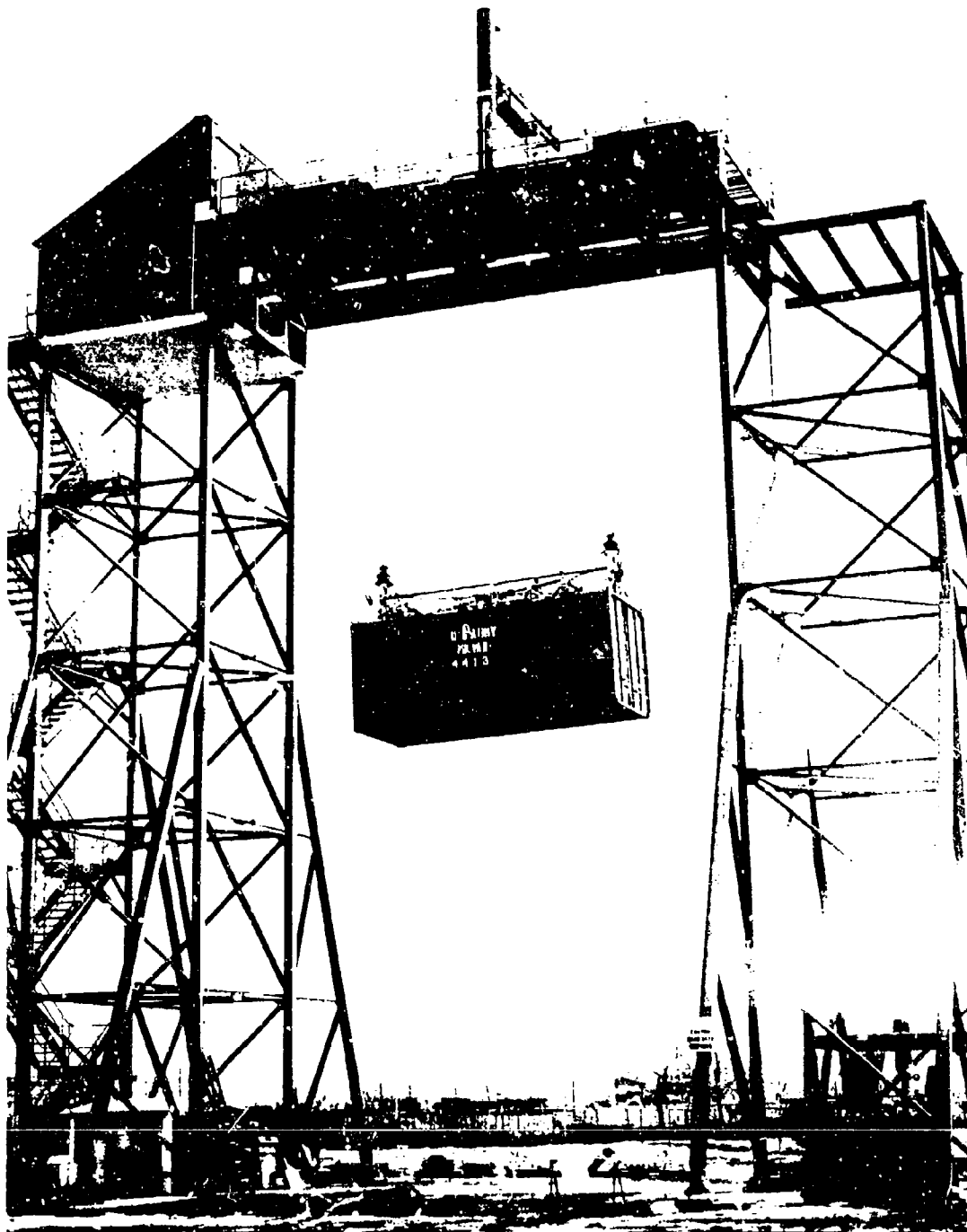


Figure 1. HLI Cargo Handling System Integrated Test Rig  
Hoisting - 29-Ton Load.

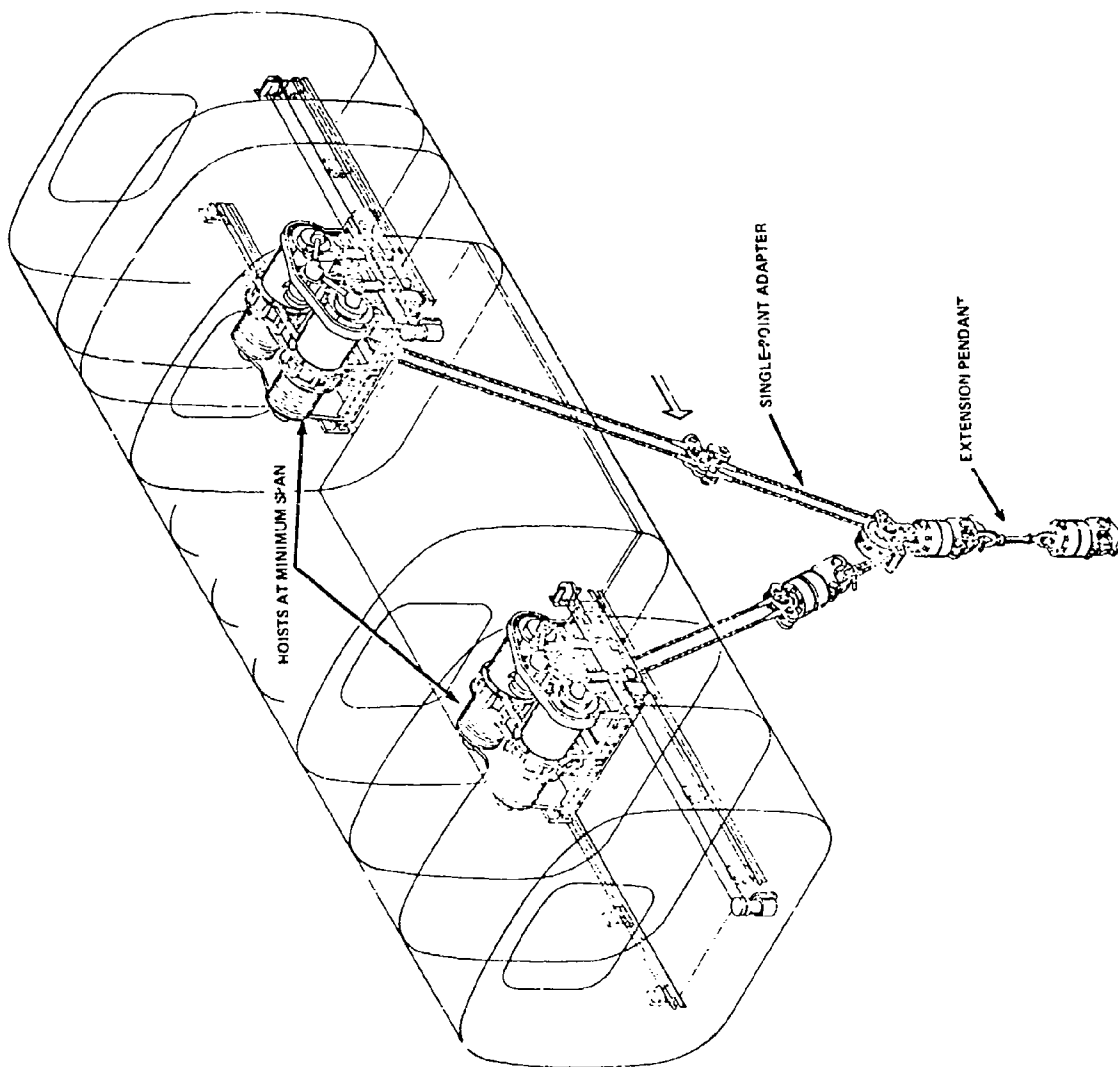


Figure 2. Single-Point Mode - Hoist Installation in HLH.

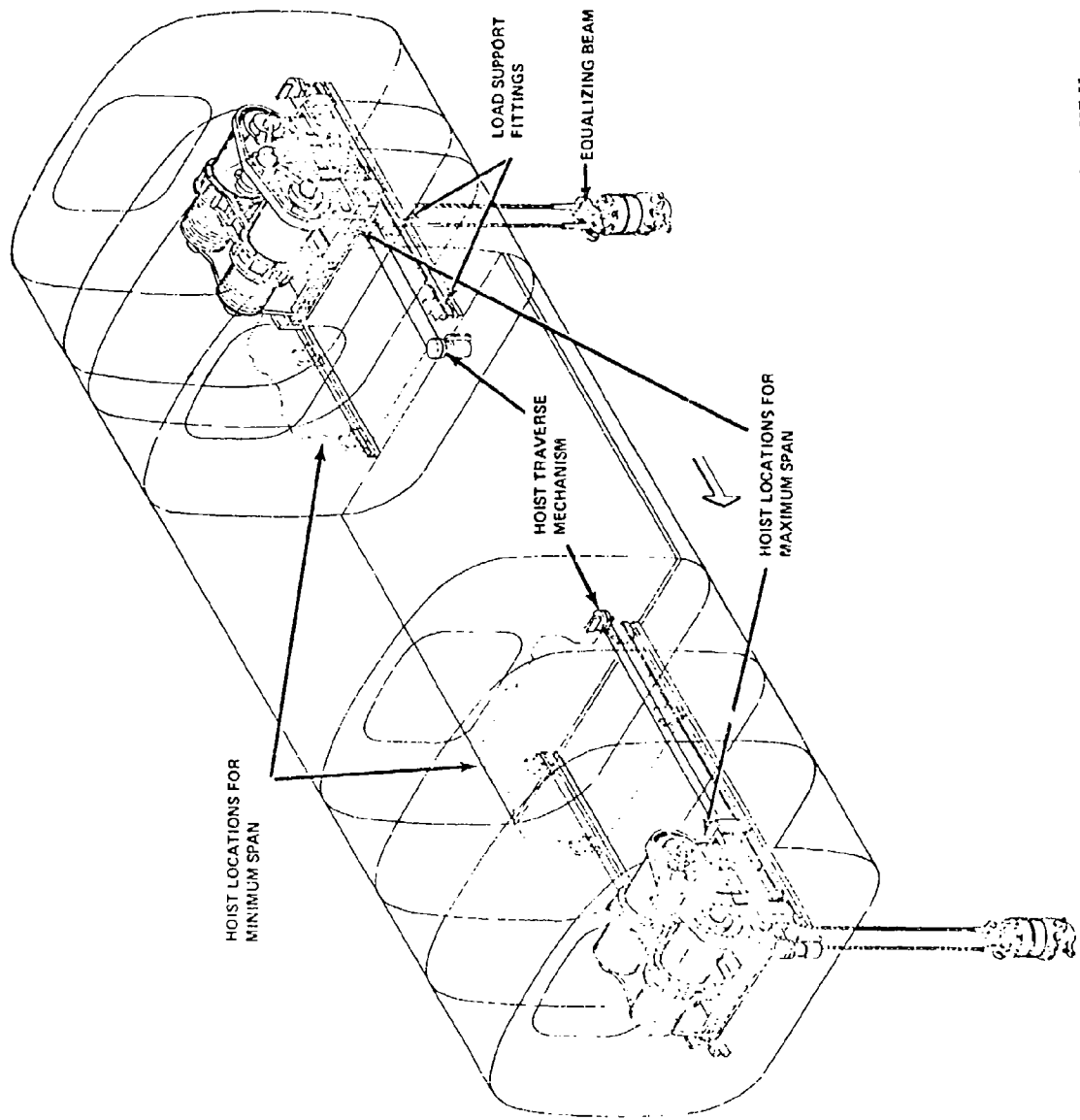


Figure 3. Dual-Point Mode - Hoist Installation in HLH.

2. Power, control, geometry and physical spacing of hoist assemblies.
3. Fuselage level attitude.
4. Relative location, type of hoist controls and displays, and layout of the rear-facing, load-controlling-crewman (LCC) station.
5. A 50-foot cable deployment with full-scale, rigged loads.

#### RIG LOCATION

The ITR is located at the Boeing Vertol Company facility, Ridley Park, Pennsylvania.

#### DESIGN REQUIREMENTS AND CRITERIA

##### HARDWARE COMPATIBILITY

The test rig design shall be compatible with the following full-scale cargo handling system hardware to be installed and operated in the test rig:

- Hoist and tension member assembly
- Hoist drive unit
- Load isolators
- Span positioning system
- Signal conductor reels
- Controls and displays systems
- Coupling
- Single-point adapters
- Cable cutters
- Pneumatic distribution system

and the following GFE (non-CHS) auxiliary equipment:

- 8' x 8' x 20' MILVAN
- Container handling device

##### SUSPENSION AND HOIST SPAN ARRANGEMENT

Both single-point and two-point suspension arrangements shall be provided. Hoist span positioning space and functional provisions shall be incorporated for both 16- and 26-foot suspension spans.



## DESIGN LOADS

### Steady Vertical Loads

The individual hoists must support vertical loads and accommodate a 60/40 load split based on the two-point suspension. The loads to be supported are:

System design operating load	= 28.0 tons
Hoist design operating load (28x0.6)	= 16.8 tons
System maximum static load (28x2.5g)	= 70.0 tons
Hoist maximum static load (28x2.5gx0.6)	= 42.0 tons
Failure case (single hoist, any span position)	= 70.0 tons

### Transient Vertical Loads

Some transient vertical loads may be encountered under differential hoisting (load levelling). The normal load tilt (multipoint mode) will not exceed  $\pm 15^\circ$ . Load release transients are insignificant (loads above 1,000 lb cannot be released).

### Side, Overturning, and End Loads

The hoists are "single-point payout"; therefore, tension member loads shall always act through the longitudinal (fore-aft) centerline of the test rig. No side, end, or overturning loads are anticipated due to testing except as may be encountered from load swinging ( $\pm 5^\circ$ , est.) or wind. Load motion shall not result in the loads striking the rig structure.

### Maximum Static Load Provisions

Tiedown provisions in the ITR foundation are required for application of the maximum design static load to either individual or both hoists as shown in Figure 4.

The loading member, ring or bar, with a maximum 2-1/2 inch diameter, is to be compatible with CHS coupling. Space must be provided for coupling attachment/release.

## FAILURE CRITERIA

Failure simulation involving severe load transients or high load release tests was not a requirement of the ATC program.



Rig survival - no collapse - was provided in the design in the event of a sling or other failure under maximum static load. The vertical columns are the main support for the control room and are independent of the column struts in case of strut removal by impact of the test load (due to a suspension failure).

#### LOAD SIZE AND ACCESS

The standard test load size is an 8' x 8' x 20' container. A 40-foot clear span between overhead supports is required to provide vehicular clearance around the load for load placement and removal. Direct access is required along the fore-aft centerline and between the supports and superstructure of one tower to accommodate a MILVAN on a flatbed trailer.

#### LOAD LIFT

The test rig is to have a clear height of 70 feet.

#### HOISTING SPEED

Design hoisting and lowering speed (with load) is 60 fpm. Design payout speed (without load) is 120 fpm.

#### UTILITIES AND COMMUNICATION

The following provisions shall be made for utilities:

1. Equipment hoist - 6,000 lb capacity
2. Intercom - LCC station to ground and test areas
3. Storage area
4. Cooling water for PPG heat exchanger
5. Work platforms around hoists
6. Lightning protection
7. Aircraft warning signals
8. Weather protection for cargo system and PPG
9. Electrical requirements for the cargo system, pneumatic power generator, related instrumentation, general utility and equipment hoist, and the cargo handling device shall be as follows:

28V DC	Direct current
5V AC	Single-phase, 400 Hz
26V AC	Single-phase, 400 Hz
115V AC	Single-phase, 60 Hz, regulated
120/208V AC	Three-phase, 60 Hz
120/280V AC	Three-phase, 400 Hz

Compliance with MIL-STD-704 is required.

## CONTROL ROOM

The control room shall house the LCC station, with a full view of the cargo in the ground and hoisted positions, the controls for the PPG and all related instrumentation. The floor loading criteria of 125-150 psf (light industrial) shall be used.

## SAFETY CONSIDERATIONS

In addition to ITR failure criteria, the structure and location of operating equipment and utilities shall meet OSHA\* requirements. The personnel stairway shall be remote from the test load.

\*Williams - Stieger Industrial Safety, Oct. 1970.

## TRANSPORTABILITY CONSIDERATIONS

The rig design shall incorporate provisions for dismantlement and reerection at another site.

## PNEUMATIC POWER GENERATOR

A pneumatic power generator (PPG), located at ground level, and air distribution system shall be used to supply air to the hoists at a pressure ratio of 4.3 and a temperature of 450°F. Operating controls, pressure regulation and safety devices are required.

## DATA REQUIREMENTS

Recordings are to be made of the following parameters (as delineated in Table I):

- Ambient conditions - temperature and pressure
- Supply air - temperature and pressure
- Air turbine operation - temperature, pressure and speed
- Control inputs and functions
- Tension member - load, angle, payout length
- Cargo system noise

## DETAIL DESIGN

### DESIGN CONSIDERATIONS

Selection of the ITR configuration was based on the following design considerations:

1. Control room requirement
  - Access to hoist test location
  - Simulation of HLH LCC station
  - Drinking water, but no lavatory or toilet facilities

TABLE I. ITR - TEST DATA AND INSTRUMENTATION REQUIREMENTS.						
Data Requested	Unit	Max. Range	Reqd. Accuracy	No. of Transducers	Visual Monitor Scope	W.R. FM.
<u>PNEUMATIC POWER GENERATOR</u>						
Supply Air Temp.	°F	0-500	±1%	1	X	
Supply Air Pressure	PSIA	0-60	±1%	1	X	
<u>HOIST DRIVE</u>						
Turbine Inlet Pressure	PSIG	0-50	±1%	4	X	
Turbine Inlet Temp.	°F	32-500	±1%	4	X	
Turbine Exhaust Temp.	°F	32-500	±1%	2	X	
Turbine Motor RPM	+VDC	5	±2%	1	X	
<u>HOIST CABLE DATA</u>						
Cable Tension	lb	0-30000	±1%	4	X	
Cable Angle-Longitudinal	DEG	30° Fwd/40° Aft	±1%	2	X	
Cable Angle-Lateral	DEG	30	±1%	2	X	
Cable Payout	FT	100	±2%	2	X	
Hoist RPM (Load Velocity)	RPM	25	±2%	2	X	
<u>CABLE LENGTH INDICATOR</u>						
Hoist Position (Fwd)	FWD/MID/AFT	N/A		0	X	
Hoist Position (Aft)	FWD/MID/AFT	N/A		0	X	
<u>HOIST CONTROL</u>						
Command Velocity	+VDC	5	±2%	2	X	
<u>CARGO COUPLING</u>						
Mech. Release Signal	+VDC	28	±2%	2	X	
<u>NEAR-&amp; FAR-FIELD NOISE</u>						
	db	140		1		X
<u>WIND VELOCITY</u>						
	MPH	60		1	X	
<u>AMBIENT AIR TEMPERATURE</u>						
	°C	-10+40		1	X	

2. Span/overhead structure  
Vehicle clearance for manipulating load  
Variable cargo hoist span  
Center mount for davit
3. Safety  
OSHA standards  
Stairway remote from test load - single egress location
4. Outriggers  
Wind loading requirement  
Access to test load through base of tower
5. Transportability  
Possible use of ITR at another site required  
inclusion of splices and bolted joints at significant locations
6. Footings and foundation  
Detail design based on known local rock formations determined by site borings.

#### CONFIGURATION DESCRIPTION

Because of height requirements of the towers and the relative low design loads, a trussed frame configuration was selected. Requirements for a control room made the selections of a 14-foot-square tower desirable. Easy installation of stairways on this type of tower was also a factor in the selection. Outriggers were used on each leg of the tower in the inboard-outboard plane for two reasons:

1. Wind loading, and
2. Need to remove secondary **bracing** in lower portions of tower to permit a loaded truck to drive through the tower.

Outriggers in the fore-aft plane were not required because the criteria used were not factors in this plane. Also, horizontal components of test loads are reacted by four frames in the fore-aft plane, while in the inboard-outboard direction only the two inner frames will react the horizontal component of loading.

Highlights of the ITR design including layouts of the test fixture and equipment are discussed in the following text.

## MATERIAL DESIGN CRITERIA

### Structure

Structural steel - Commercial A36 (AISC Specification)

Allowable 22,000 psi

Yield 36,000 psi

Ultimate 60,000 psi

Welding practice - AISC

Bolts - ASTM A325

Design loads - per requirement section.

Environmental factors - Wind - 30 psf (loaded)

50 psf (unloaded)

Snow - NE local standard

Safety factor - 1.5 (on maximum/failure load using material allowable.)

### Footings and Foundation

Concrete - 3,000 psi compressive strength at  
28 days - reinforced, continuous pour

Reinforcing steel - per ASTM A61

Anchor bolts - ASTM Spec A325

Footings - 3,000 psi concrete on bedrock

Anchor beams - to mate with Crosby-Laughlin round  
pin shackles

## TRADE-OFFS/COST REDUCTION

An alternative A-frame configuration for the structure was considered. It was rejected in favor of the selected design since higher column stiffness requirements would have resulted in a heavier design. Also, bolted and riveted construction was eliminated as more costly than the welded design used. Use of a 60-foot span to accommodate a 40-foot load length was also considered. This option was not implemented due to an estimated cost increase of approximately 20%.

## MEMBER SIZING

	<u>Size</u>	<u>Type</u>	<u>Lb/Ft</u>
Main beams	30x14	WF	172
Main vertical supports (towers)	8x8	WF	40
Tower bracing	(2) 4x3-1/2	Angles	9.1

## TOWER STRUCTURES

### Transportability

To facilitate disassembly and reerection, each of the tower columns included splices which consisted of a combination of welding and bolting at the joints, bolted connections at each of the four diagonal struts in the adjacent bay, and bolted joints at the outrigger struts.

Using this construction configuration, each tower could be dismantled into two discrete sections and the supporting outriggers could be disassembled for shipping without the necessity for welding cuts.

### Stairways

Stairways were enclosed by 42-inch guardrails on both sides with a 32-inch handrail being incorporated on the inside guardrail. The handrail was located 3 inches into the 30-inch stairway.

The stairway was located entirely outside the outboard tower columns for safety considerations. This removed it as far as possible from the test load in the event of any failure in the suspension system.

### Utility Connection to Tower Column

An outboard column was used as the terminating point for the utility connection from an adjacent building. The connection was made between the 28-foot and 42-foot levels with an attachment force of approximately 1,000 lb.

### Warning Beacons

Beacon type lights normally required atop the structure for the protection of low-flying aircraft were not required for the site selected because of higher structures in the adjacent area.

## OVERHEAD STRUCTURE

### Overhead Beam Location Holes

The tower/beam assembly was predrilled to simplify assembly. The tower/beam holes are close fitting. The hoist module attachment was also predrilled.

Slots were not provided for temperature changes as estimated changes were small.



Camber was not specified in the overhead beams, since only 0.7 inch beam deflection was anticipated at the 70-ton static load.

#### Work Platform

Grip strut, an expanded-metal, nonslip surface, was used for the open work platforms around the hoists. A uniform floor loading of 235 psf was used.

#### Davit Assembly

The davit assembly or jib crane reach was 12 feet. It had a 6,000-lb hoisting capability. This satisfied the need for lifting the hoist modules and other equipment from the ground, over the side, or through the hoist location to the overhead structure.

The hoist cable reach was 89 feet. Maximum hoisting rate was 40 fpm. Guardrails surrounding the hoist module locations had sections which were removable for hoist installation and removal by the over-the-side method.

#### HOIST ASSEMBLY MODULES

The hoist module consisted of a simple welded framework and lift points including the hoist main support fittings and hoist positioning tracks. The function of the module was to facilitate preinstallation and checkout of each hoist assembly on the ground (similar to engine quick prep-packages). The module weight was minimized to keep the davit and utility hoist size down. Web material was 1-inch and flanges were .75-inch plate.

Each hoist was removable from its module. Both modules were identical and provided a hoist span positioning travel of 60 inches.

#### Hoist Shelters

A portable shelter (tarpaulin) was provided for each hoist test location. Each section was removable during hoist testing.

#### CONTROL-ROOM OPERATOR'S STATION

The control-room operator's station eye level duplicated the HLH LCC position as shown on Figure 5.

The control room floor area was approximately 130 sq ft. A solid, precast-concrete, nonslip floor was used with a design

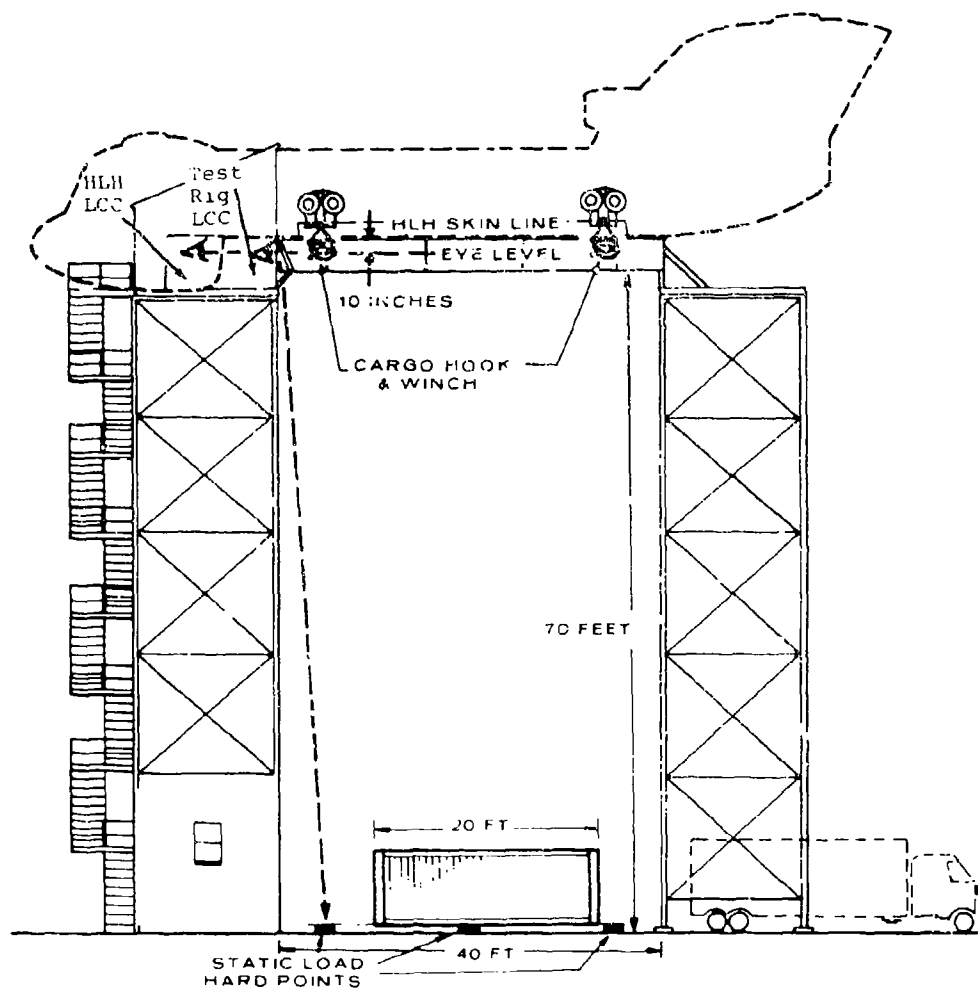


Figure 5. Test Rig, Showing Load-Controlling Crewman's View of Cargo.

floor loading of 172 psf.

LCC load viewing windows, looking aft and downward, were constructed of high-impact-strength polycarbonate (Lexan). A removable protective bar was installed across the main window. Side observation windows were of thermopane.

Except for doors and windows, the enclosure was fully insulated. Heat was furnished by a 3-kw thermostatically controlled space unit. A 5,000-Btu window-mounting airconditioner was provided for summer operation.

#### SITE LOCATION AND FOUNDATION

The test rig site location was selected to make maximum use of local rock foundations. To minimize service roads, the test rig was oriented adjacent to existing roadways.

The service road included a "Y" with one branch through the base of one tower and the other parallel to the overhead structure for truck pickup or deposit of a load within the reach of the jib crane.

The service road also provided for servicing of the fuel tank located 100 feet from the PPG unit.

Underground static load points in the concrete surface between the towers, along the hoist centerline, provided a 70-ton loading capability at the center point and 42 tons at each of the multi-point loading points. Slots were provided for entry of the cargo hooks, and covers were used to allow the surface traffic over these points.

The working surface around the test rig was asphalt covered, for vehicular traffic and surface drainage.

#### STRESS ANALYSIS

A stress analysis was performed to substantiate the design strength of the towers and overhead structure, using a Boeing Watfor "plain frame" computer program. An analysis of the hoist module frames and foundation reaction loads was also made. Analysis details are provided as Appendix I.

#### UTILITIES AND COMMUNICATION

Primary electrical service to the ITR was 277/480 volts AC 3Ø 60 Hz supplied from an adjacent building bus via a utility pole. Four distribution panels were provided in the control room: 480-volt, lighting, regulated-power, and 28-VDC.

Raceways and conduit were provided between the control room, the work platform and the PPG location.

Specific power requirements were serviced from the following sources:

115-Volt Regulator - Stabiline EMT 4115

15 KVA  
Input: 95-135V  
Output: 115V 50/60 cy. 1Ø  
110-120V adjustable  
Long-range amps: 130.0

400-Hz Electronic Inverter - Sorensen FCD 3P1000

1000 VA - 3 Ø  
Input: 208/230V 48 to 65 cy 3 Ø  
Output volts: +10% 115/200V  
3 Ø 4 wire  
115V L-L 3 Ø 3 wire  
Output volt reg: +1%  
Output freq.: 360-440 cy +1%  
Load range: 0-1000 VA  
Distortion unity PF load: (360-400 CPS) 3%

28VDC - Instrumentation

Solid-state power supply - Sorensen MA28-125

Output:

<u>Volts</u> <u>(DC)</u>	<u>Current</u> <u>(AMPS)</u>	<u>Reg.</u>	<u>Metered</u>
18-36	0-125	.2%	Yes

28VDC - PPG Starting System

4-12VDC heavy duty truck batteries

Communication provisions included:

1. Telephone linking the control room, plant switch-board and base of tower.
2. Powered intercom system including four headsets, two keyed and two noise-cancelling microphones, and five interconnect boxes.

#### PNEUMATIC POWER GENERATOR (PPG)

Four possible PPG locations were considered: two on the overhead structure and two on the ground level. The two locations at the top of the structure were eliminated due to the additional structure required. In comparing the ground locations - the bay below the control room and the ground bay of the opposite tower - the latter was selected for the following reasons:

1. Elimination of:
  - a. fire hazard in vicinity of stairway for operating personnel.
  - b. the need for a fireproof structure around the air supply unit and special ducting, and
  - c. the need for an alternate stairway or secondary means of egress.
2. Simplification of PPG fuel system.
3. Ease of servicing and isolation from personnel.
4. Provision of direct view of PPG from upper-level control room during operation.

The pneumatic power generator unit (air supply) was self-contained and consisted of the following components described in the system schematic, Figure 6:

1. 250-C20 Allison engine rated @ 294 HP at sea level 95°F, serving as a power unit. Additional components include a starter/generator, voltage regulator, 24VDC battery pack, fuel boost pump, oil reservoir, and oil/water heat exchanger.
2. A T-63 compressor with a modified T-63 gearbox.
3. An interface gearbox (step-up) and power transfer shaft as shown in the overall gearing arrangement, Figure 7.
4. A surge control valve designed to prevent compressor surge under all operating ambients from zero to maximum capacity of the compressor. The valve was controlled by a vacuum switch located in the compressor inlet bell mouth.
5. A pressure controller compatible with the cargo system pneumatic power distribution system. This valve was controlled by pressure sensed in the header upstream of the hoists.

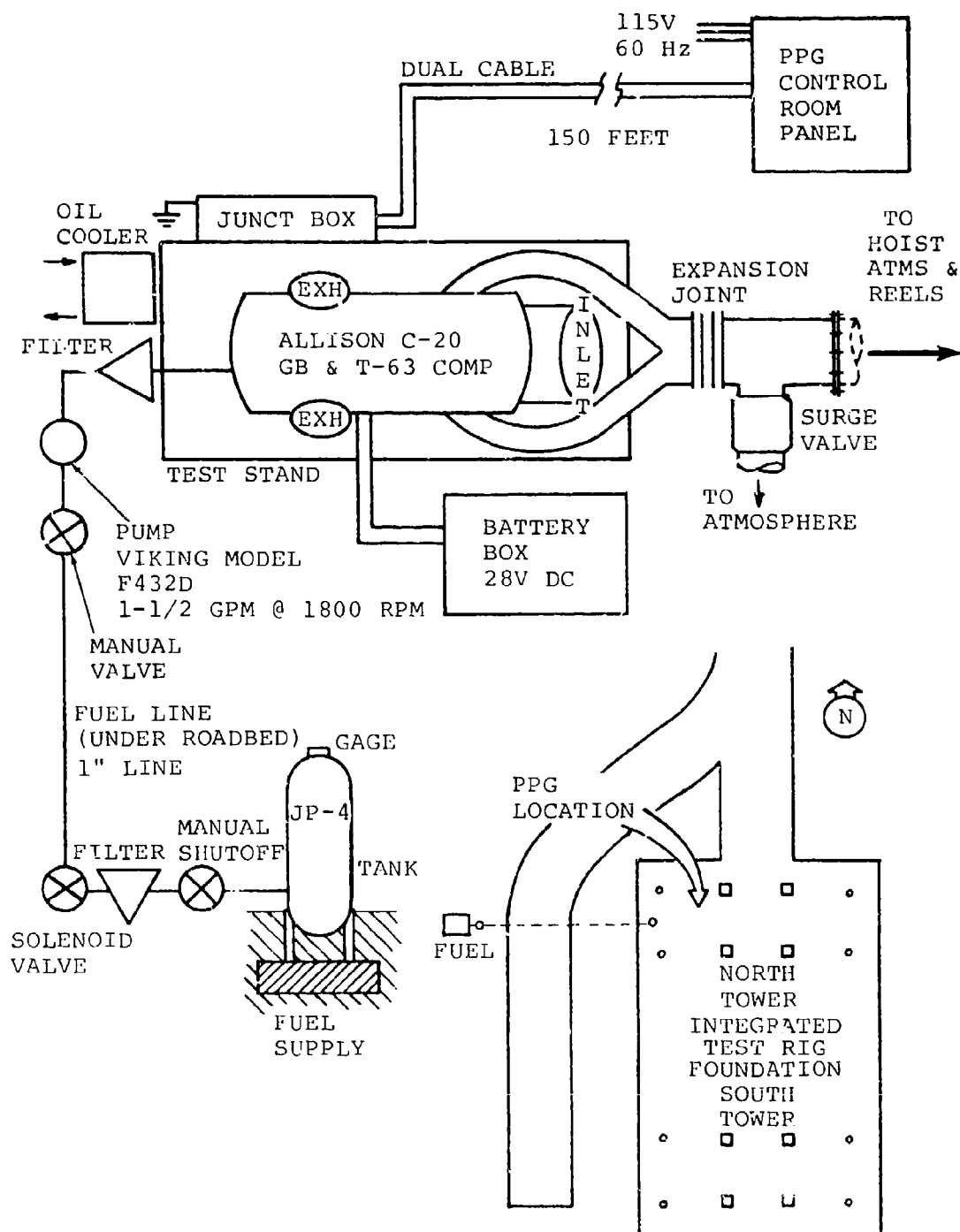


Figure 6. Schematic - HLH/ATC Pneumatic Power Generator (PPG) and Generator Location.

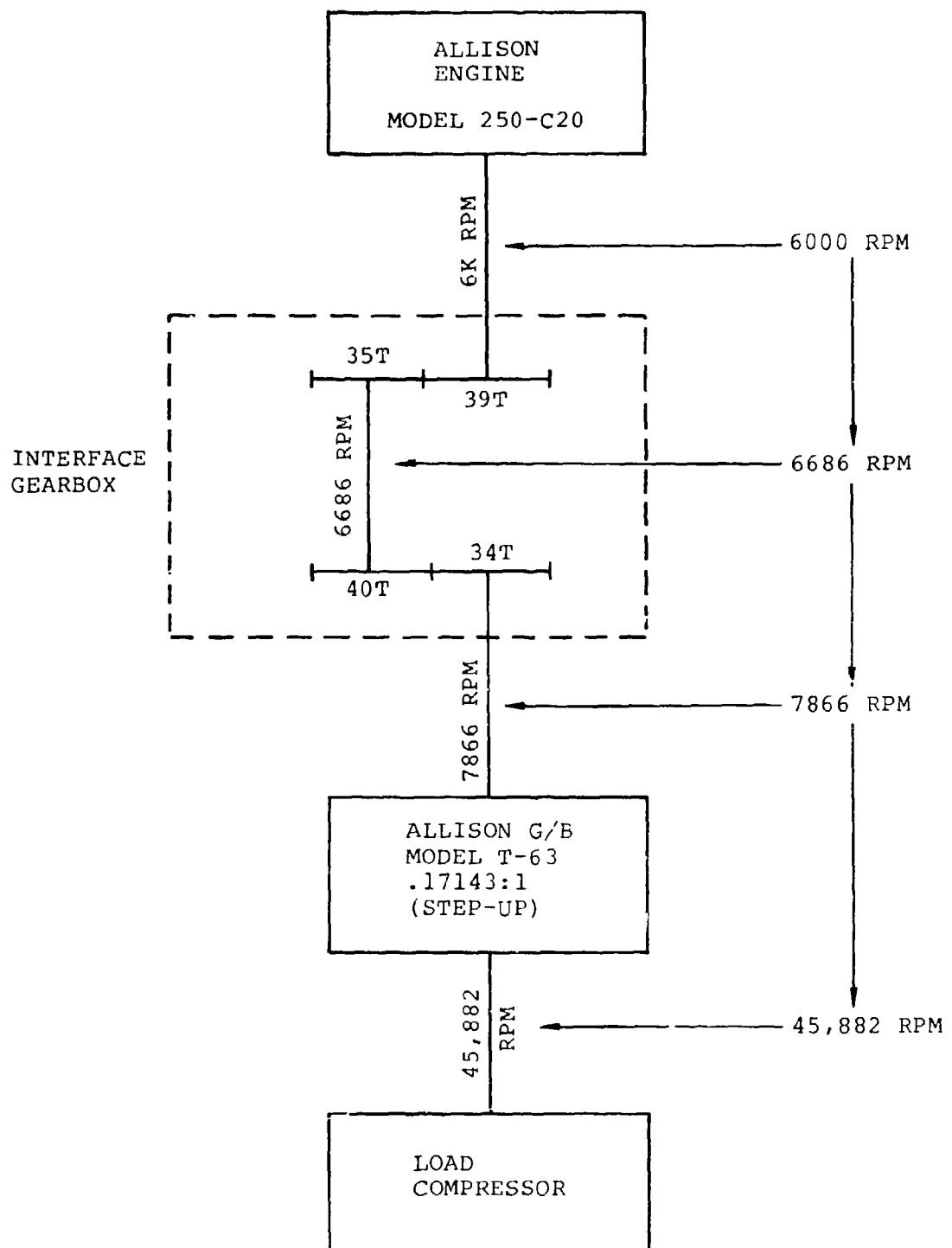


Figure 7. PPG Gearing Arrangement.

6. PPG control panel located adjacent to the LCC station, which included engine/compressor gearbox speeds, temperature and pressure instrumentation, fuel and battery charger controls, and emergency shutdown devices.
7. A 275-gallon fuel tank with manual and emergency valving with spark-arresting provision.
8. Approximate pressure gages and gearbox chip warning sensors and 150-foot control cable.

#### Air Distribution System

The air distribution system, constructed of Schedule 5 304 stainless steel, was rated for 60 psi at 543°F. The system consisted of:

- |                  |   |
|------------------|---|
| 1. Main riser    | 6-inch I D  |
| 2. Main header   | 6-inch, upstream of both hoists                               |
| 3. Supply risers | 3-inch, feeder between hoists                                 |
| 4. Insulation    | 2-1/2-inch flexible ducts                                     |
|                  | 3-1/2-inch, with weatherproofed aluminum jacket               |
| 5. Dump valve    | 2-1/2 globe, manually operated (for start and warmup control) |
| 6. Drain valve   | for removal of line condensation.                             |

A total pressure pickup and a thermocouple (I/C junction) were included in the 6-inch header to measure air supply conditions.

#### INSTRUMENTATION

The ITR instrumentation system was specifically designed to record and display the parameters necessary to evaluate the function and performance of the HLH cargo handling system. The parameters recorded and displayed were in addition to the load-controlling crewman station monitoring equipment.

An instrumentation console located in the ITR control room contained the equipment necessary to monitor and record the temperatures, pressures, isolator loads, cable angles and other auxiliary signals.

A listing of the instrumentation equipment used is shown in Table II. For wiring and other detail, refer to the Instrumentation Family Tree ST30861 listed under Design Layouts and shown in Appendix II.



TABLE II. COMPONENT LISTING, INSTRUMENTATION SYSTEM - HLH CARGO HANDLING SYSTEM.				
Equipment	Model	Characteristics	Function	Warning Comments
Temp. Indicator, Love Controls (7ea)	100-818	Range 0-600 Deg T/C Type "J" (I/C) Accuracy $\pm 1\%$ of scale Recorder output 0-1V	Indicates exhaust temp; supply air temp & hoisting & rev. valve temp.	No Auxiliary output used to drive recording galvo.
Temp. Indicator Love Controls (1ea)	Same	Same	Indicates ATM temp. for operator monitoring	No Operates thru selector switch for ATM temp.
Temp. Selector Switch Thermo-Electric (1ea)	33112	No. channels-12	Switches ATM T/C inputs	-
Temp. Probe; Thermo-Elec. (1ea)	5J2120L	Accuracy $\pm 0.5^\circ$ Probe Length 6" Type Iron-Constantan	Temp transducer for temp indicators	-
Pressure Ind. Dynisco (5ea)	ER466A2	Range 0-100 PSI Accuracy $\pm 1\%$ full scale Response time 1.5 sec Aux. output 0-1 Volt	Indicates supply air, hoisting & rev. air pressures	Yes Auxiliary output used to drive recording galvo, conditions & amplifies pressure transducer signal.
Pressure Transducer (5ea)	103	Range 0-75 PSI, 112.5 PSIA Max.	Press. transducer for press. indicators	-
Load Isolator panel	ST30869	Range 0-100KIP Accuracy $\pm 0.1\%$ of input	Indicates compression load in each of four load isolators	No Receives load signal & ref. from cable tension & angle interface electronics

TABLE II. CONTINUED.

Equipment	Model	Characteristics	Function	Warning	Comments
Cable Angle Ref. Panel	ST30873	Range 0-100 Deg. Accuracy $\pm 0.1^\circ$	Indicates angle of four-hook cables	No	Same as above
Closed-Circuit TV System Hitachi Ltd.	ST30852	Range-50 yards Field of view 3ft x 3ft	Used to observe moving parts on cargo handling modules & container position	No	—
Intercom System	ST30859	No. of stations-4 Type-Headset with mike	Used for communication between the operator & crewmen	—	—
Oscillograph Recorder CEC (3ea)	5-124	No. channels-18 Speeds 0.25, 1, 4, 16, 64 IPS	Used as graphic recording devices	No	—
Pressure Probe United Sensor	PTC12	Adjustable immersion length Max. operating temp-1600°F	Static pressure transmitter	No	—
	PTC8	Same as above	Total pressure transmitter	No	—
Galvo CEC (21ea)	7-315	Frequency response 0-60 Hz (+2%) Sensitivity 12.2 UA/IN Impedance 26 Ohms	(Used as transducers for graphic recording oscillograph)	No	See dwg. ST30854A for recorder parameter info.
Galvo CEC (2ea)	7-319	Frequency response 0-350 Hz (+2%) Sensitivity 426 UA/IN Impedance 26 Ohms	Same as above	—	Same as above

TABLE II. CONCLUDED.

Equipment	Model	Characteristics	Function	Warning	Comments
Galvo CEC (7ea)	7-351	Frequency response 0-12 Hz (+2%) Sensitivity 2.66 UA/IN Impedance 33 Ohms	Same as above	-	Used to record outputs of temp. indicators-see ST30854A.
Oscillator H-P (1ea)	200CDR	Range 0-600 kHz Output 0-40 Volts	Used as time event marker source	-	-
Amplifier Burr-Brown (6ea)	2088/16	Frequency range 0-10 kHz Gain ADJ 1-1000 CMR ADJ 40-140 DB	Used as buffer amplifier-see ST30860	No	-
D.C. Null Meter G-P	413A/R	Range 1mV-1000 V Accuracy $\pm 0.1\%$ of input	Used as an auxiliary monitor	No	-
Power Supply Kepco	ABC 10.C.75M	Range 0-10V 0-0.75A	Used as a calibration voltage source	No	-
Load Cell Bud (2ea)	T3P2B	Range 0-50K lb Sens 3mV/V	Used to measure hook load	No	-
Meter Panel Simpson	-	Range 0-50mA Accuracy $\pm 1\%$	Used to indicate cable speed	No	Receives cable length signals from hoist interface electronics.
Anemometer-Taylor Instrument Co. Rochester, N.Y.	-	Dual range-zero adjust 0-25 mph 0-100 mph	Used to indicate wind velocity & direction	No	-

The instrumentation system was configured, as shown on Figure 8, in a two-bay console designated rack "A" and rack "B". Rack "A" included the equipment to monitor and record hoisting and reversing air turbine motor (ATM) temperatures, exhaust temperatures, main supply air temperature, hoisting and reversing air pressures, supply pressure, isolator loads, and cable angles. The recording oscillograph, closed-circuit television (CCTV) system switch panel, +15 volt power supply, event oscillator, and intercom station were also located in rack "A". The CCTV monitor, DC null voltmeter, hoist control unit, hoist interface electronics, cable tension and length interface electronics, recording oscillograph and buffer amplifiers were located in rack "B". The data recording format is shown in Table III. Additional equipment added to the system included a recording oscillograph "C" used for recording cable tensions, two null panel meters for observing cable payout speed, an oscillograph control box with event button, a brake release counter, eight chip detectors, and a 12-channel temperature panel with selector switch. During initial checkout, supply pressure and antisurge valve characteristics were investigated. An x-y plotter, B&F conditioner with amplifier, and a dekabox were used to plot supply pressure.

#### Temperature Instrumentation

Temperature probes were inserted into the air ducts of each hoist drive unit to measure the turbine inlet, exhaust and supply air temperatures. They acted as transducers for the seven temperature indicators in the instrumentation system.

Evaluation of the hoist duty cycle required monitoring of the surface temperatures of the hoist main bevel gear casing, the secondary forward and aft bevel gear casings, the hoist brake casing, the ATM oil pressure line, and the ATM oil return line of each hoisting module. Silver-soldered T/C junctions were attached to each of the six surfaces with Caulk grip cement (dental cement) and routed to a 12-channel T/C selector switch via a six-lead thermocouple cable. The selected channel temperature was displayed on a separate indicator. The selector switch and temperature indicator were located on a panel which was positioned in front of the hoist operator.

#### Pressure Instrumentation

Pressure probes were inserted into the air duct at the same location as the temperature thermocouples in the ATM inlet and were connected by flexible tubing to the pressure transducers.

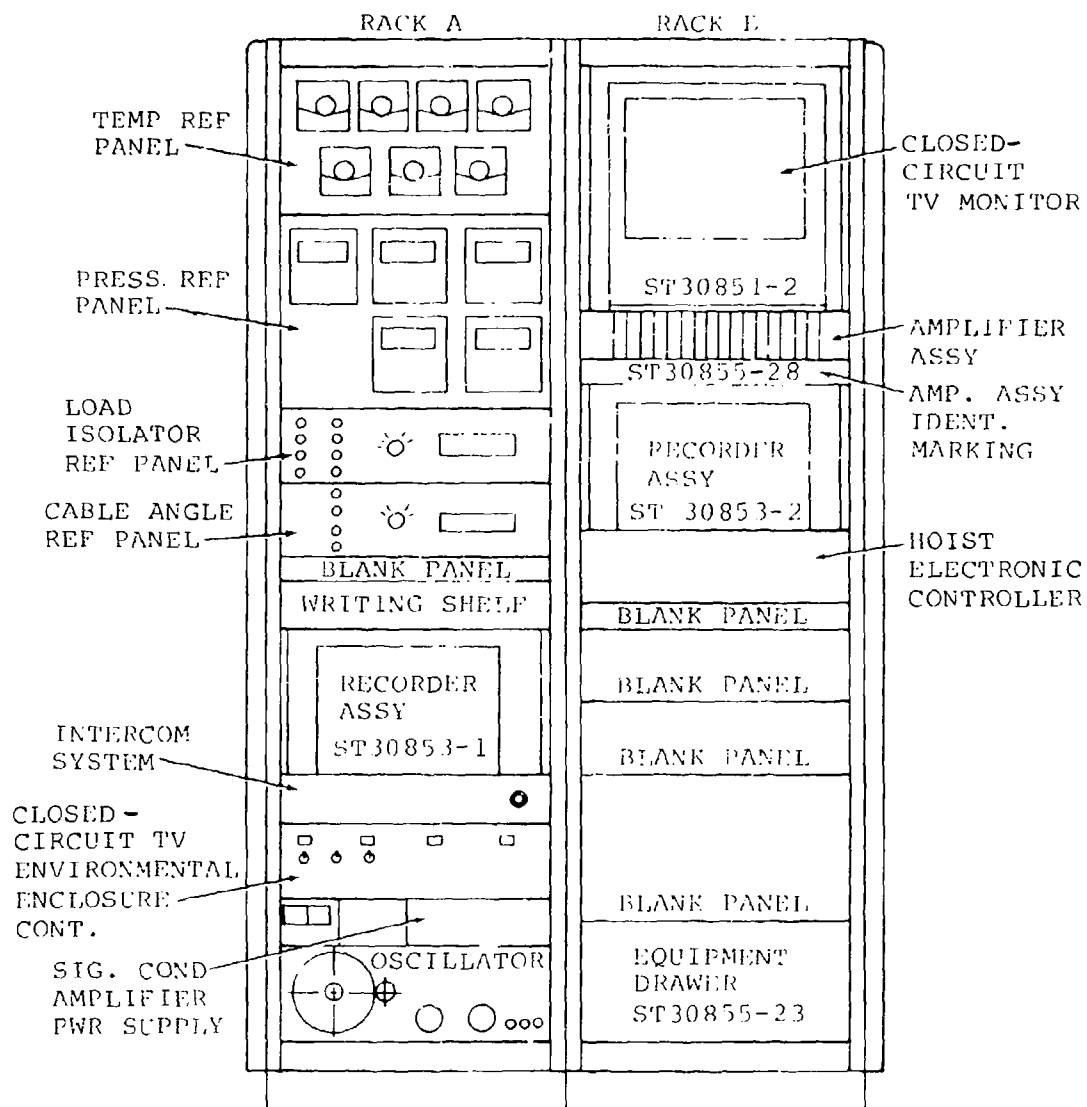


Figure 8. Instrumentation Rack Console ST30855-1.

TABLE III. ITR INSTRUMENTATION SYSTEM - RECORDED DATA FORMAT.					
Recorder A		Recorder B		Recorder C	
Chnl.	Measurement Parameter	Chnl.	Measurement Parameter	Chnl.	
1	T1-Fwd,hoisting,°F	1	P2-Fwd,hoisting,psig		<u>Load Isolators</u>
2	Time/event	2	P4-Aft,hoisting,psig	1	Fwd hoist, fwd
3	T2-Fwd,reverse,°F	3	A1-Fwd,turbine speed	2	Fwd hoist, aft
4	T3-Aft,hoisting,°F	4	-- Blank	3	Aft hoist, fwd
5	T4-Aft,reverse,°F	5	A2-Fwd,cable payout	4	Aft hoist, aft
6	T5-rwd,exhaust,°F	6	A5-Aft,turbine speed	5	Load cell
7	-- Blank	7	-- Blank	6	Time/event
8	T6-Supply air,°F	8	A6-Aft,cable payout		
9	T7-Aft,exhaust,°F	9	-- Blank		
10	P1-Supply air,psia	10	Fwd,speed command		
11	P5-Aft,reverse,psig	11	A7-Aft,H,Mod.Valv.amps		
12	Aft,torque sw.	12	Aft,speed command		
13	Aft,brake sw.	13	-- Blank		
14	P3-Fwd,reverse,psig	14	A3-Fwd,H,Mod.Valv.amps		
15	-- Blank	15	-- Blank		
16	Fwd,torque sw.	16	-- Blank		
17	Fwd,brake sw.	17	Sync.Signal		
18	-- Blank	18	Time/event		

### Load Isolator Instrumentation (Cable Tension)

A load cell for axial load sensing was an integral part of each load isolator. The output from the 350-ohm strain gage bridge on the cell was amplified and conditioned by the hoist cable tension and cable length interface electronics.

### Cable Tension and Load Instrumentation

The load isolator reference panel located in rack "A" had a five-position rotary selector switch, four calibration adjustment potentiometers and a three-place digital panel meter which displayed the selected axial load.

An auxiliary method of measuring cable tension was employed using two load cells in series with each coupling and a BLH universal percentage indicator.

### Cable Angle Instrumentation

The cable angle sensing mechanism employed two linear transformers (see Figure 9) which produced electrical signals proportional to the longitudinal and/or lateral angles that the cable made with the vertical centerline of the hoist.

The cable angle reference panel had a five-position selector switch, four angle adjustment potentiometer, and a digital panel meter for readout.

### Cable Payout Instrumentation

The cable length sensing mechanism (see Figure 10) employed a linear transformer whose electrical output signal was a function of the number of turns of the cable drum from the cable up (stow) position.

Two cable length signals, one for each hoist assembly, were recorded. By relating the cable payout trace deflection to oscillograph paper speed, a cable payout rate or load velocity could be determined.

### Hoist Input Command Instrumentation

The hoist operator's control grip thumb switch supplied a command signal to the hoist controller logic. The command signal, conditioned to a 0-5VDC output for 0-100% command and interfaced with other controller network logic, determined the hoisting or lowering rate. In the single hoist mode, each command signal was recorded on rack "B" oscillograph.

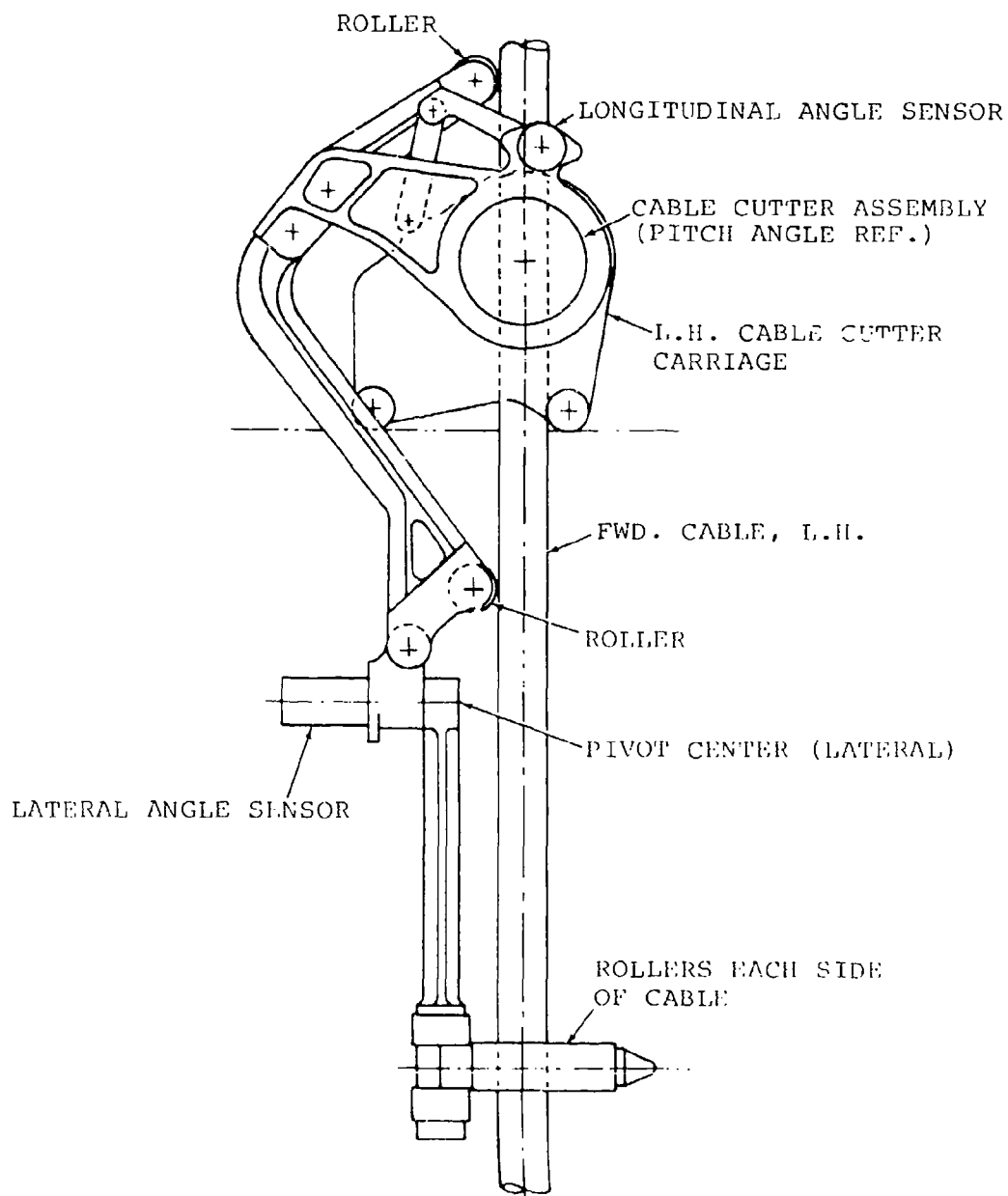


Figure 9. Cable Angle Sensor Mechanism.



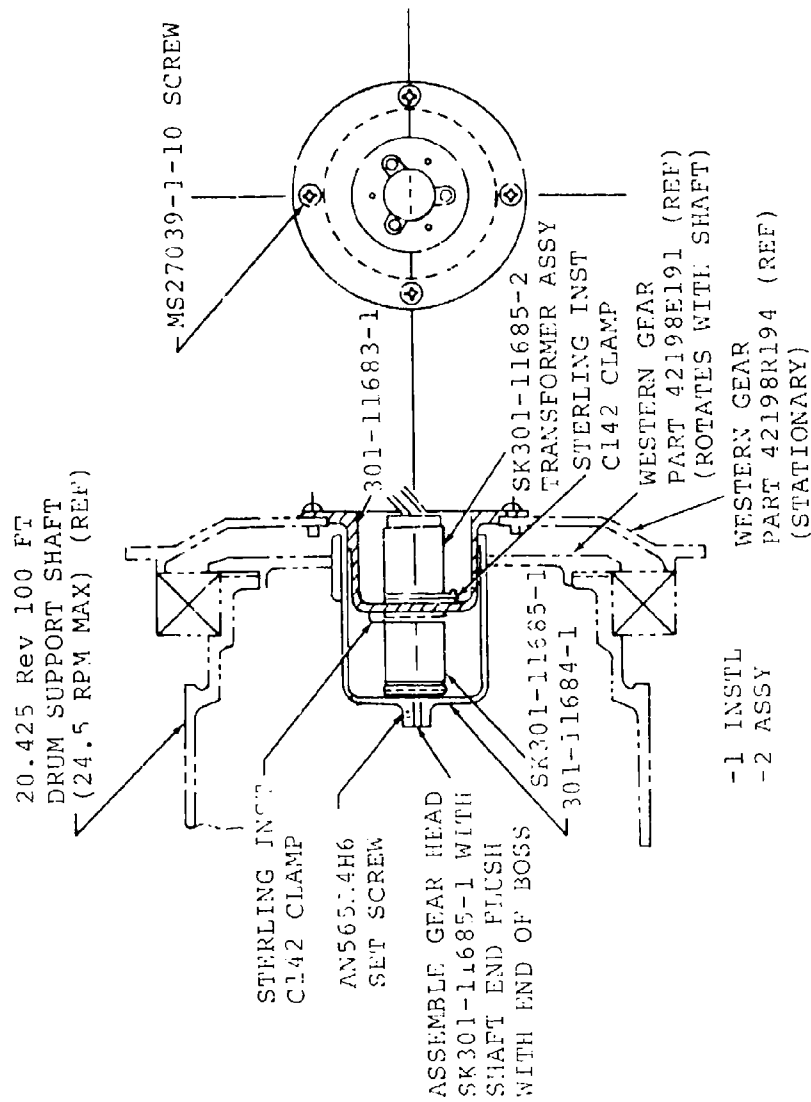


Figure 10. Cable Payout Sensor •

In the sync mode, only the forward hoist command signal was recorded since the aft hoist unit was commanded from the forward grip thumb switch.

#### Cargo Coupling Instrumentation (Mechanical Release Signal)

The hoist operator's cargo system control panel mechanical release switch was used for recording the command signal to the coupling solenoids and was recorded on rack "B" oscillograph.

#### Cable Payout Speed Instrumentation

The cable payout was derived from a sensing mechanism employing two magnetic pickups (MPU) which sensed ATM shaft speed. The ATM shaft speed was directly related to cable payout speed by the system gearing as represented in Table IV.

The MPU analog signal was conditioned and recorded on the rack "B" oscillograph.

#### Cable Speed Instrumentation

The payout speed signal was also fed to two zero center panel meters directly in front of the hoist operator. This provided the operator with a visual indication of cable payout speed and rate change or load velocity.

#### Power Requirements

The instrumentation system required 115 VAC 50-60 Hz and 28 VDC power supplies.

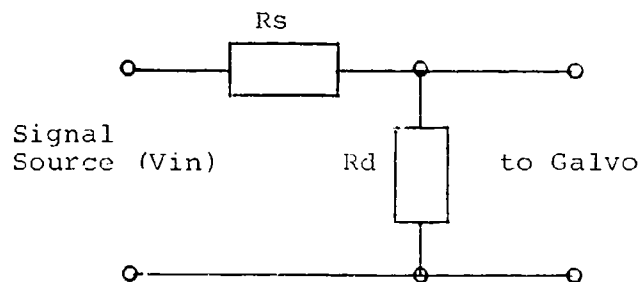
#### Recording Instrumentation

The recording instrumentation consisted of the galvanometer input network, the galvanometer, and the recording oscillograph which housed the galvos. The galvo input networks were as shown in Figure 11. The networks were installed on the terminal boards on the junction panel located in rack "A". All the recorded signals from each source were terminated in the same manner with the matching networks located on the rack "A" junction panel.

The recording galvanometers selected for pressure, rpm and modulating valve measurements had a minimum upper frequency response limit of 200 Hz to permit observation of oscillatory, fluctuating or step input signals.

TABLE IV. ATM SHAFT SPEED RELATED TO CABLE PAYOUT  
SPEED BY SYSTEM GEARING.

Cable Speed (fpm)	Shaft Speed (rpm)	MPU Frequency (Hz)	Speed Signal (VDC)
120	8,000	5,250	5.00
60	4,000	2,625	2.50
30	2,000	1,313	1.25
15	1,000	656	0.625
7.5	500	328	0.312
3.75	250	164	0.156



Resistor values were derived from the following formula:

$$R_s = \frac{V_{in}}{D \cdot I_g} \quad \text{where}$$

- $R_s$  = calculated source resistance
- $V_{in}$  = input voltage
- $D$  = desired deflection in inches; usually 2.0
- $I_g$  = galvo undamped D-C sensitivity in UA or MA/IN.  
(see manufacturer's specs)
- $R_d$  = required damping resistance, from manufacturer's spec sheet

Figure 11. Galvo Input Networks.

Galvanometers with a lower frequency response limit were used for the temperature, cable payout and command signal instrumentation.

Six Burr-Brown amplifiers located in rack "B" were used as buffer amplifiers to isolate those signal sources which did not have sufficient current magnitude to drive the recording galvanometers directly.

A 5-Hz signal was fed to one recording channel on each oscillograph to provide an accurate time/data correlation. The signal was in series with an event switch which served to correlate the start time on each of the three recording oscillographs. The event switch was located at the LCC station, with two recorder "on" lights and two recorder start switches.

#### Closed-Circuit TV System (CCTV)

A CCTV system was installed to monitor the moving parts on the hoist assembly during system testing. The system consisted of a monitor, a switch panel, and a camera enclosed in an environmental housing. The switch panel was located in rack "A", while the monitor was at the top of rack "B". The camera zoom lens is manually adjustable for the desired field of view.

#### Intercom System

A four-station intercom system was provided to enable the LCC station to have constant communications with three other stations. They were:

1. Ground crewman station
2. Rack "B" instrumentation monitor
3. Hoist assembly station

Each station was fitted with a dynamic mike and headset combination.

#### Calibration Procedures

Each system was calibrated daily, prior to PPG startup and post-calibrated when the day's testing has been accomplished, as described in Appendix IV. Periodic calibrations were in compliance with MIL-C-45662. All instrumentation calibrations were traceable to the National Bureau of Standards.

### Container Weighing Instrumentation

The weighing instrumentation for the loaded MILVAN container consisted of two 50,000-lb Baldwin-Lima-Hamilton load cells and a Baldwin-Lima-Hamilton universal percentage indicator. The cargo handling system was in the two-point suspension configuration with one load cell in series with each hook. The SR4 universal percentage indicator was used to measure the individual loads. Connections to the load cells were made using two 70-foot eight-conductor cables from the load cells up to the test rig control room. The measured load using the load cells was within 0.7% of the calculated load.

### Maximum Load Test Instrumentation

Three of the load isolator transducers from the hoist units were used for the maximum static load demonstration test. The calibrated transducers were used as the main load-sensing elements during the test.

The isolator load signals which represented the hook load were conditioned and recorded on the CEC recording oscillograph, "C". The conditioned load signal was also displayed for the LCC on a digital voltmeter which indicated the load directly in pounds. The equipment is shown schematically in Figure 12.

### DESIGN LAYOUTS

The integrated test rig is described by the engineering drawings given in Appendix II. Besides the basic ITR structural design, additional subsystems were required for the assembly, installation, functional operation and test of the cargo system. Each of the required design items is identified on the following drawings:

#### Syster Test Drawing Tree - SK301-11676 (See Figure 65 in Appendix II)

- Hoist installation, including
  - Hoist module
  - Span positioning
- Control system wiring
- Assembly/handling fixtures
- LCC station platform
- Miscellaneous test fixtures and wiring

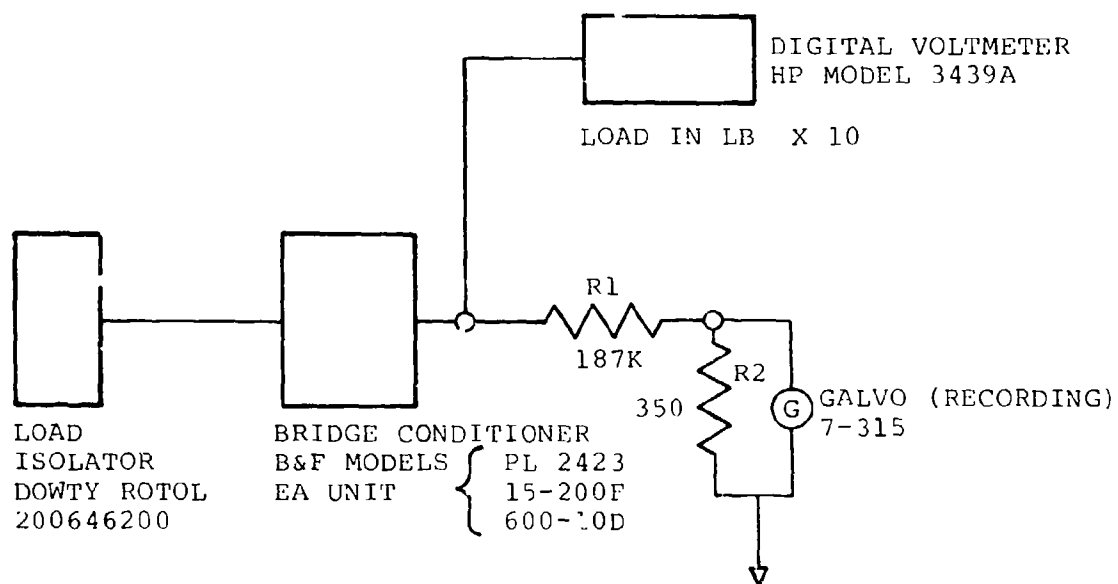


Figure 12. Maximum Load Test Network.

System Wiring - SK301-11694  
(See Figure 68 in Appendix II)

A schematic showing the electrical integration of the cargo handling system is shown in Figure 67.

FABRICATION

STRUCTURE PREFABRICATION

The following items were prefabricated and yard assembled prior to delivery to the test site:

1. Fore-aft frames
2. Control room enclosure
3. Hoist modules
4. Overhead beams
5. Overhead lateral beams and davit supports
6. Control room floor
7. Stairway sections
8. Outriggers and frame bracing
9. Footing pads

FOUNDATION

Excavations: exact site location, and footing designs were adjusted to utilize available rock strata. Figures 13 and 14 show the excavation, location of bedrock, and foundation preparation. Figure 15 shows the ground tiedowns provided in the foundation.

The rig was erected with prefabricated fore-aft frames joined at the splices and supported by outriggers. Inboard and outboard frames were assembled between the main columns on alignment pins and welded in place.

The main overhead beams were drilled and bolted on assembly. The lateral beam components and davit supports were then welded in place. Figures 16 through 26 illustrate the assembly method and main features of the structure. The control room enclosure assembly, Figure 27, was hoisted in place and joined to the main column pads. Figure 28 shows the ITR after complete erection, including the air distribution duct. For reassembly at another site, the towers (split into four units at the splice joints), the overhead structure disassembled to its basic components, the davit, outriggers, and control room shelter can be shipped as individual items. Table V delineates weight breakdown of the ITR structural assemblies.





Figure 13. Excavation for ITR Foundation.



Figure 14. Preparation of Footings and Foundation.

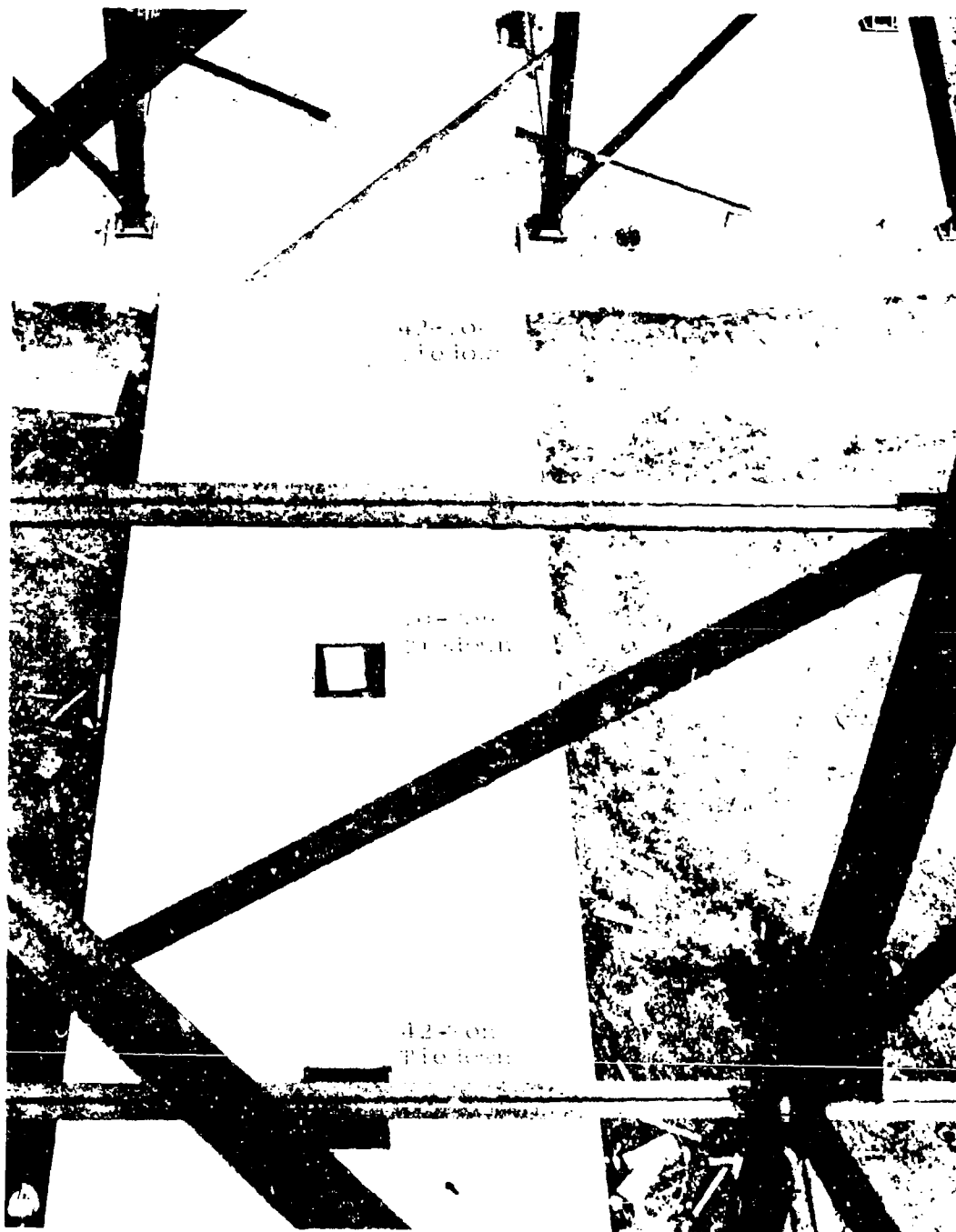


Figure 15. Three Ground Tiedown Points in Foundation.

<u>Number</u>	<u>Component</u>	<u>Number</u>	<u>Component</u>
1.	Lower Inboard-Outboard Frames	7.	Rails
2.	Upper Inboard-Outboard Frames	8.	Frame Struts
3.	Overhead Beams	9.	Modules
4.	Work Platforms	10.	Davit Mast
5.	Outrigger Members	11.	Davit Platform
6.	Stairways and Landings		



Figure 16. Rig Components and Raising of Lower Main Frames.

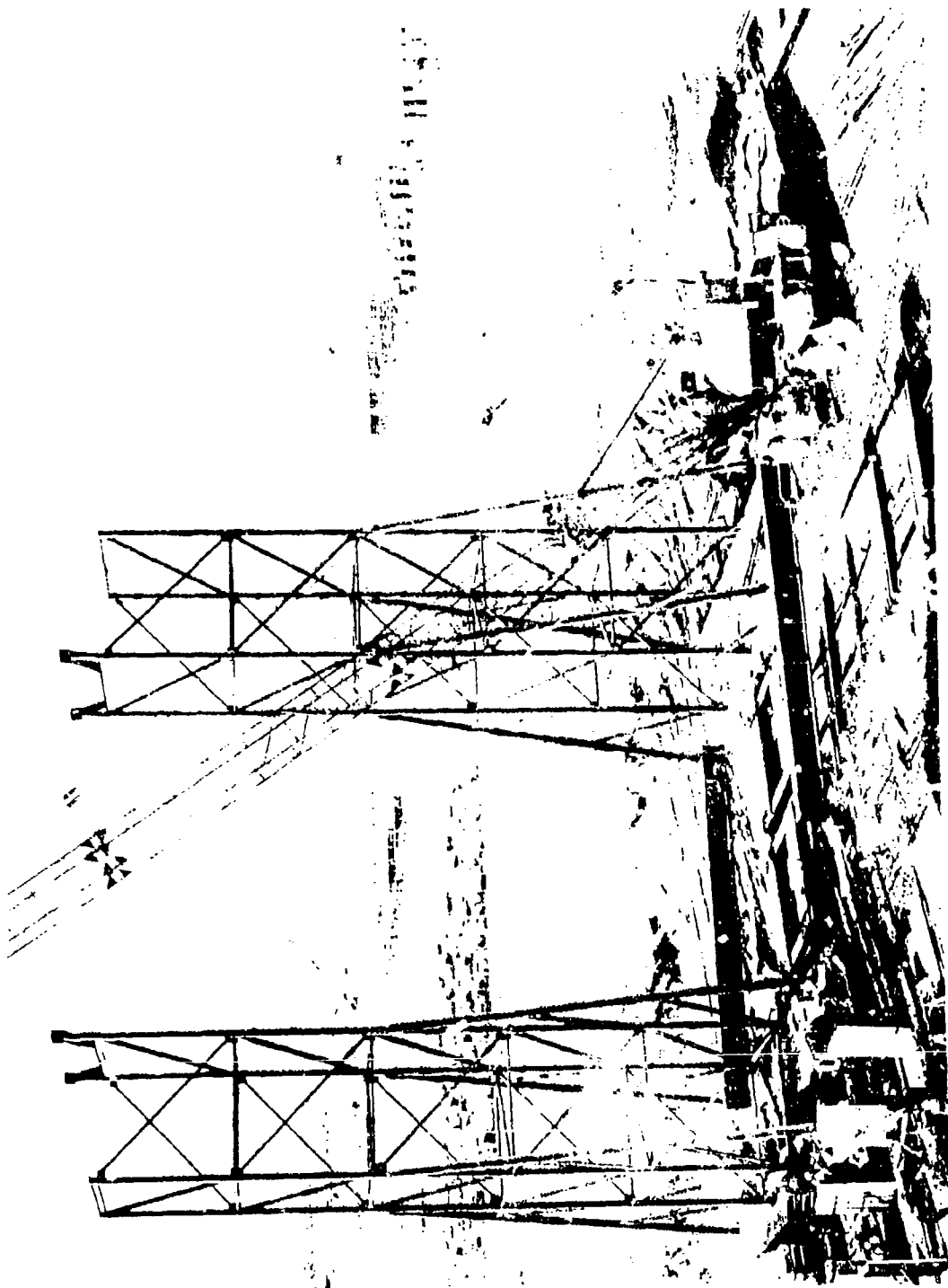


Figure 17. Complete Tower Frames Erection.

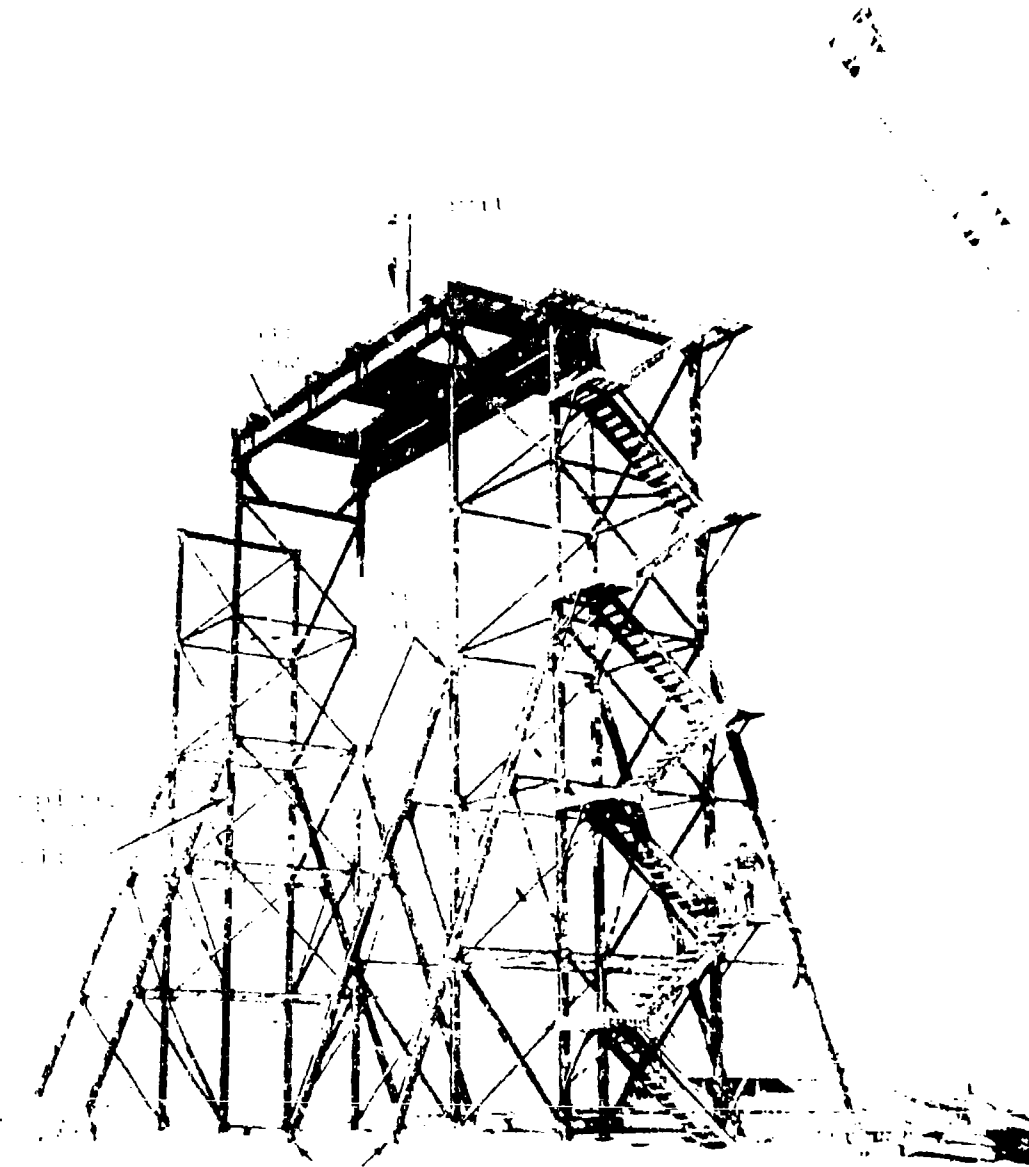


Figure 18. Overhead and Stairway Installation.

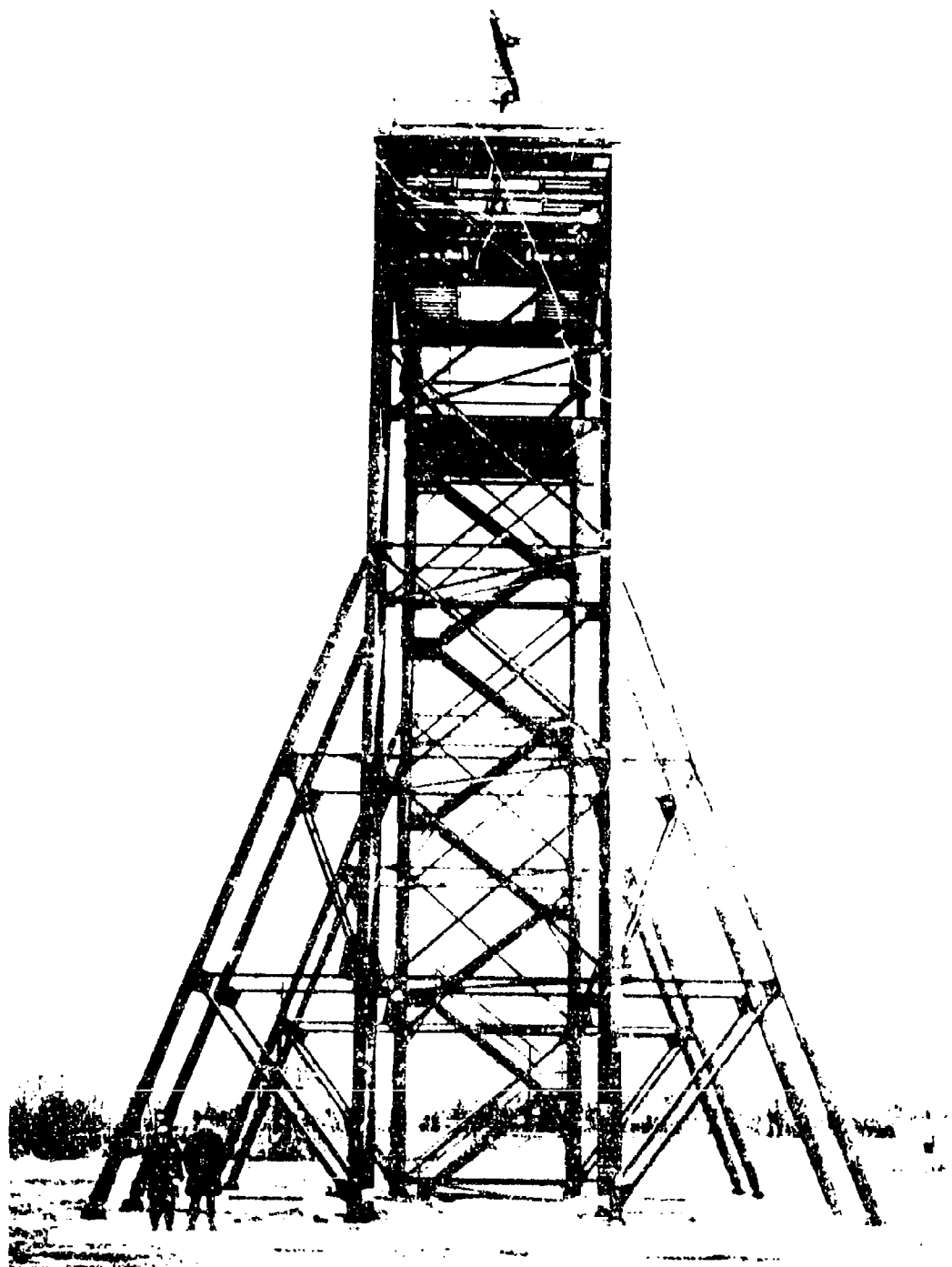


Figure 19. End view of tower (over).

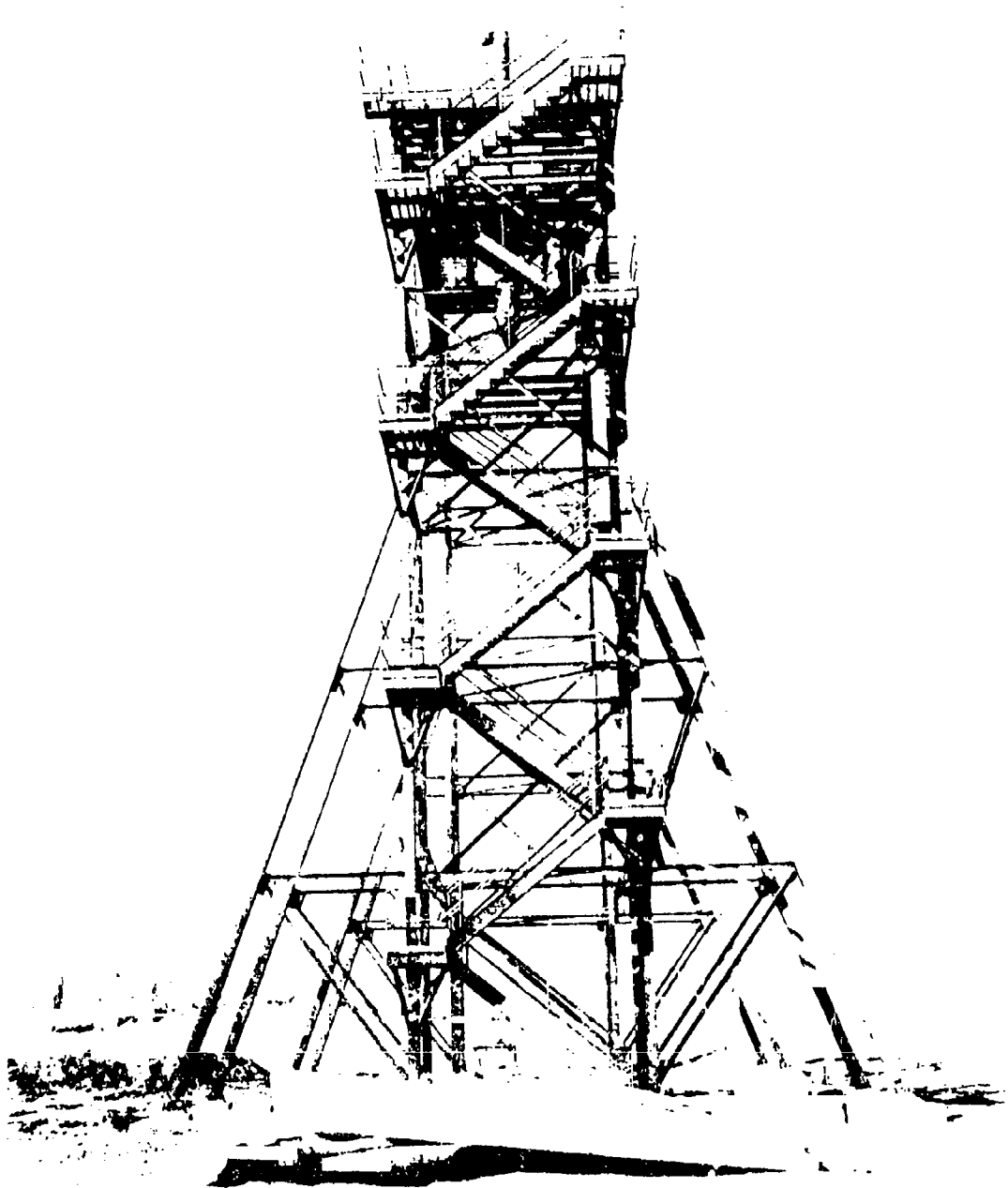


Figure 20. Side View of Forward Tower.



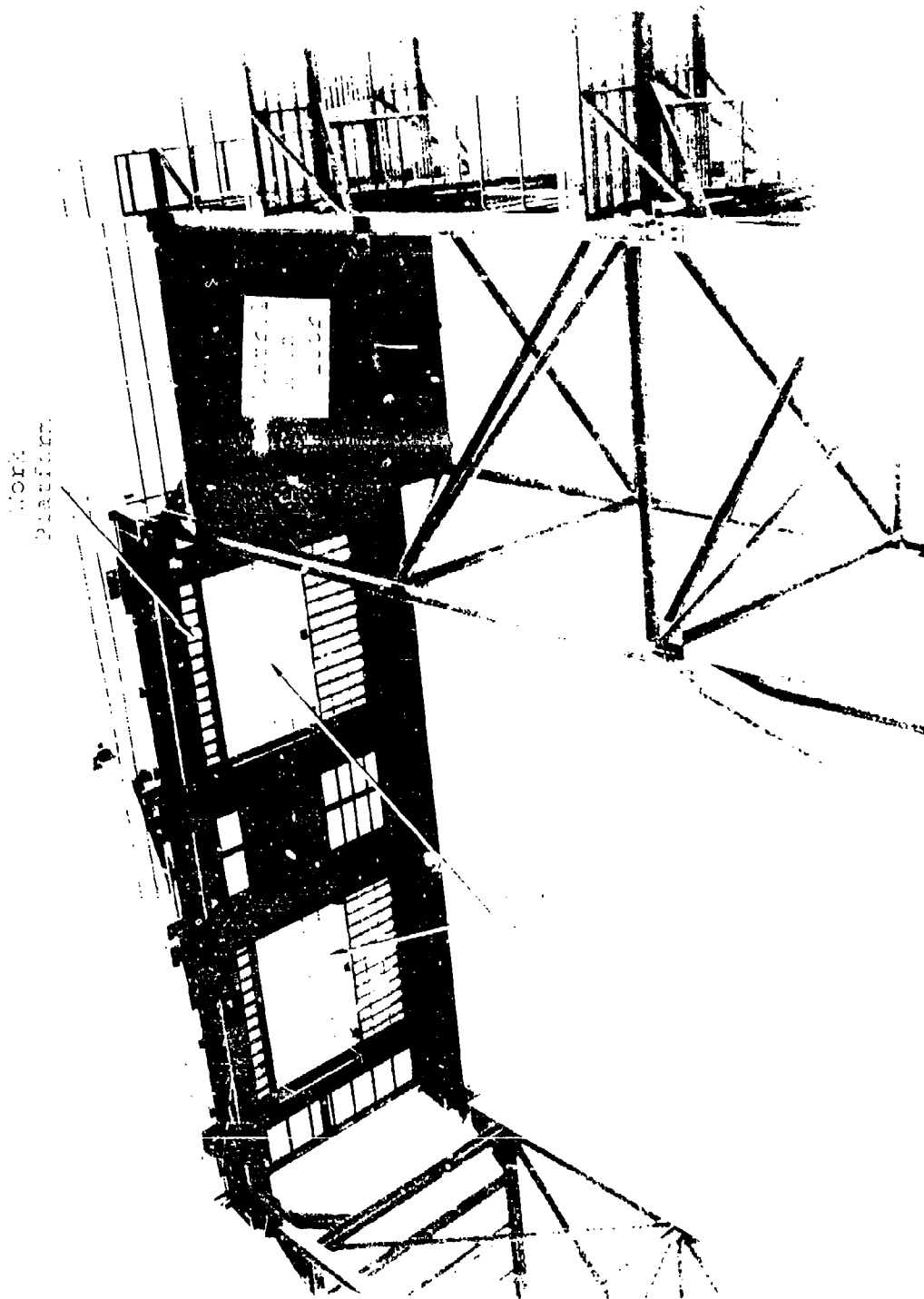


Figure 21. Under View of Overhead Structure and Hoist Module Location.

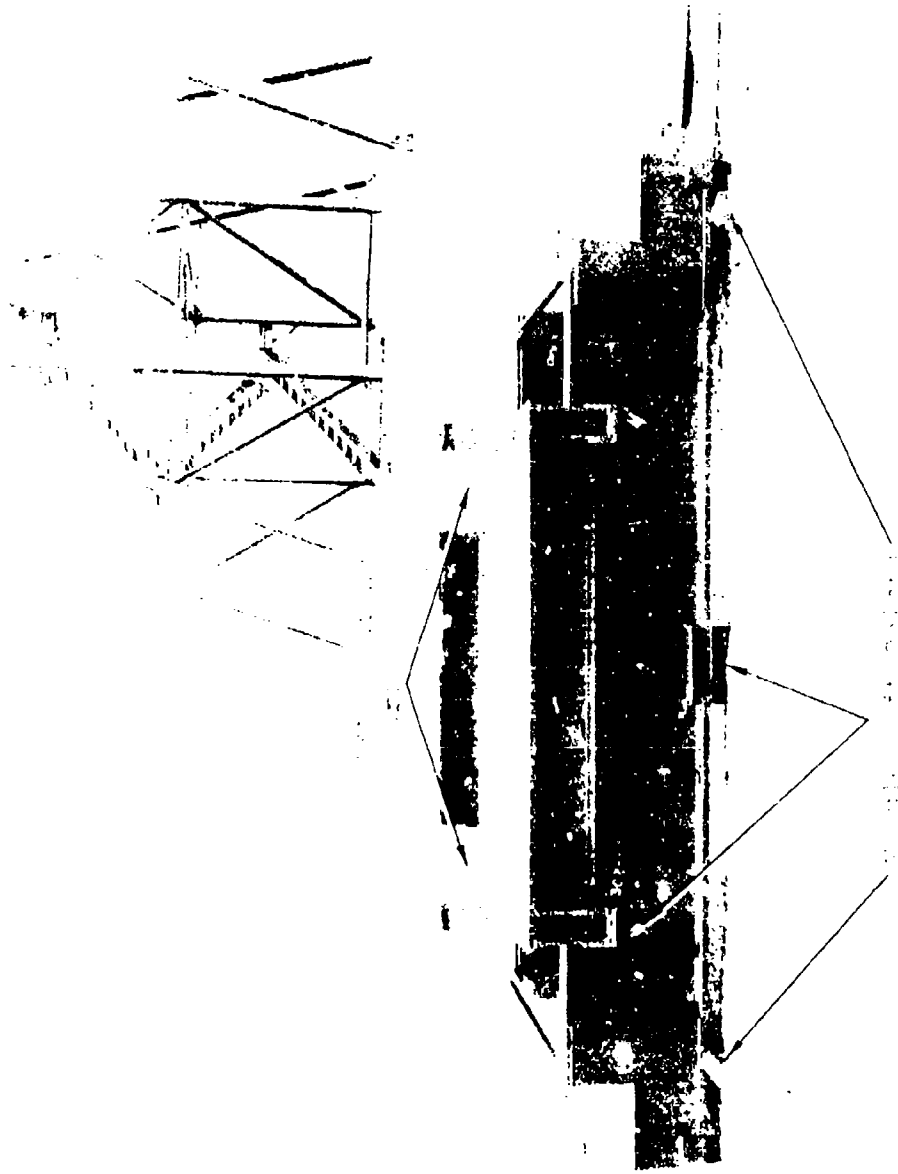


Figure 22. Hoist Modules (Inverted).

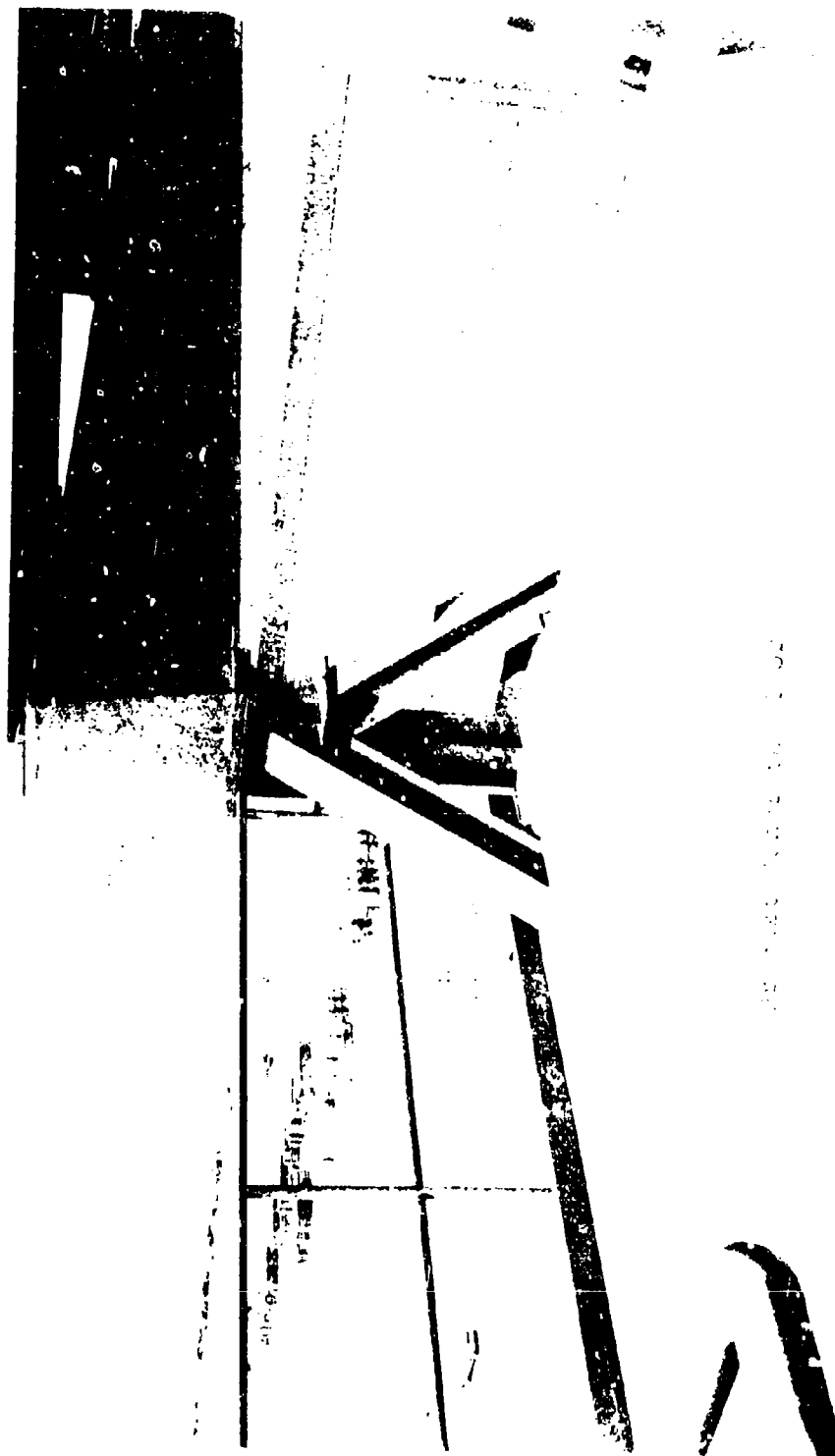


Figure 23. Control Room Area - Top of Forward Tower.

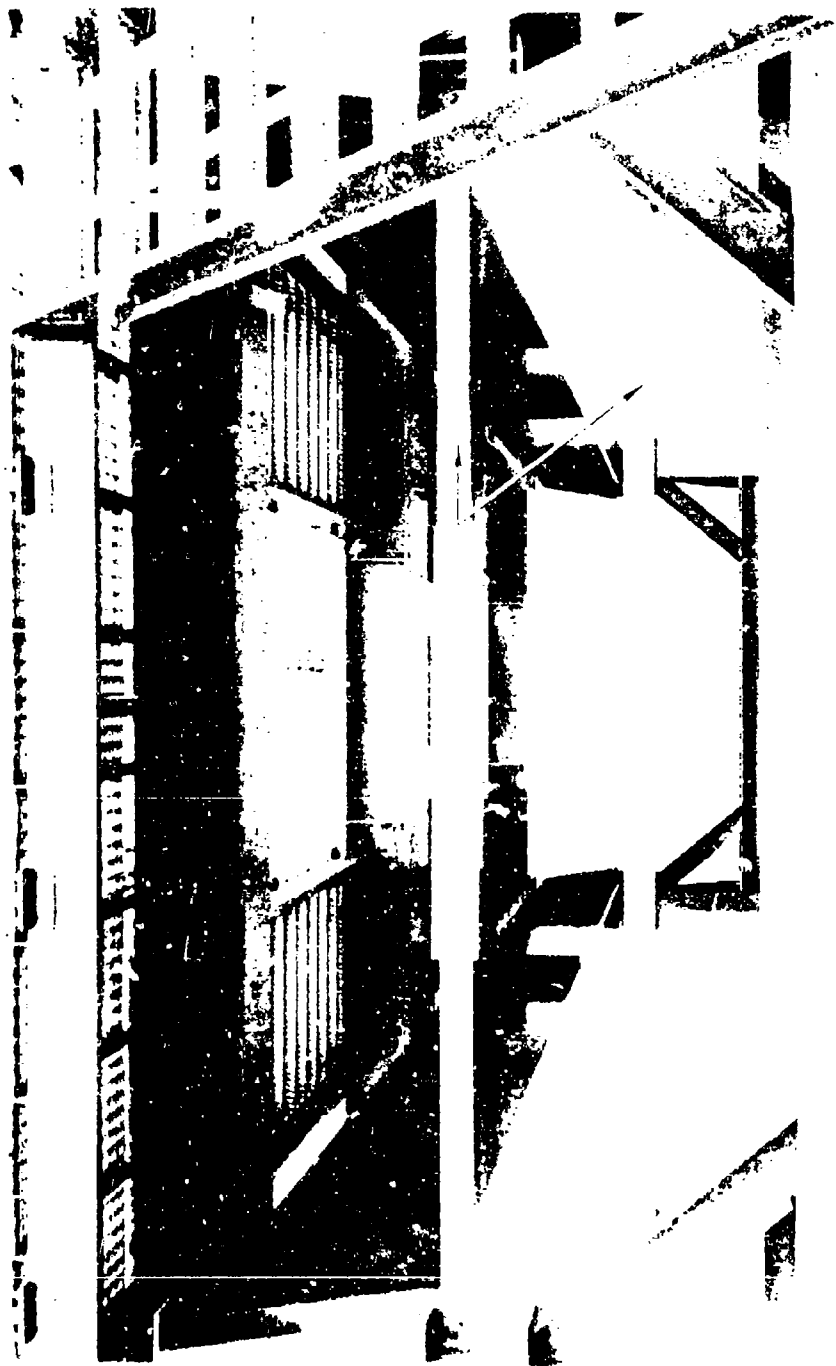


Figure 24. View of Overhead Section From LCC Position; Forward Tower.

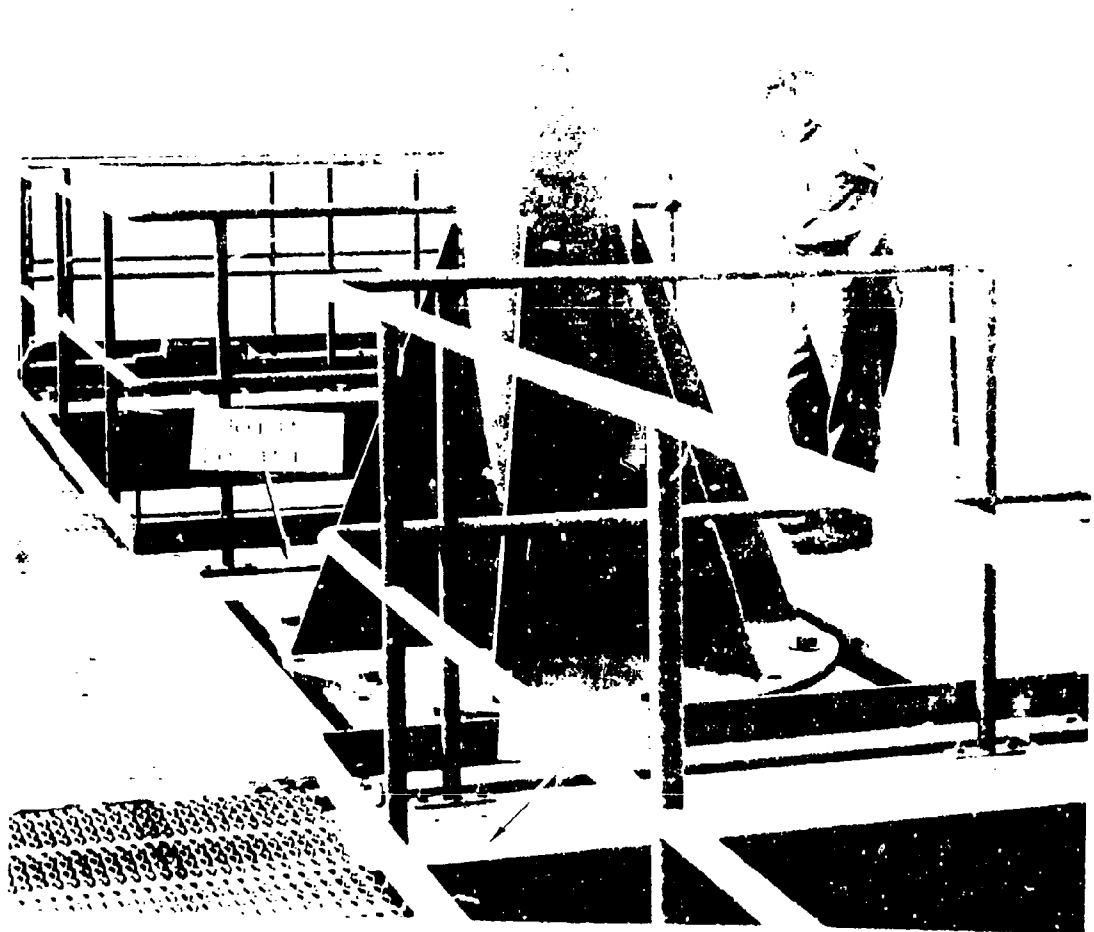


Figure 25. Work Platform and 14710 located during Control Room location.

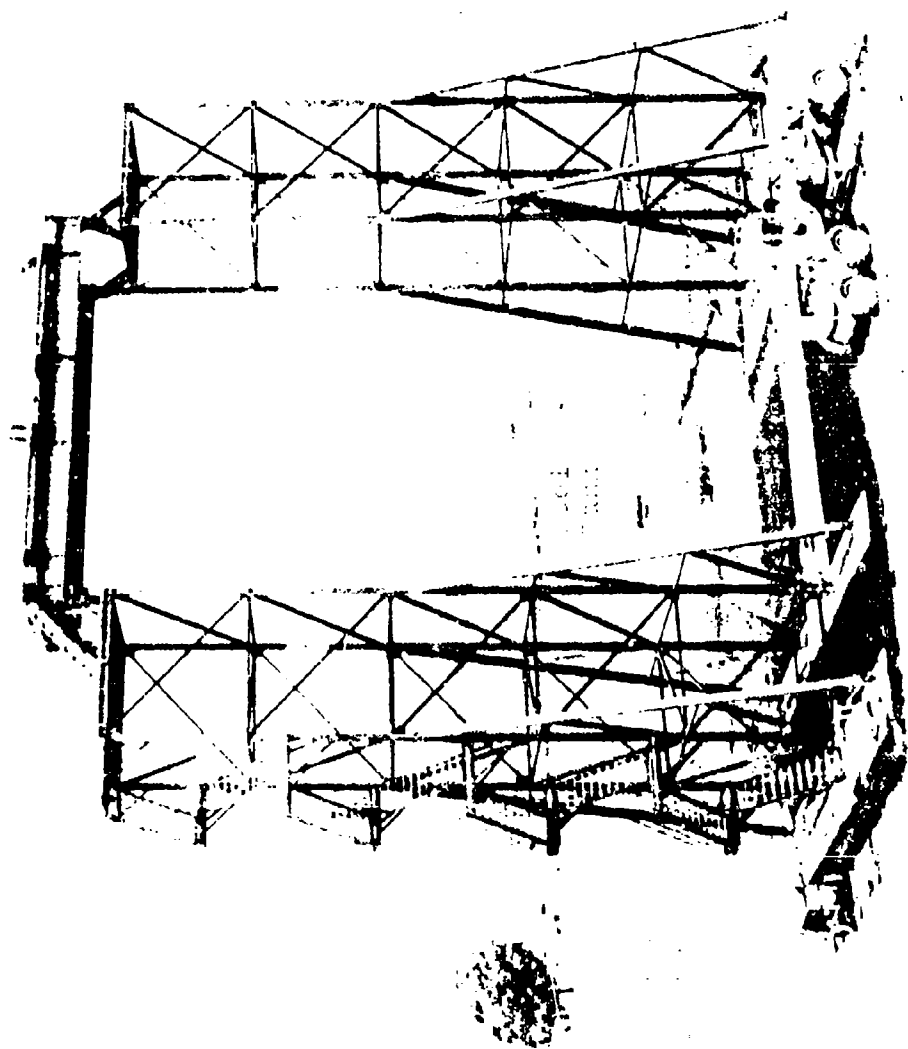


Figure 26. Completed ITR Framework on Foundation.

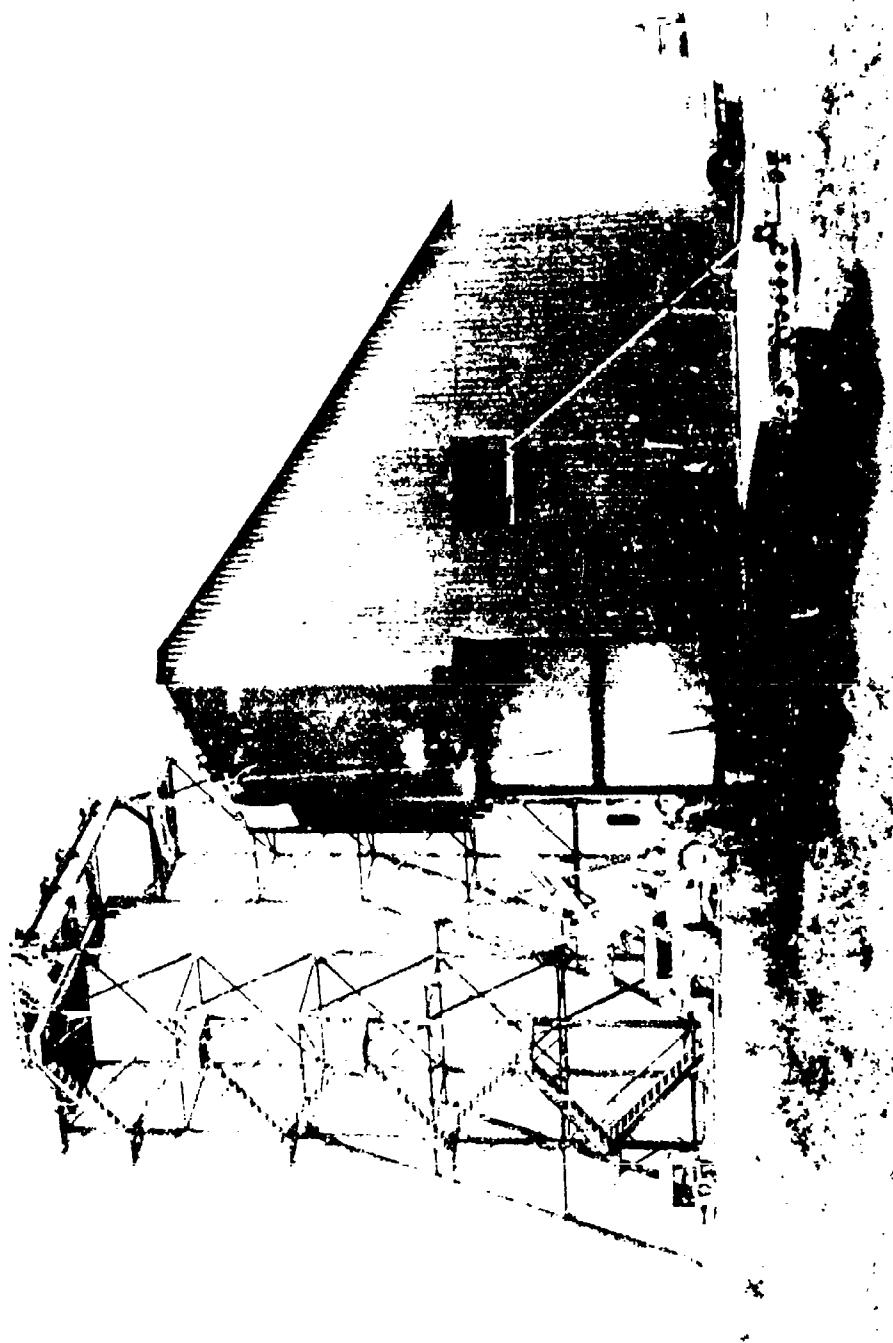


Figure 27. Control Room Enclosure Assembly.



Figure 10. Aerial view of complete structure and hot air distribution duct.



TABLE V. ITR WEIGHT BREAKDOWN.				
Drawing No.	Nomenclature	Qty.	Wt/Assy (lb)	Total Wt. (lb)
SK301-11304-1	Tower Assy.	1	38,723	38,723
SK301-11304-2	Tower Assy.	1	42,750	42,750
SK301-11302-1	Hoist Module	2	2,470	4,940
SK301-11302-2	Davit Mount	1	6,207	6,207
SK301-11302-3	Module Support	2	6,274	12,548
SK301-11302-4	Platform Assy.	1	831	831
SK301-11302-6	Platform Assy.	1	831	831
SK301-11302-5	Beam Assy.	2	8,081	16,162
Aux.Hoist - 6,000-Lb Load Lifter		1	3,365	3,365
Mast (Jib Crane)		1	3,965	3,965
X73-003-AS-Y03	Control Room Enclosure	1	15,000	15,000
			TOTAL	145,322

## HOIST MODULE INSTALLATION

Each module was hoisted over the side of the ITR work platform from a truckbed using the overhead davit and a short-reach, adjustable-cg sling, ST40972. The side and module handrails were removed at each module location. Hoisting clearance between the jib crane hook and the work platform kick plates was 9 feet 4 inches. (Note: Safety harnesses were worn by personnel on the ITR platform; hard hats below.) Figure 29 shows both modules enplacements. Each module was secured with four bolts, each torqued in place to AISC requirements. The hoist enclosure handrails were used to support a tarpaulin for protection against the weather.

"Open loop" operation of each hoist was required to accomplish installation of the cargo coupling and attachment of the signal conductor cable. Test instrumentation and lash-up of control room equipment followed.

The following cargo system components were removable from the ITR overhead work platform without requiring operation of the system:

1. Hoist drive
2. Pneumatic valves and ducting
3. Signal conductor reel
4. Hoist
5. Span actuator
6. Rails (with hoist removed)

The following components were removable, but they required operation of the PPG and control system:

1. Hoist cables
2. Cable cutter assemblies
3. Signal conductor disconnect

## PPG AND AIR SUPPLY INSTALLATION

The air supply installation consisted of the pneumatic power generator (power unit, load compressor and controls) prepared as a separate package and the air distribution system. The PPG location and air distribution insulated piping are shown in Figures 30 through 35.

## INSTRUMENTATION INSTALLATIONS

The LCC station and test rig instrumentation installations are shown in Figures 36 through 41.



Figure 29. Hoist System Installed in ITR.

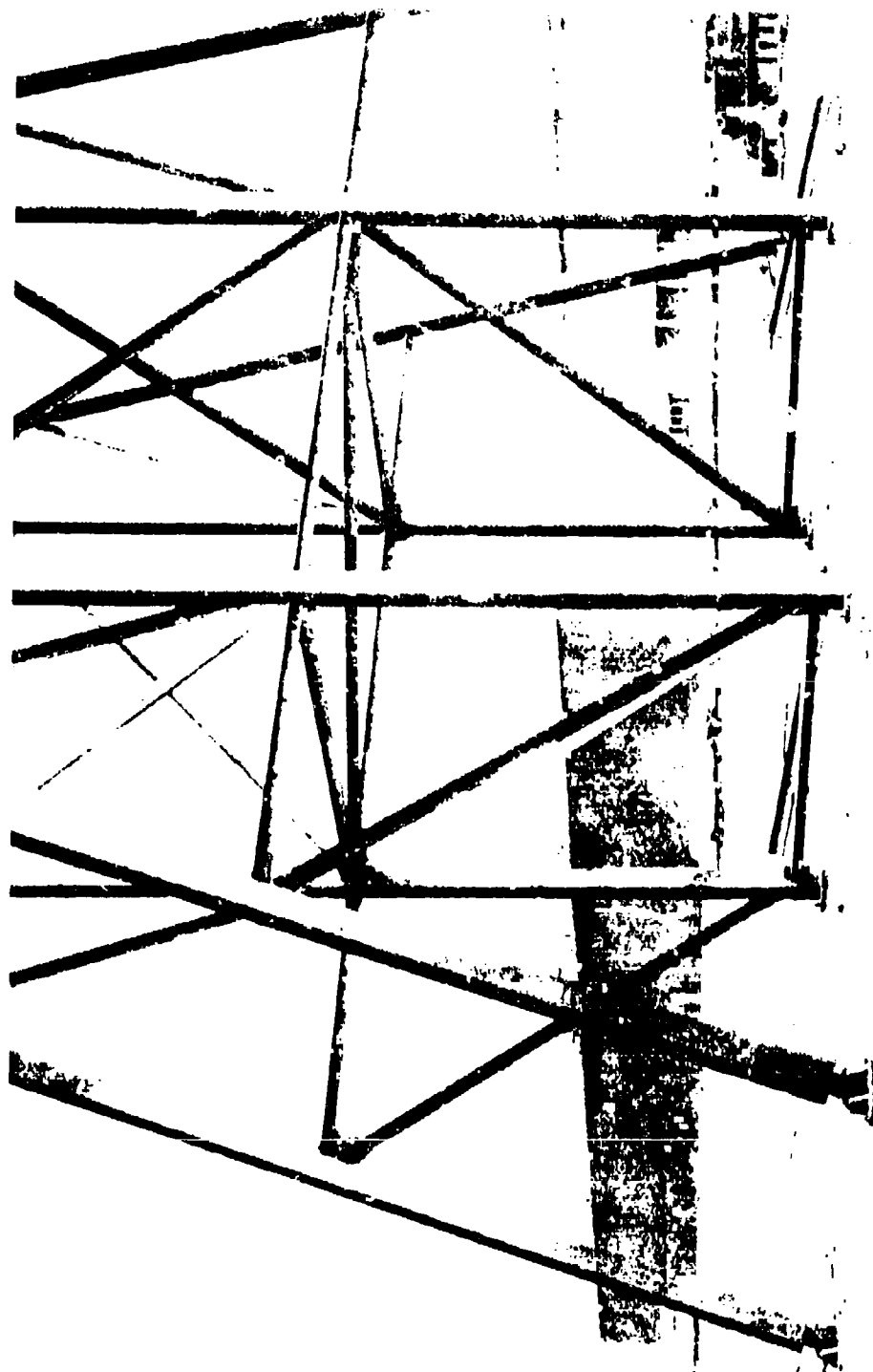


Figure 31. 200 Pad at Aft Tower.

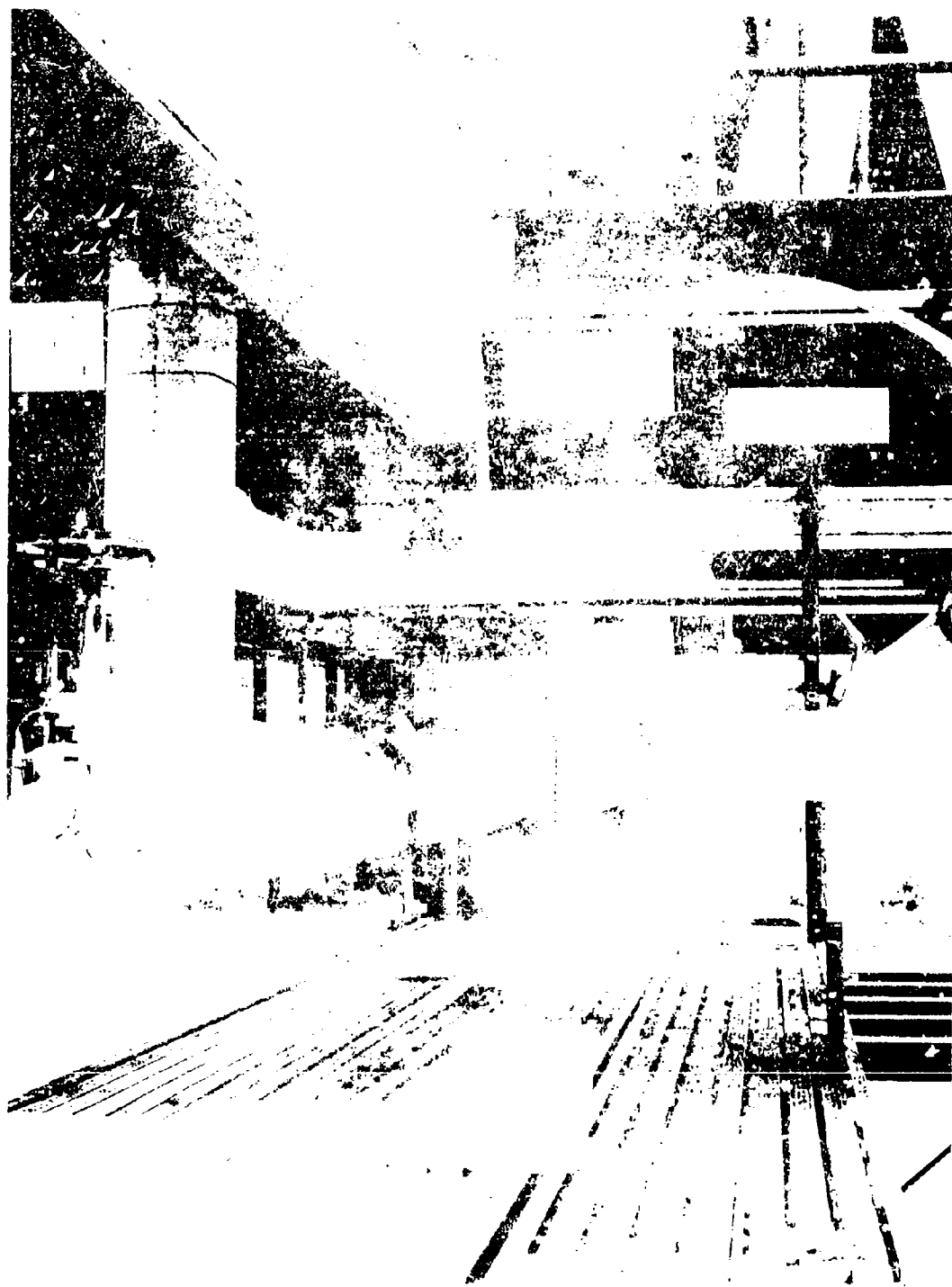


FIGURE 21. A view of the ship's deck and an air supply.

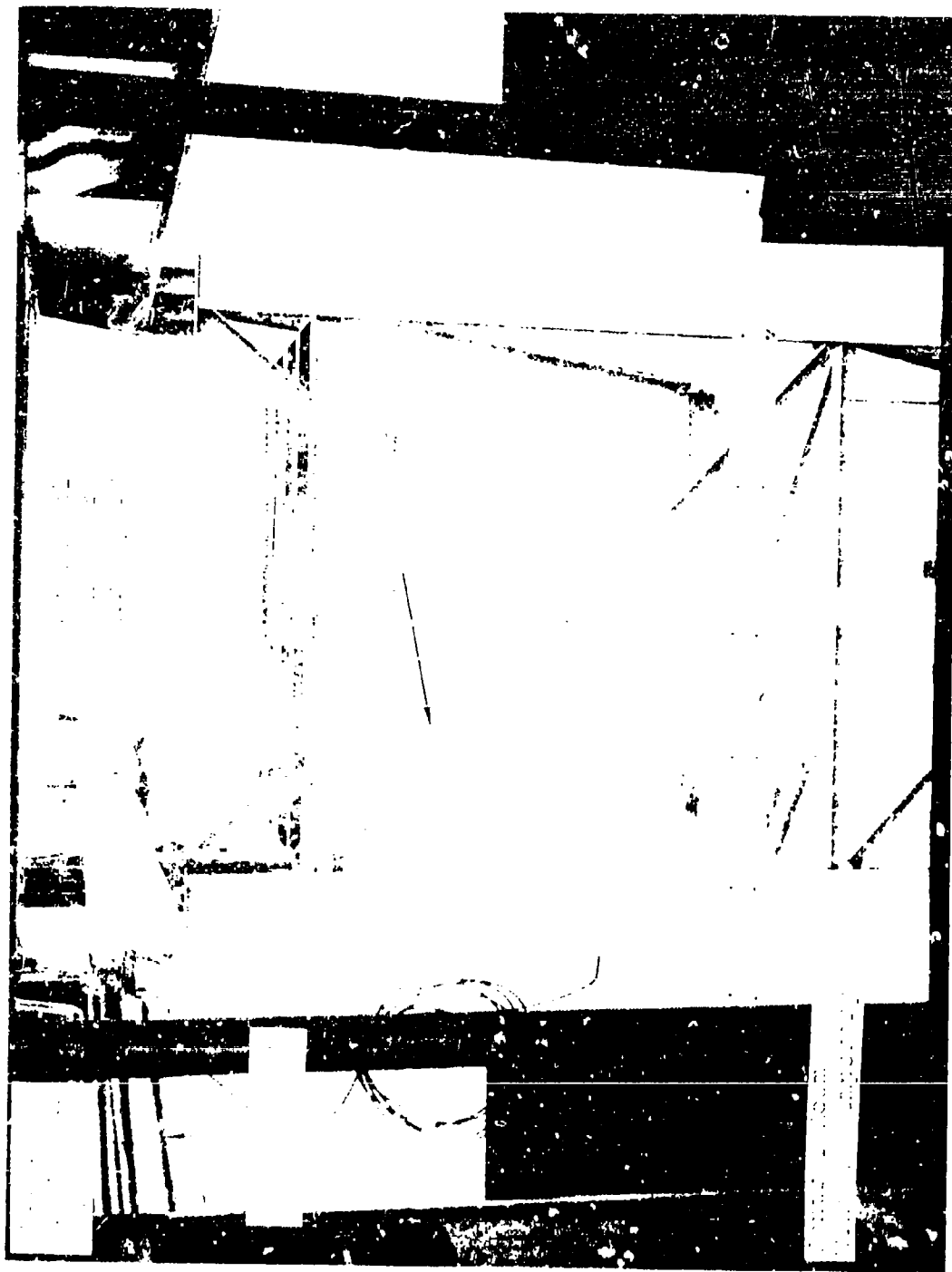


Figure 32. Insulated Air Header and Riser.

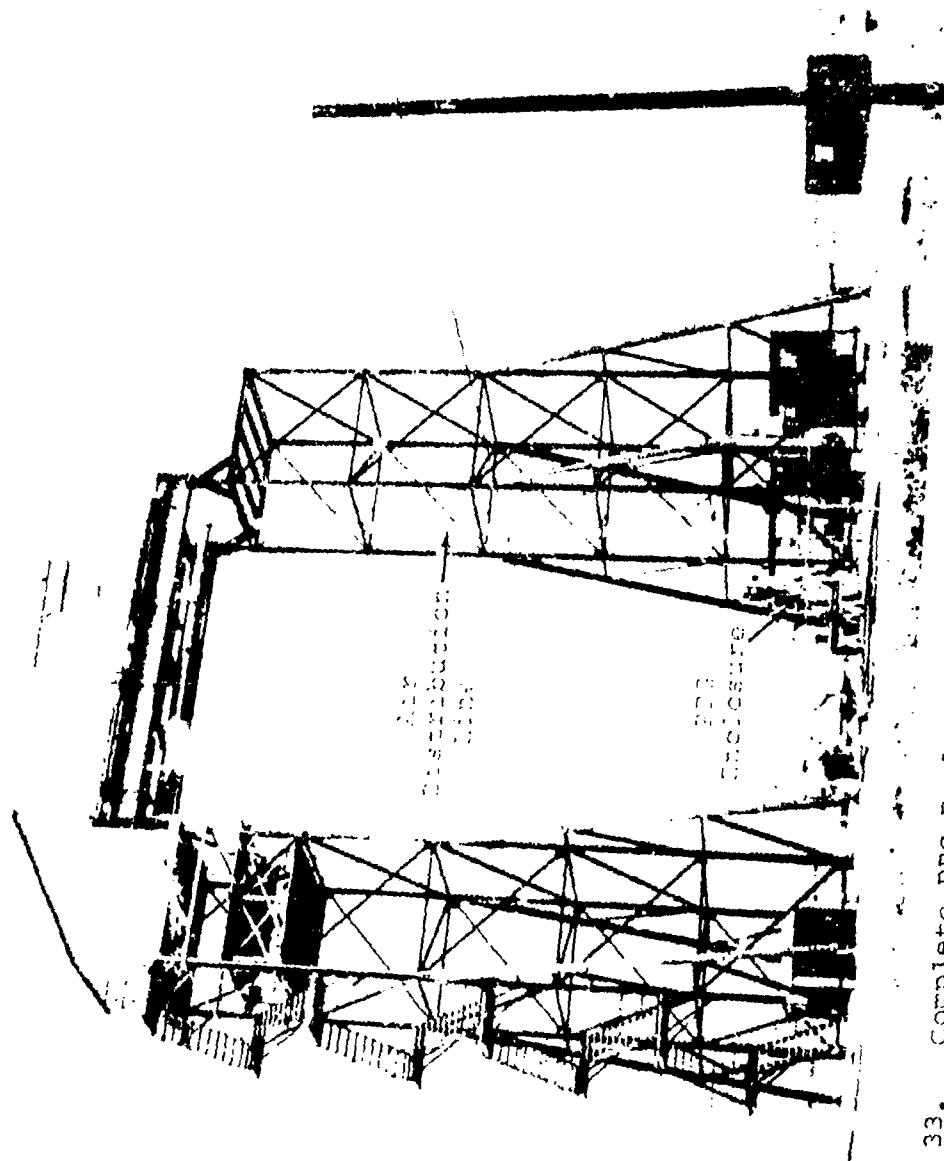


Figure 33. Complete PPG Enclosure and Air Distribution Line.

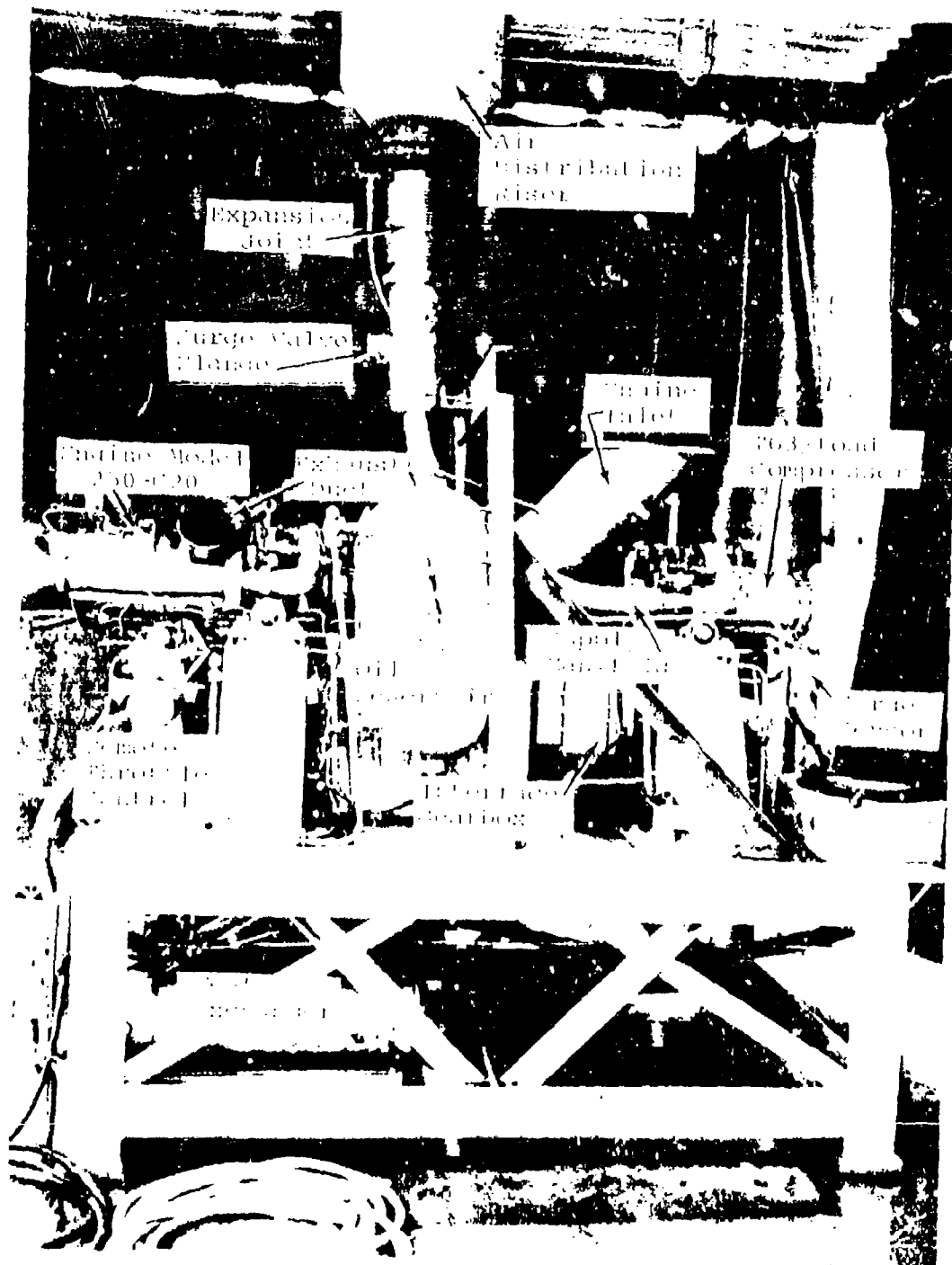


Figure 34. Side View of PPG.





Figure 10. Pilot in the cockpit of the aircraft.

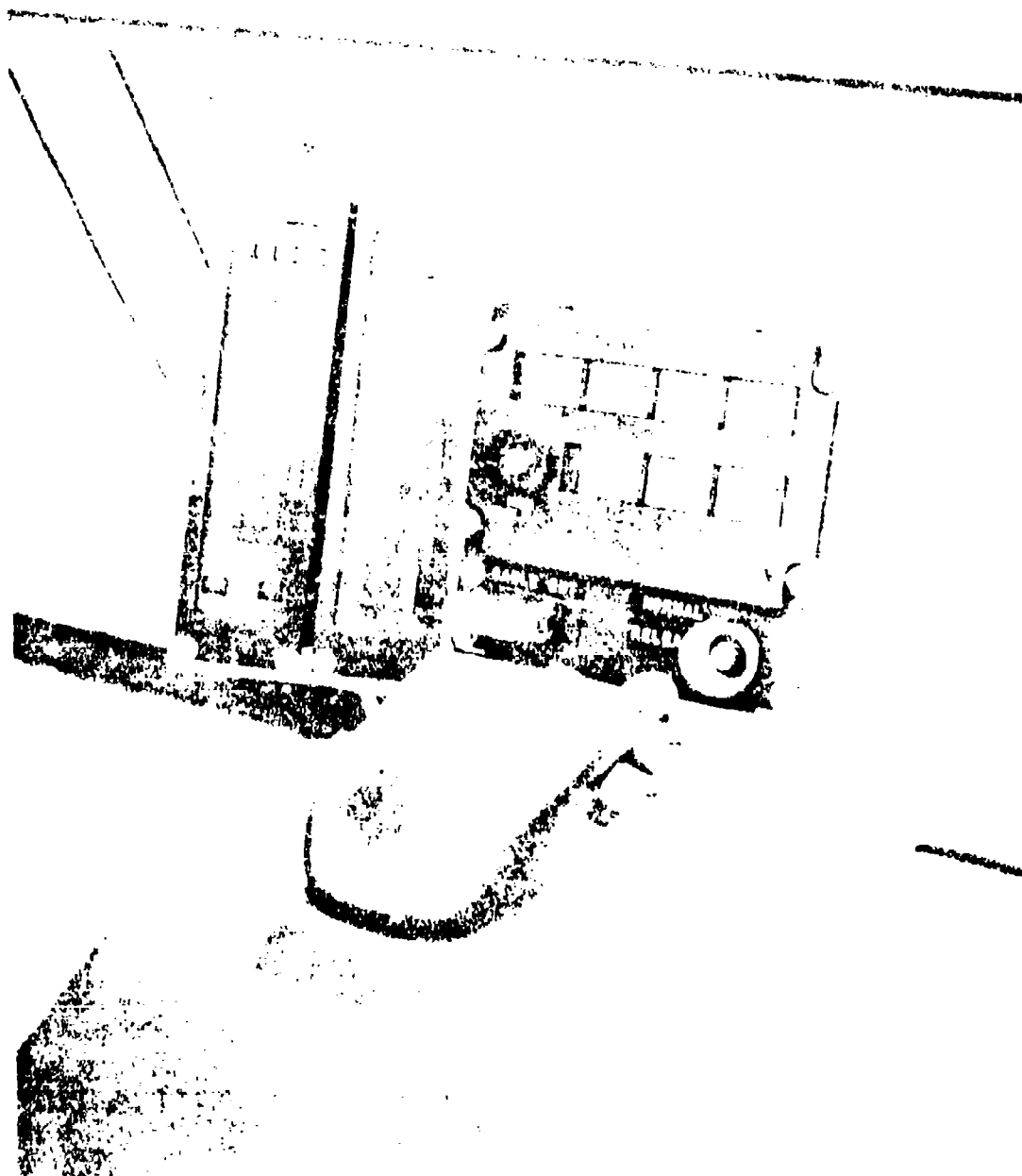




Figure 37. Instrumentation, DPC and MCC Station.

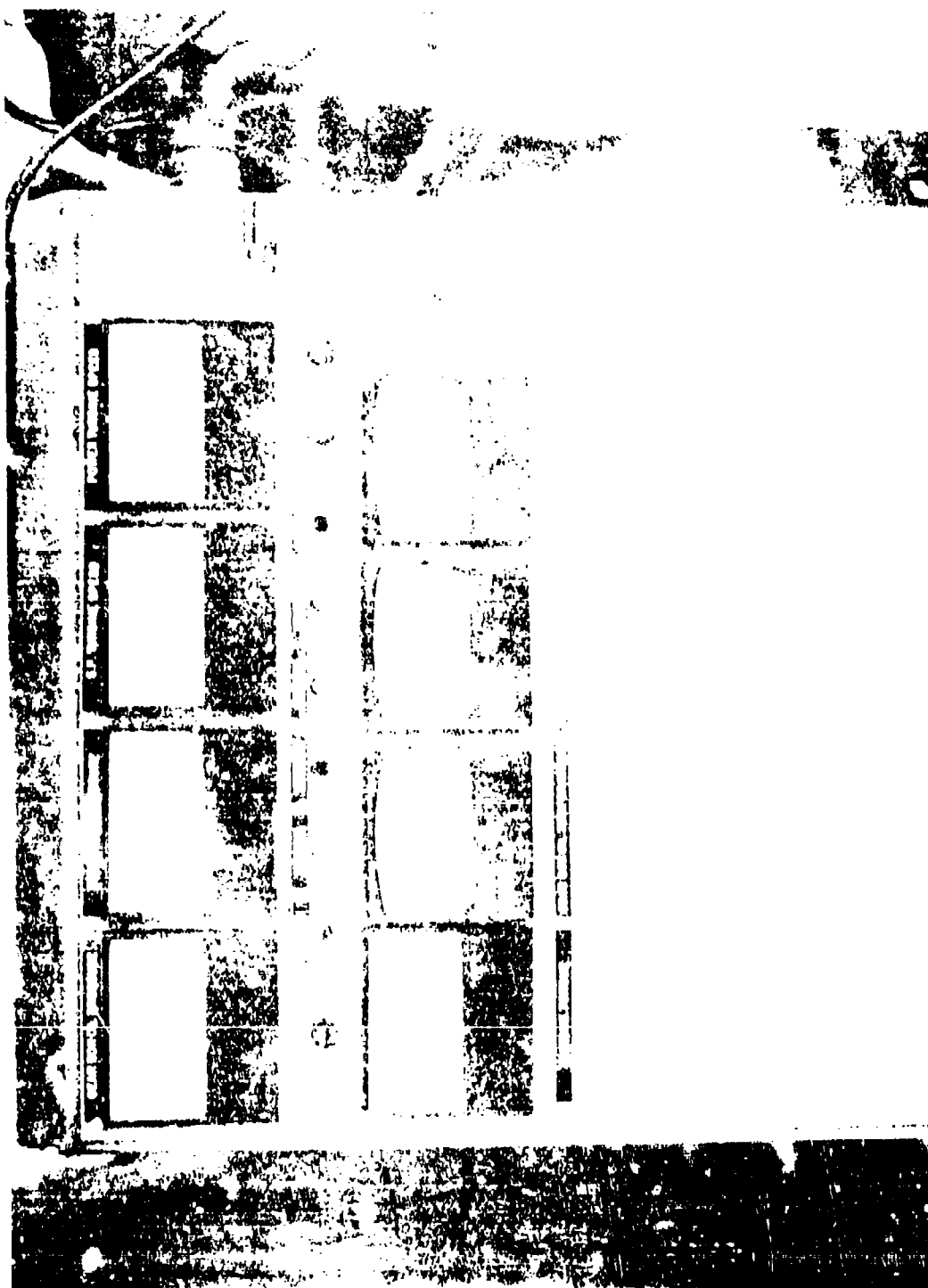








Figure 41. Static Load Test Instrumentation.

### PROOF LOADING

Handling fixtures and the utility hoist were proof-loaded in accordance with the following schedule:

<u>Fixture</u>	<u>Rated Load, lb</u>	<u>Proof Load, lb</u>
Hoist sling ST-51273	2,000	4,000
Hoist and module sling ST-40972	6,000	12,000
Utility hoist-load lifter Series 700 and jib crane (davit) model no.990740	6,000	9,000

### SPECIMEN INSTALLATION AND REMOVAL

The cargo system demonstration hardware was initially installed in the individual hoist modules at ground level. Only small hand tools and a sling were required. The modules were elevated on 4-foot workstands to provide access below the assembly. The module buildup started with the span positioning track and drive. The installation of the hoists (with cable) followed, with addition of the equalizer bar, angle and payout sensors, load isolators, hoist drives, pneumatic ducting, and system wiring. Figure 42 shows the hoist/module assembly before emplacement in the ITR.

### USE OF INTEGRATED TEST RIG

The integrated test rig was used to perform the following general categories of cargo handling system tests:

1. Functional check of components
2. Control system studies (open and closed loop)
3. System performance
  - a. No load - speed
  - b. Constant load - speed
  - c. Stall torque (live loads)
4. Static loads to maximum (ground tiedown)
5. Individual hoist operation
6. Endurance
7. Asymmetric load
8. PPC regulator valve development
9. General operating indoctrination
10. Dynamic analysis (high-speed photography)
11. Load acquisition demonstrations using a container handling device

The PPC starting and operating procedures used are outlined in Appendix III. The instrumentation calibration procedure used is given in Appendix IV.



• ROYAL CANADIAN MOUNTED POLICE •  
• OFFICER •



Test operations were performed under the prevailing weather conditions for the Philadelphia area during the months of October through April, which included rain, freezing temperatures, snow, hail, and winds up to 35 miles per hour with higher gusts.

Typical events in the ITR program are illustrated in the following figures.

Figure 43 shows a view of the control room during a typical operation. One man is monitoring the instrumentation, one is monitoring the PPG control panel, and the third is operating the hoists from the simulated LCC station.

Figure 44 shows a view of the cargo couplings from the LCC station. The forward coupling, which is nearest the viewer, is in the stowed position. The aft coupling is in the full-up position.

Figure 45 shows a view over the LCC operator's left shoulder during synchronous hoisting of two separate kirkstone block loads.

Figure 46 shows a U.S. Army MILVAN container being hoisted using the GFE helicopter-transported container handling device.

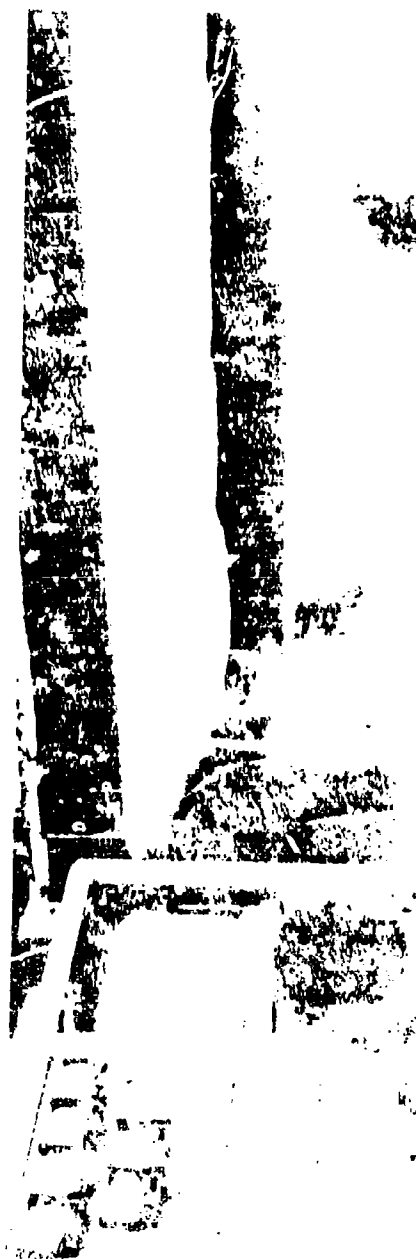
Figure 47 shows a kirkstone block load being hoisted using the single-point adapter and sheave system, which is designed to large-load vertical lifts of up to 35 tons.

Figure 48 shows the drive-through capability of the aft test rig tower. A MILVAN container on a standard high-bay flatbed trailer is being positioned for hoisting.

#### CONCLUSIONS AND RECOMMENDATIONS

The integrated test rig was a viable fixture for the cargo handling ATC program from the viewpoints of compliance with contractual design objectives, design simplicity, fabrication, erection, system installation, and instrumentation.

Uses of the integrated test rig could be expanded to include maintenance training if work platforms were added beneath the overhead structure to provide access to the hoist assemblies similar to what will exist in the actual helicopter. Improvements are needed in the PPG system; to increase its air supply capacity and equipment reliability.



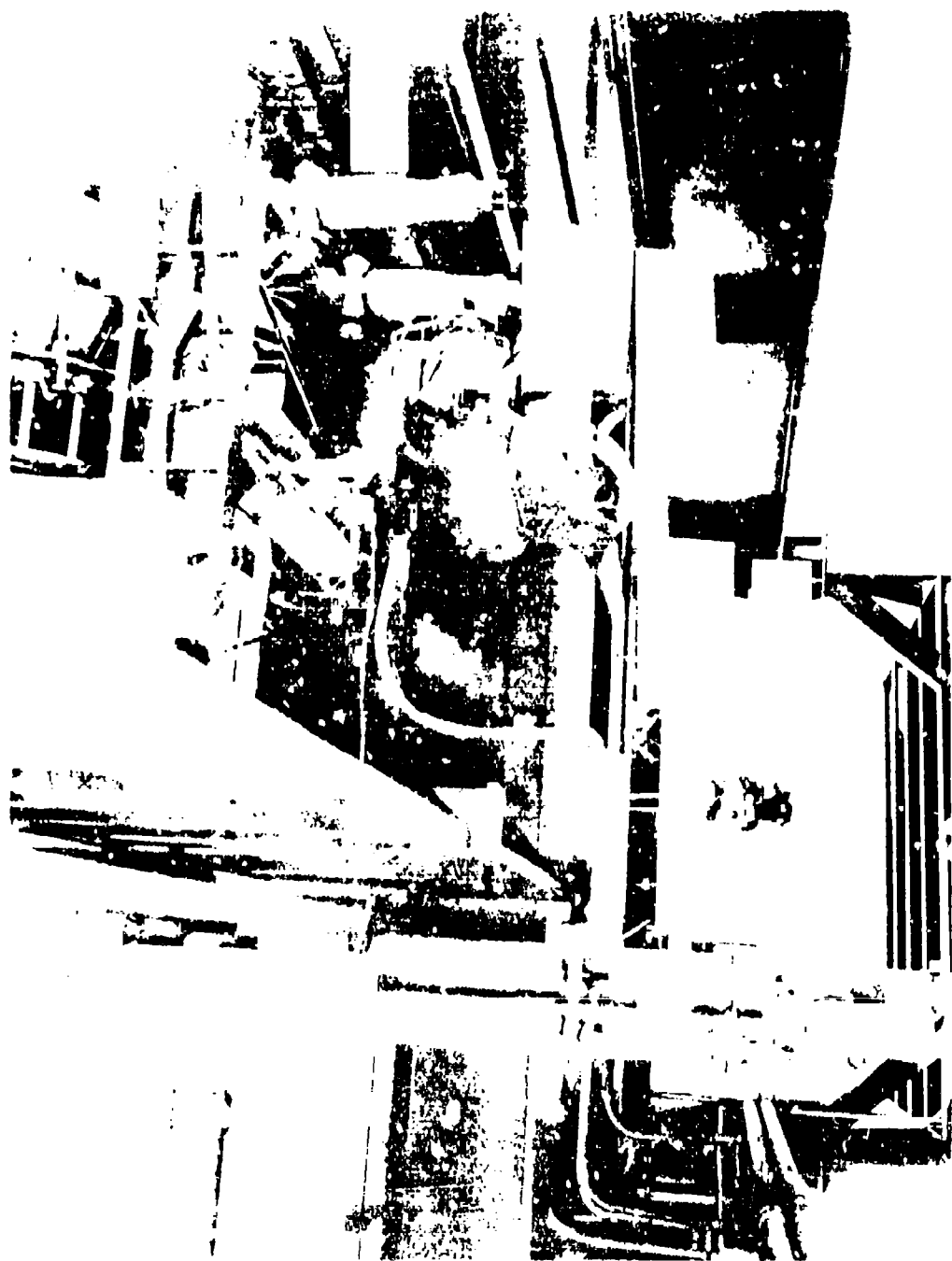




Figure 45. View of Two Individual Cables Near Top of Lift Cycle.

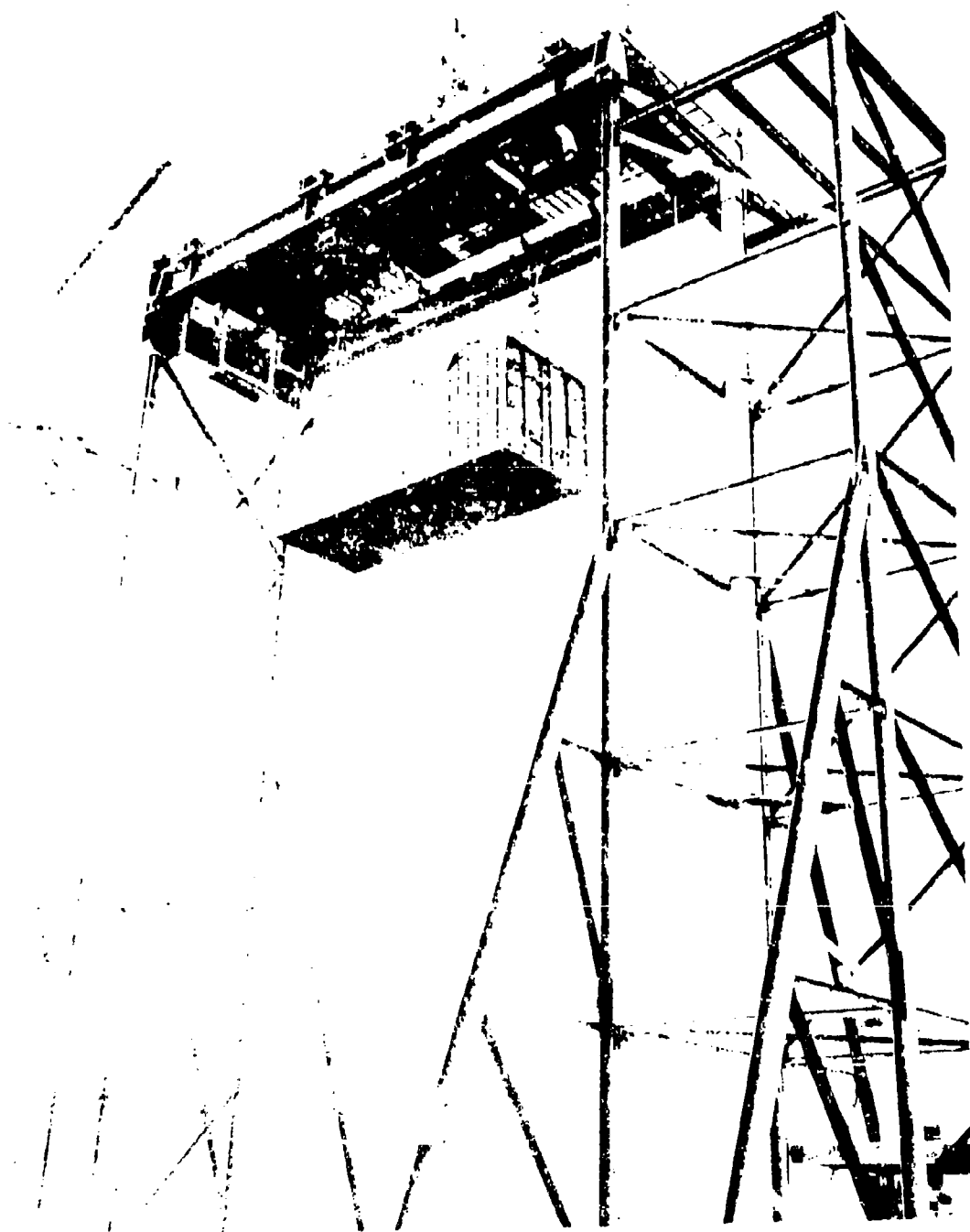


Figure 4b. 20' x 6' Miller Weld Tower. MILLVAN WITH CONTAINER  
BEING LOWERED INTO POOL 1011.

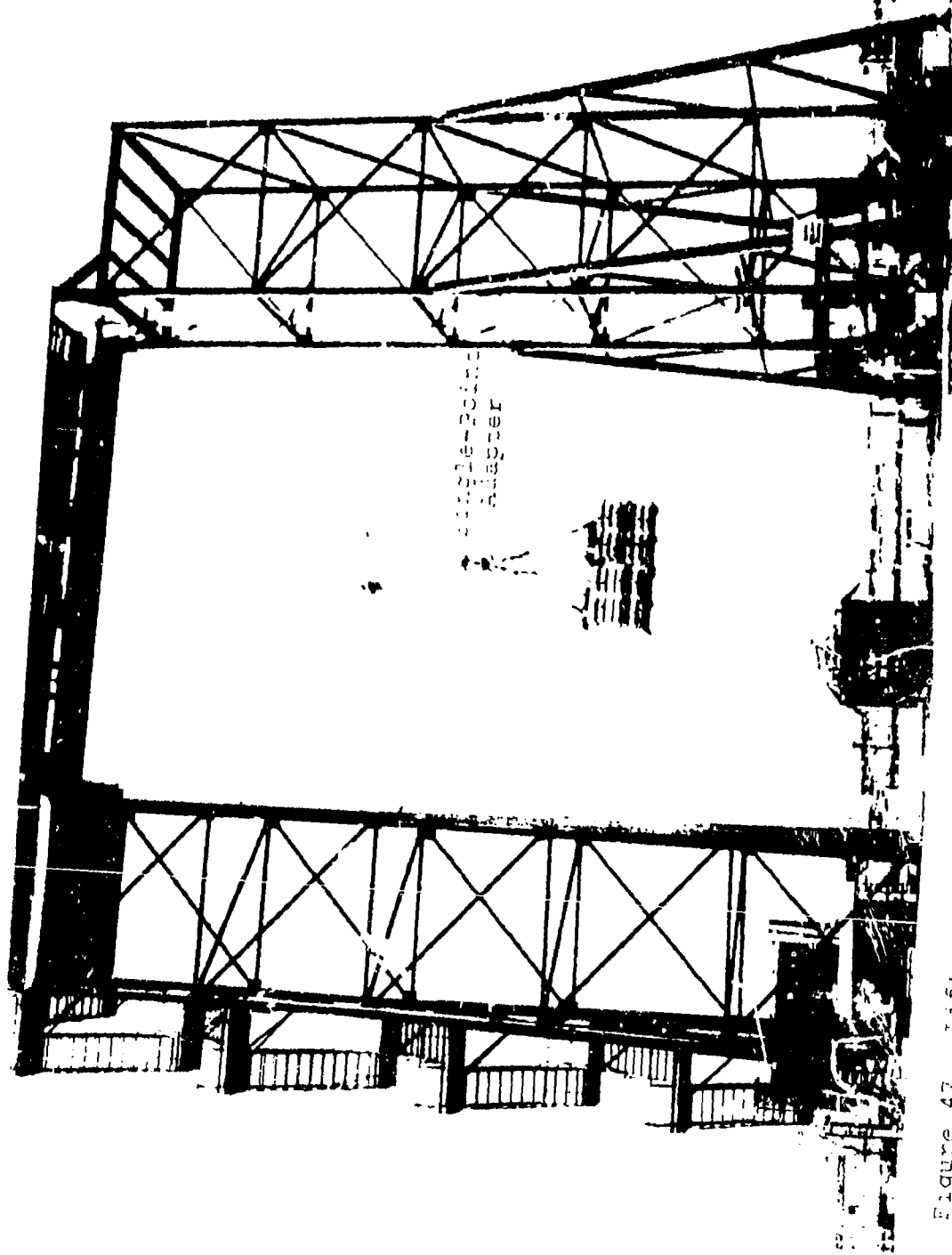


Figure 47. Lifting a 29-Ton Load With Single-point Adapter - Hoist Spar  
W = 16 Feet.

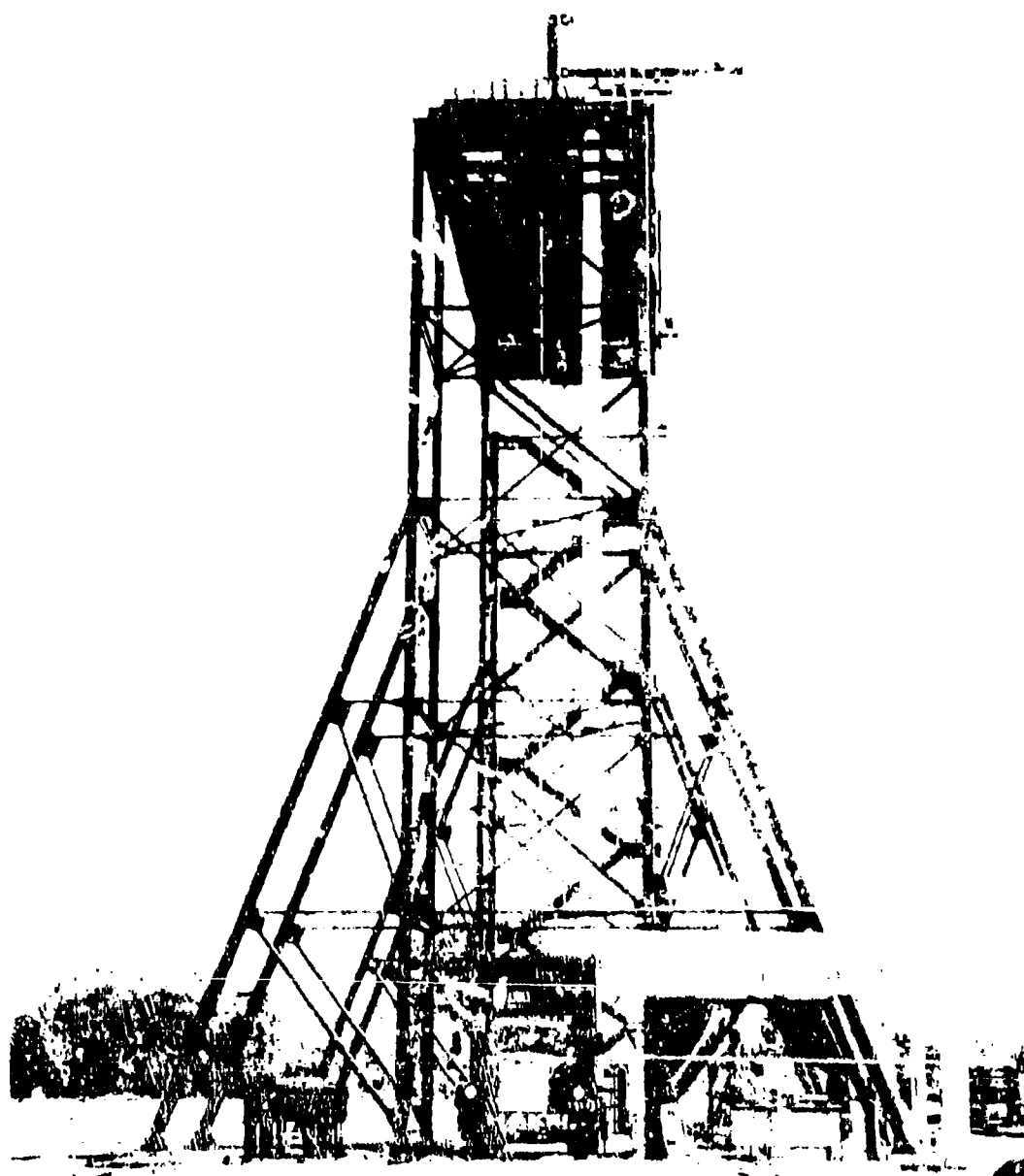


Figure 47. Entering MPMW through Base of Aft Tower.



APPENDIX 1  
INTEGRATED TEST RIG STRESS ANALYSIS

TABLE OF CONTENTS

	<u>Page</u>
LIST OF SYMBOLS . . . . .	80
SUMMARY . . . . .	81
STRESS ANALYSIS - OVERHEAD ASSEMBLIES . . . . .	83
Hoist Module Assembly. . . . .	83
Auxiliary Hoist Mount. . . . .	87
Hoist Module Support Assembly. . . . .	89
Work Platform Assembly . . . . .	91
Main Fore-and-Aft Beams. . . . .	92
STRESS ANALYSIS - TOWER ASSEMBLIES. . . . .	102
Fore-Aft Frame - Member Loads. . . . .	103
Inboard-Outboard Frame - Member Loads. . . . .	113
Column and Strut Splice. . . . .	120
Outrigger Tiedown Assembly . . . . .	121
FAILURE LOADS . . . . .	124
FOUNDATION LOADS. . . . .	128
DEAD WEIGHTS . . . . .	132
CONTROL ROOM SHELTER. . . . .	133

### LIST OF SYMBOLS

A	Cross Section Area - In. <sup>2</sup>
c	Distance from Neutral Axis to Extreme Fiber of Beam
d	Depth of Beam or Girder - Inches
E	Modulus of Elasticity of Steel - KSI
e	Load Eccentricity - Inches
F <sub>A</sub>	Allowable Stress for Compression Members - KSI
f <sub>a</sub>	Computed Axial Stress - KSI
f <sub>b</sub>	Computed Bending Stress - KSI
F <sub>CR</sub>	Failure Stress - KSI
f <sub>v</sub>	Computed Shear Stress - KSI
I	Moment of Inertia of Section - In. <sup>4</sup>
J	Polar Moment of Inertia - In. <sup>4</sup>
L	Span Length - Inches
l	Actual Unbraced Length - In.
M	Moment - In. KIPS
M.S.	Margin of Safety
P	Applied Load - KIPS
q	Shear Flow - lb./In.
R	Reaction - KIPS
r	Governing Radius of Gyration - In.
S <sub>s</sub>	Section Modulus - In. <sup>3</sup>
T	Torsional Moment - In. K
V	Static Shear on Beam - KIPS
Δ	Deflection - Inches
τ	Torsional Shear Stress - KSI

APPENDIX I  
INTEGRATED TEST RIG STRESS ANALYSIS

SUMMARY

This appendix is presented to substantiate the strength of towers and overhead steel structure designed for use during testing of cargo hoists of the heavy lift helicopter. It is intended that the data contained herein be used as a basis for establishing the structural integrity of the structure for load changes during HHH testing and/or for tests other than the HHH tests.

A-36 type steel was used throughout since it is common for the steel industry and affords an economical design. American Institute of Steel Construction, Inc., Allowable Stresses and Design Practices, as printed in the sixth edition of their manual, were used. ASTM A325 bolts were used for all connections.

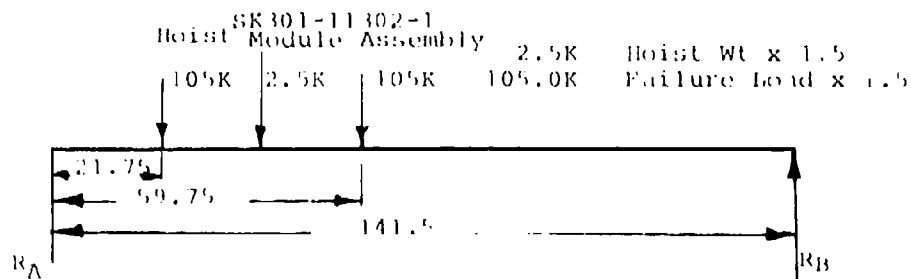
Strength requirements were based upon cargo system loads: 28-ton, design; 70-ton, limit. Rig survival - no collapse - is one of the provisions in the design in the event of a sling or other failure during testing. The towers were designed using 1.5 factors of maximum test load, or failure loads, with a horizontal load resulting from a 5° component in both cases. All horizontal load components were considered reversible. Stairways, walkways, and work platforms were designed to meet the requirements of the Occupational Safety and Health Standards (Code of Federal Regulations, Title 29, Chapter XVII, Part 1910).

Because of height requirements of the towers and the relative low design loads, a trussed frame configuration was selected. Requirements for a control room in a location near the hoists so that the hoist operator could be located in the same relative position as he would be in the helicopter made the selection of a 14-foot-square tower desirable. Easy installation of stairways on this type of tower was also a factor in the selection of this configuration. Outriggers were used on each leg of the tower in the inboard-outboard plane for two reasons:

1. The possibility of an enclosure being erected over the top of the towers which would result in considerable wind loading.
2. The requirement that the secondary bracing be removed in the lower portions of the tower to permit a loaded truck to drive through the tower.

Outriggers in the fore-aft plane were not required because the two conditions stated above are not factors in this plane. Also, horizontal components of test loads are reacted by four frames in the fore-aft plane, while in the inboard-outboard direction only the two inner frames will react the horizontal component of loading.

The tower assemblies as delivered by the fabricator were not of the configuration as intended in the original design. This optional construction was analyzed and found to have adequate strength for the requirements of the hoist test tower loads. The reaction loads on the foundation are not as evenly distributed in the optional construction as in the original design; however, since the foundations are resting on bedrock, the optional configuration is sufficient for all test requirements. The configuration as delivered is referred to in the stress analysis as the optional construction configuration.



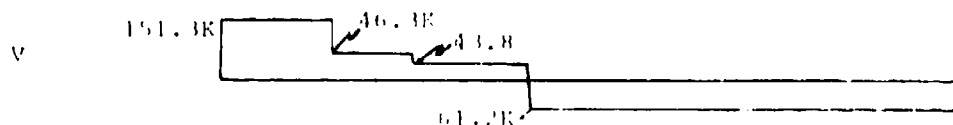
Failure Condition  
70 tons on (1) hoist

$$\sum M_{R_A} = 105(21.75) + 105(59.75) + 2.5(141.5) - R_B(141.5)$$

$$2280 + 6280 + 354 = 141.5 R_B$$

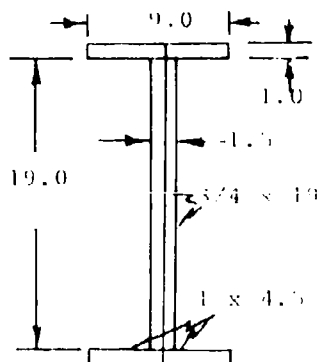
$$R_B = \frac{8661}{141.5} = 61.2K$$

$$R_A = 61.2 + 105 + 105 + 2.5 = 151.3K$$



$$M_{MAX} = 81.75(61.2) = 5000 \text{ in}\cdot\text{K}$$

$$S_{Req'd} = \frac{5000 \text{ in}\cdot\text{K}}{\frac{20}{30} F_{BI}} = 250 \text{ in}^3$$



$$(3) 3/4 \times 19 = 428.7(3) = 1286.1 \text{ in}^4$$

$$(4) 1 \times 4.5 = 200 \times 9 = 1800.0 \text{ in}^4$$

$$2657.4 \text{ in}^4$$

$$S = \frac{I}{c} = \frac{2657.4}{10} = 265.74 \text{ in}^3$$

$$f_b = \frac{5000 \text{ in}\cdot\text{K}}{265.74 \text{ in}^3} = 18.85 \text{ KSI Bending Stress}$$

Max Test Load = 1/2 Failure Condition

$$f_b = 9.4 \text{ KSI Bending Stress}$$

SK301-11302-1

Cross-Sectional Area at Point of Max Shear

$$A = (2)11(.75) = 16.50 \text{ in.}^2$$

$$f_s = \frac{V}{A} = \frac{151.3}{16.5} = 9.17 \text{ KSI Shear Stress}$$

$$M.S. = \frac{14.5}{9.17} - 1 = .58$$

Long. Shear  $f_s = \frac{VQ}{Ib}$  at weld (4) 3/8 welds

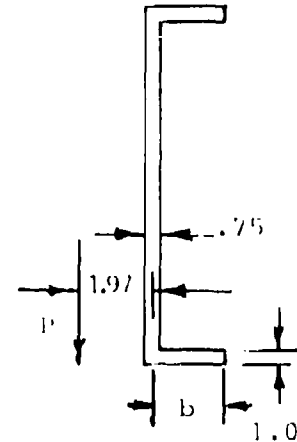
$$Q = 9(10) = 90 \text{ in.}^4$$

$$f = \frac{151.3(90)}{2657(1.5)} = 3.42 \text{ KSI Long. Shear}$$

SK301-11302-1  
Torsional Shear

Element	b	t	bt <sup>3</sup>
Flange	4.125	1.0	4.125
Flange	4.125	1.0	4.125
Web	20.0	.75	8.43
		$\Sigma bt^3 =$	16.680

$$J = \frac{2bt^3}{3} = \frac{16.68}{3} = 5.56$$



$$\text{Torsion} = \frac{(1.97)(52.5K)}{1.622} = 1.35(52.5) = 70.8 \text{ lb. K}$$

Torsional Shear Stress

$$\tau = \frac{Tt}{J} = \frac{70.8(.75)}{5.56} = 9.55 \text{ KSI}$$

$$P = \frac{F.S.}{4} = \frac{70T(1.5)}{4} = 52.5K$$

M.S. - Margin of safety based on allowable design stress  
(22 KSI tension, 14.5 KSI shear)

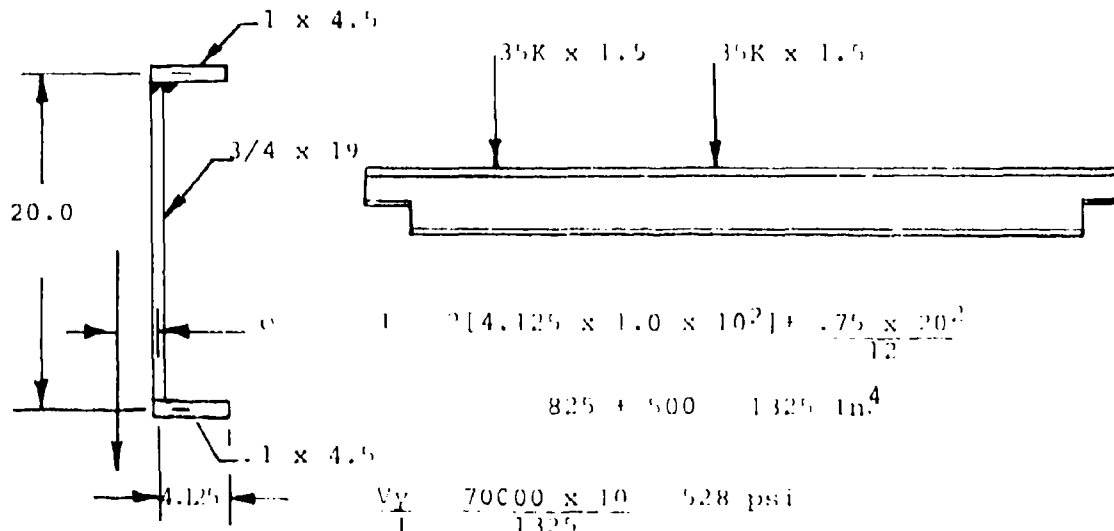
Bending

$$M.S. = \frac{F_a}{f_a} - 1 = \frac{22}{18.85/1.5} - 1 = .75 \text{ (F.S.)}$$

Shear

$$\frac{14.5}{9.55/1.5} - 1 = 1.28$$

SK301-11302-1  
Shear Center



Flange  $q = 528(1.0 \times x)$  when  $x=0$   $q=0$   
 $x = 4.125$   $q = 2175 \text{ lb/in. at corner}$

Web  $q = 2175 + 528(10) \frac{.75}{2} = .667y (.75y)$

$$2175 + 1982 = .250y^2$$

$$4157 = .250y^2$$

$$2175(20) + 1982(20) .667 = 70,000$$

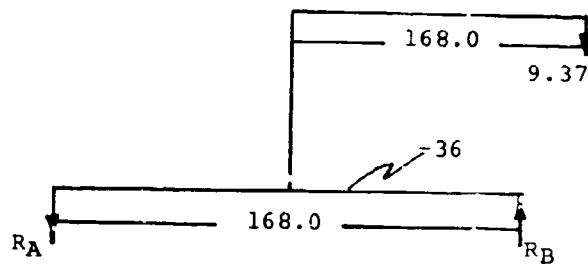
$$70,000 = 4.12(528)20 = 43500$$

$$e = \frac{43500}{70000} = .622$$



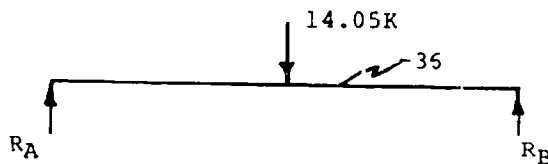
SK301-11302-2 Assembly  
Auxiliary Hoist Mount

DJ Wt Hoist 3.37  
 Max. Load 6.00  
 9.37K



$$-R_A = R_B = \frac{14.05(168)}{168} = 14.08K \text{ (2 beams)}$$

7.64K/beam

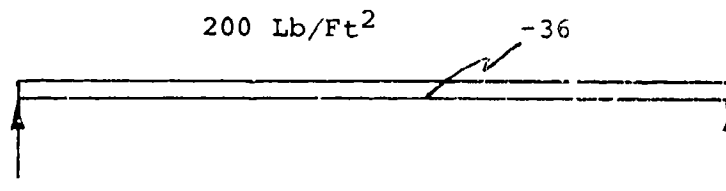


$$R_A = R_B = \frac{P}{2} = \frac{14.05}{2} = 7.02K$$

$$\begin{aligned} \text{Total Moment} &= 14.05 (168) + 7.02 (84) - 2 \text{ beams} \\ &= 2360 + 592 \\ &= 2952 \text{ In K} - 2 \text{ beams} \\ &= 1476 \text{ In K/beam} \end{aligned}$$

$$f_b = \frac{M}{S_x} = \frac{1476}{121.1} = 12.2 \text{ KSI} - 14 \text{ W } 78$$

SK301-11302-2



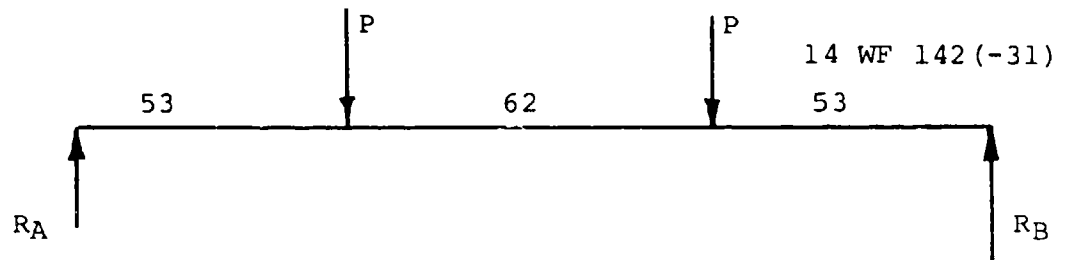
$$M = \frac{wl^2}{8} = \frac{.05(168)^2}{8} = 176 \text{ In. K}$$

$$f_b = \frac{M}{S_x} = \frac{176}{121.1} = 1.45 \text{ KSI}$$

Total Stress - Uniform Load + Hcist Load

$$f_b = 12.2 + 1.45 = 13.65 \text{ KSI}$$

SK301-11302-3  
Hoist Module Support Assembly

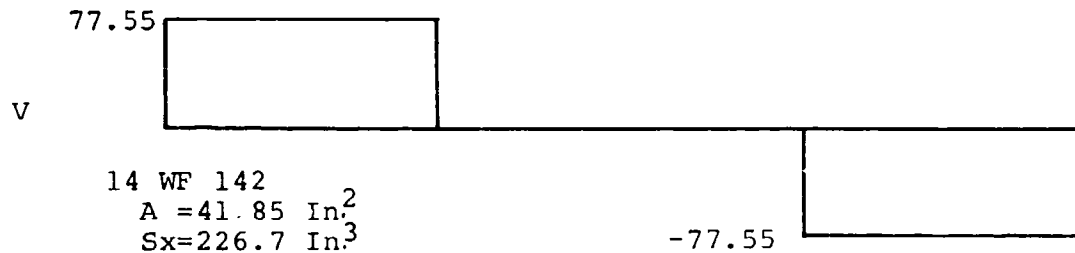


Failure Condition

$$P = \frac{151.3}{2} + \frac{DD \text{ Wt}}{2} =$$

$$= 75.65 + \frac{2.5(100.75)}{141.5} = 75.65 + 1.9 = 77.55$$

$$R_A = R_B = 77.55$$



$$f_s = \frac{P}{A} = \frac{77.55}{41.85} = 86 \text{ KSI Shear Stress}$$

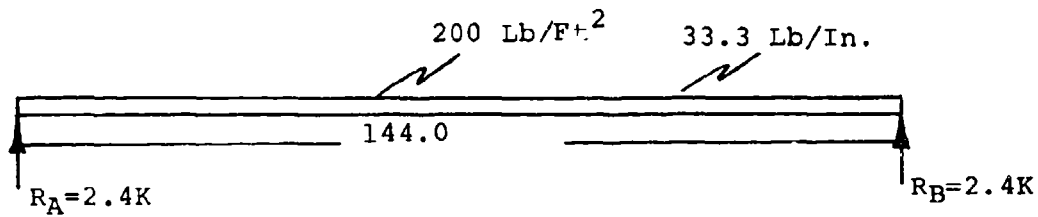
$$M_{MAX} = 77.55(53) = 4120 \text{ In} \cdot \text{K}$$

$$f_b = \frac{M}{S_x} = \frac{4120}{226.7} = 18.15 \text{ KSI}$$

Max. Test Load = 1/2 Failure Condition

$$f_b = 9.1 \text{ KSI}$$

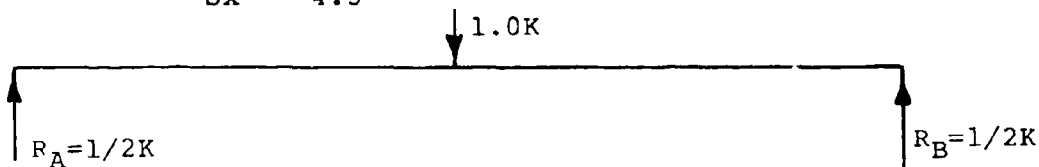
SK301-11302-3



$$-32 \quad 6[8.2 \text{ Lb} \quad S_x = 4.3 \text{ In.}^3$$

$$M_{MAX} = \frac{wl^2}{8} = \frac{33.3(144)^2}{8} = 86.3 \text{ In. K}$$

$$f_b = \frac{M}{S_x} = \frac{86.3}{4.3} = 20.07 \text{ KSI}$$



$$M_{MAX} = \frac{Pl}{4} = \frac{1.000(144)}{4} = 36 \text{ In. K}$$

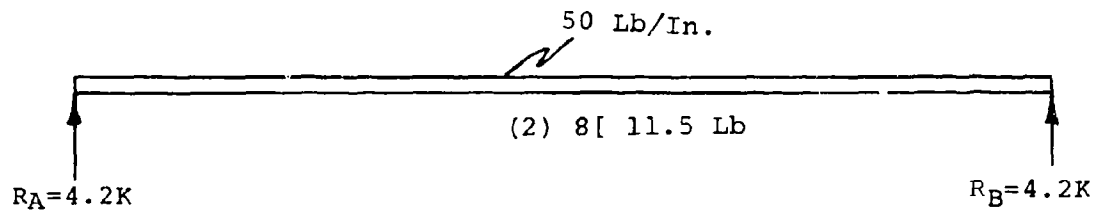
$$f_b = \frac{M}{S_x} = \frac{36}{4.3} = 8.38 \text{ KSI}$$

Grip Strut Flooring Capacity (members tack welded together)

Concentrated load	1100 Lb.
Uniformly dist. load	353 Lb/Ft <sup>2</sup>

SK301-11302-4  
Work Platform Assembly

Design load 200 Lb/Ft.



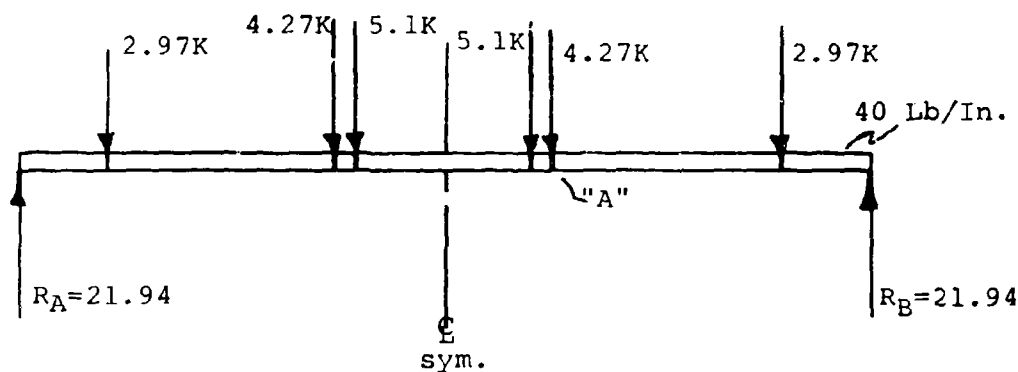
$$M_{MAX} = \frac{w l^2}{8} = \frac{.05 (168)^2}{8} = 176.2 \text{ In. K}$$

$$f_b = \frac{M}{S_x} = \frac{176.2}{16.2} = 10.88 \text{ KSI}$$

Grip strut on this assembly has a capability of 235 Lb/Ft<sup>2</sup> of uniformly distributed load and a concentrated load of 633 Lb at the midpoint.

SK301-11302-5  
Main Fore & Aft Beam Assembly  
30WF172  
A = 50.72 In<sup>2</sup>  
S<sub>x</sub> = 528.2 In<sup>3</sup>

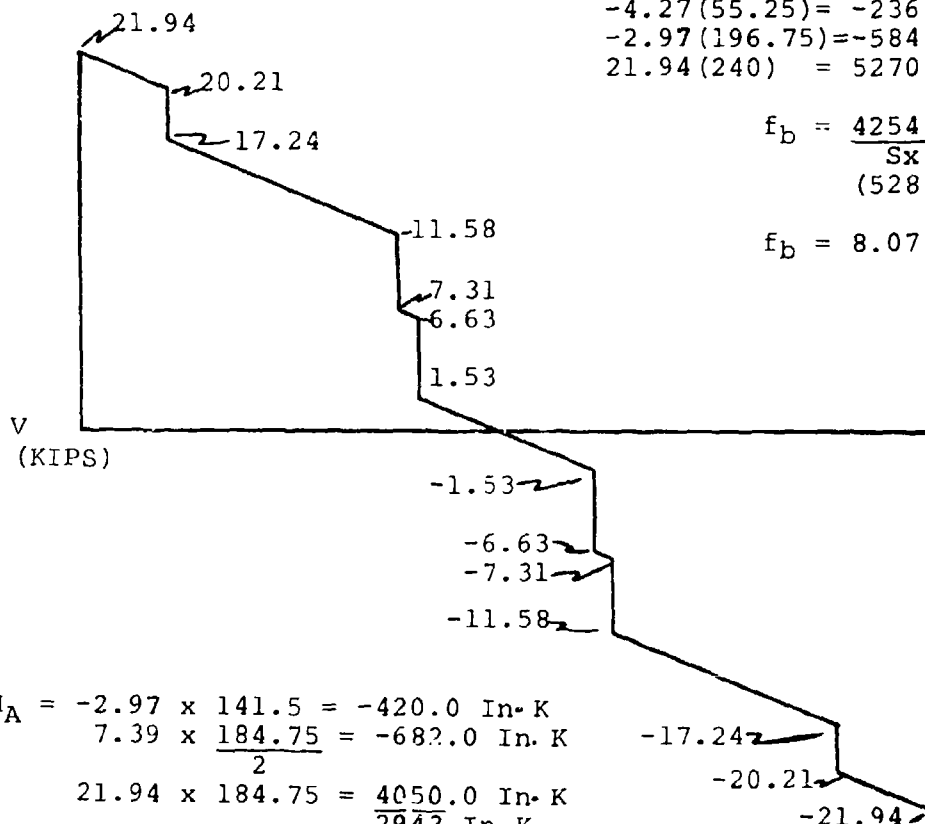
Dead Wt Only  
16' position



$$\begin{aligned} M_E &= -5.1(38.25) = -195.2 \\ &-4.27(55.25) = -236.0 \\ &-2.97(196.75) = -584.0 \\ &21.94(240) = 5270.0 \end{aligned}$$

$$f_b = \frac{4254.8 \text{ In} \cdot \text{K}}{S_x (528.2)}$$

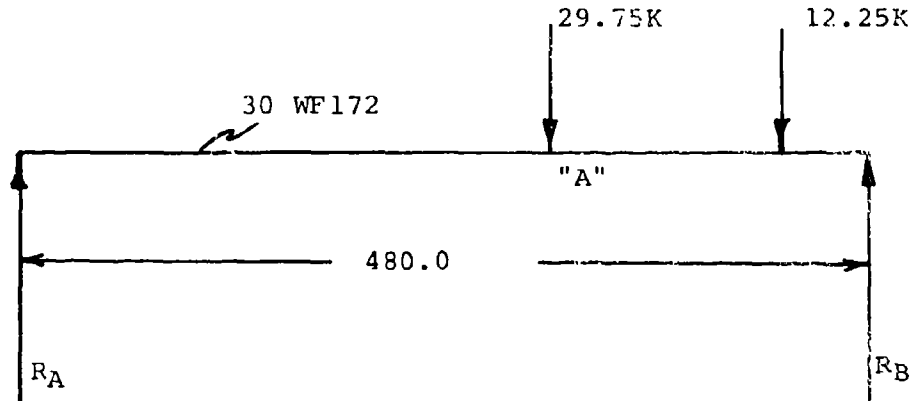
$$f_b = 8.07 \text{ KSI}$$



$$\begin{aligned} M_A &= -2.97 \times 141.5 = -420.0 \text{ In} \cdot \text{K} \\ &7.39 \times \frac{184.75}{2} = -682.0 \text{ In} \cdot \text{K} \\ &21.94 \times 184.75 = 4050.0 \text{ In} \cdot \text{K} \\ &\frac{2943}{S_x} \text{ In} \cdot \text{K} = 5.58 \text{ KSI} \\ &(528.2) \end{aligned}$$

SK301-11302-5  
30 WF172

Test load -  
Failure condition  
16' hoist position  
28-ton load @ 1.5 F.S.



$$\sum M_{R_A} = 29.75(295.25) + 12.25(436.75) - 30 R_B$$

$$R_B = \frac{8780 + 53.50}{480} = \frac{141.30}{480} = 29.5K$$

$$R_A = 12.5$$

$$M_A = -141.5(12.25) + 184.75(29.5) \\ -1732 + 5450 = 3718 \text{ In. K}$$

$$f = \frac{M_A}{S_x} = \frac{3718}{528.2} = 7.04 \text{ KSI Failure load}$$

	5.58 KSI DD Wt
	12.62 KSI

SK301-11302-5

30 WF17

Proof load

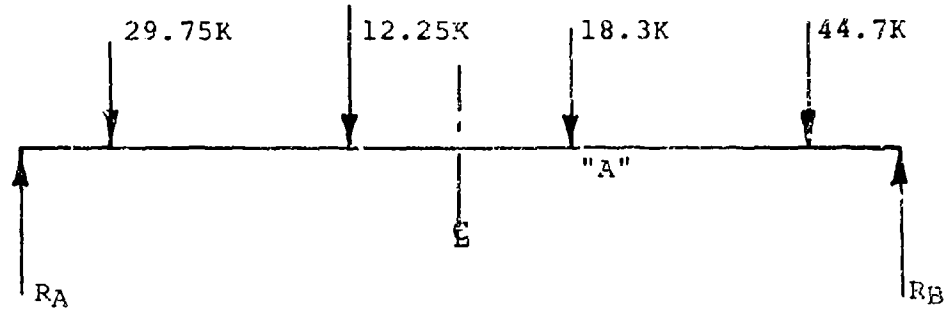
26' hoist position

70-ton load @ 1.5 F.S.

.60-.40 load split

40% proof load

60% proof load

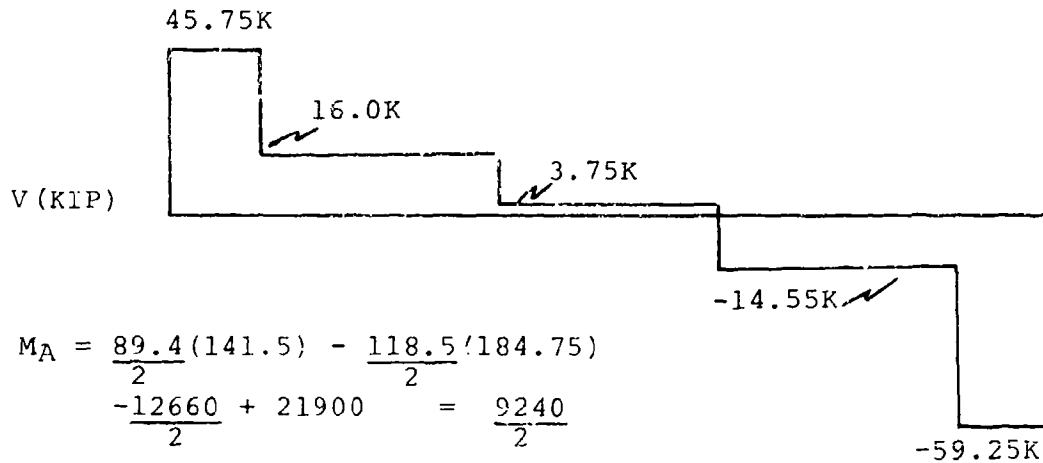


$$\sum M_{R_A} = 29.75(43.25) + 12.25(104.75) + 18.3(295.25) + 44.7(436.75)$$

$$\frac{2575}{2} + \frac{4530}{2} + \frac{10800}{2} + \frac{39000}{2} = -480R_B$$

$$R_B = \frac{2845.2}{480} = 59.25K$$

$$R_A = 45.75K$$



$$M_A = \frac{89.4(141.5)}{2} - \frac{118.5(184.75)}{2}$$

$$\frac{-12660}{2} + 21900 = \frac{9240}{2}$$

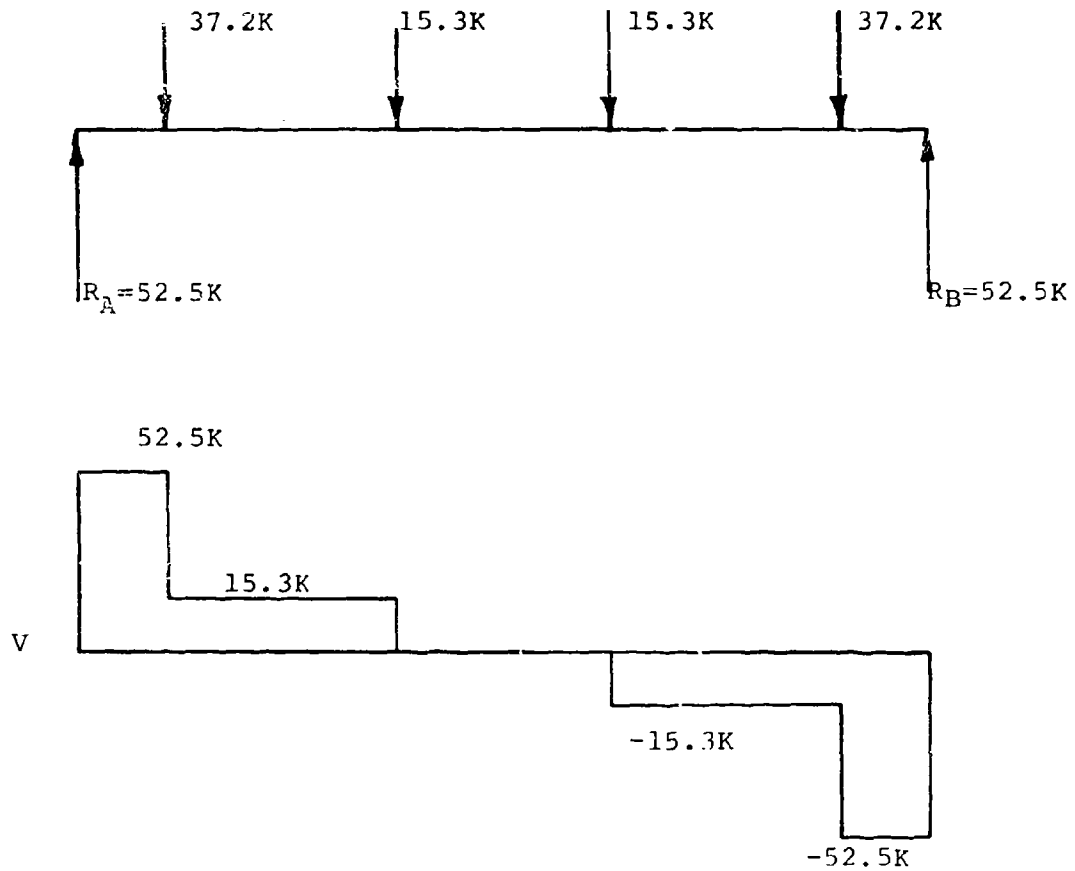
$$f_b = \frac{4620}{528.8} = 8.75 \text{ KSI Load}$$

$$\frac{5.58 \text{ KSI DD Wt}}{14.33 \text{ KSI}}$$



SK301-11302-5  
30 WF172

Proof load  
26' hoist position  
70-ton load @ 1.5 F.S.  
.50-.50 load split



$$M_{MAX} = -52.5(184.75) + 37.2(141.5)$$

$$= 9700 + 5270 = 4430 \text{ In.K}$$

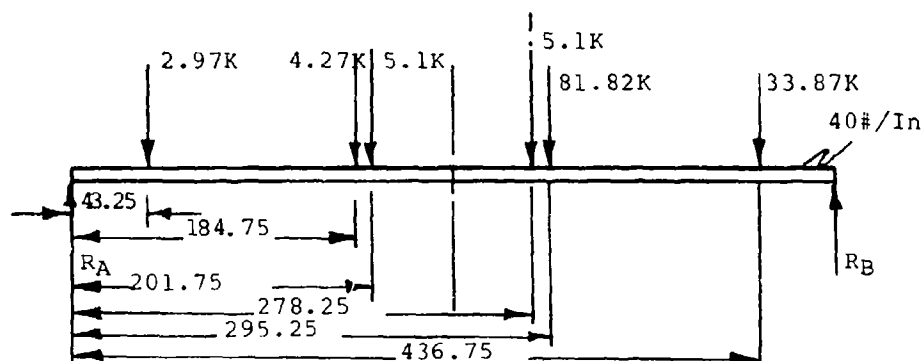
$$f_b = \frac{M_{MAX}}{S_x} = \frac{4430 \text{ In.K}}{528.2 \text{ In.}^3} = 8.75 \text{ KSI load}$$

$$\frac{8.07 \text{ KSI DD Wt}}{16.82 \text{ KSI}}$$

30 WF172  
 A = 50.72  
 Sx = 528.2 in<sup>3</sup>

SK301-11302-5

Failure Condition  
 at 16' position  
 70-ton proof load  
 (DD Wt included)

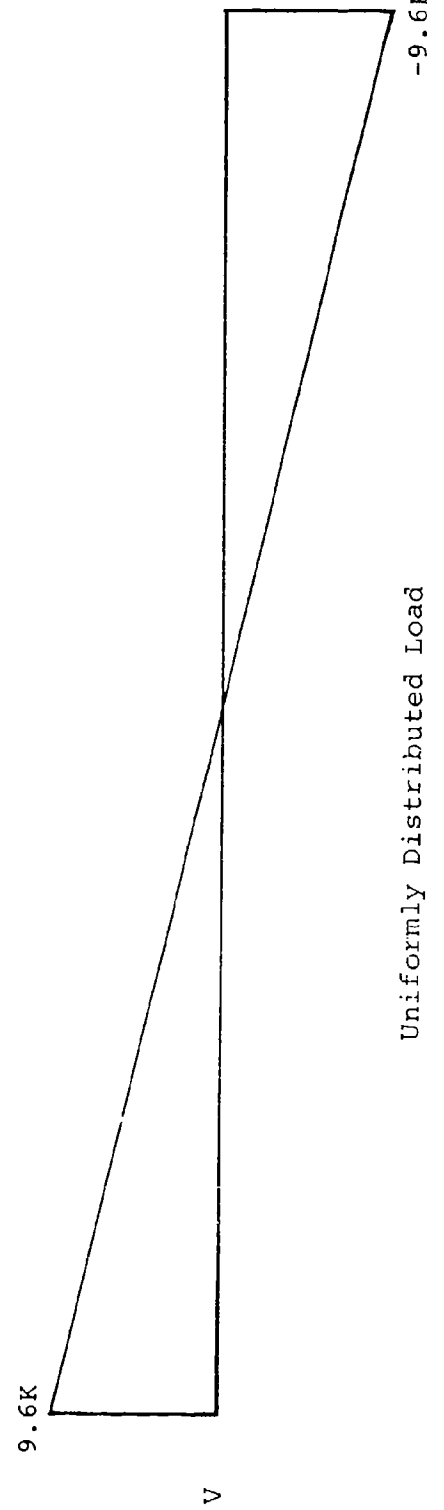
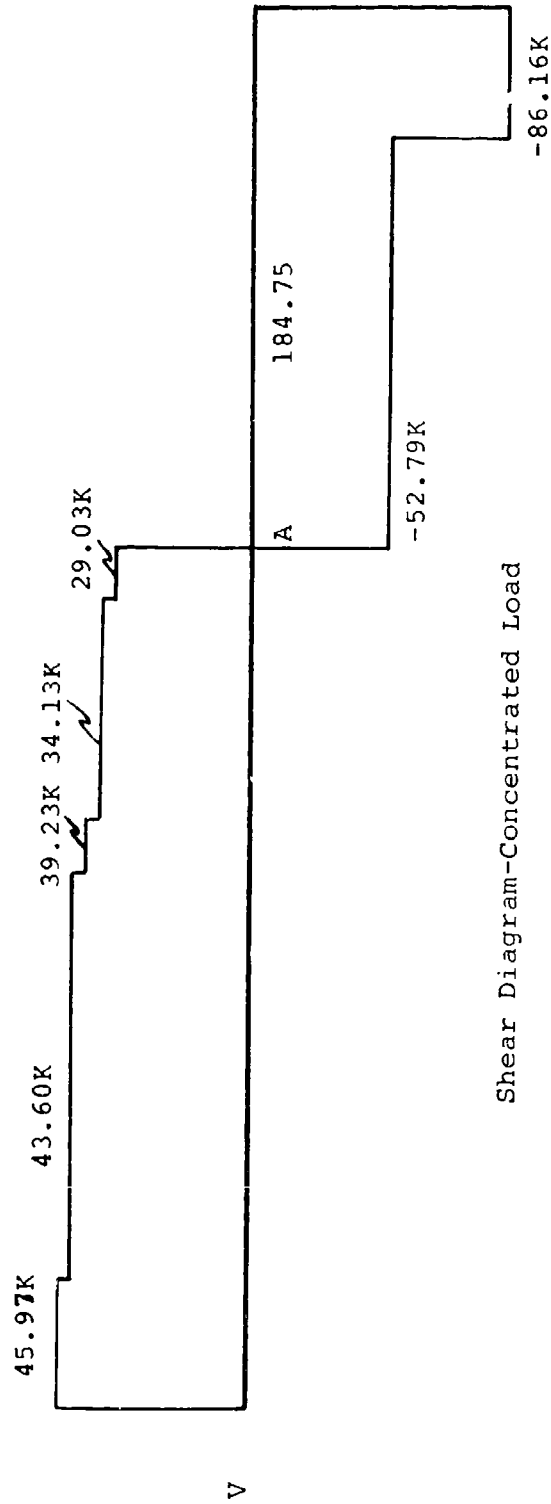


$$\begin{aligned}\sum M_{R_A} &= 19.20(240) + 2.97(43.25) + 4.27(184.75) + 5.1(201.75) + 5.1(278.25) \\ &\quad + 81.82(295.25) + 33.87(436.75) - 480R_B \\ &= 4608 + 128.45 + 788.88 + 1028.93 + 1419.08 + 24157.36 + 14792.72 \\ R_B &= \frac{46923.42}{480} = 97.76K\end{aligned}$$

$$\begin{aligned}\sum F_u &= +19.20 + 2.97 + 4.27 + 5.1 + 5.1 + 81.82 + 33.87 - 97.76 - R_A \\ R_A &= 54.57\end{aligned}$$

SK301-11302-5

Failure condition  
at 16' span  
70-ton proof load



30 WF172

SK301-11302-5

Failure condition  
at 16' span  
70-ton proof load

Moment is Max. at PT. "A"

Solving for Max. Mom.

$$M_A = -86.16(184.75) - 9.6(184.75) + 33.87(141.5) - 400.45$$

$$= 12498.6 \text{ In. K}$$

$$f_b = \frac{12498.6}{528.2} = 23.7 \text{ KSI}$$

30 WF172  
 $A = 50.72 \text{ in}^2$   
 $S_x = 528.2 \text{ in}^3$

SK301-11302-5

Failure condition  
 at 26' position  
 70-ton proof load  
 (DD Wt. included)

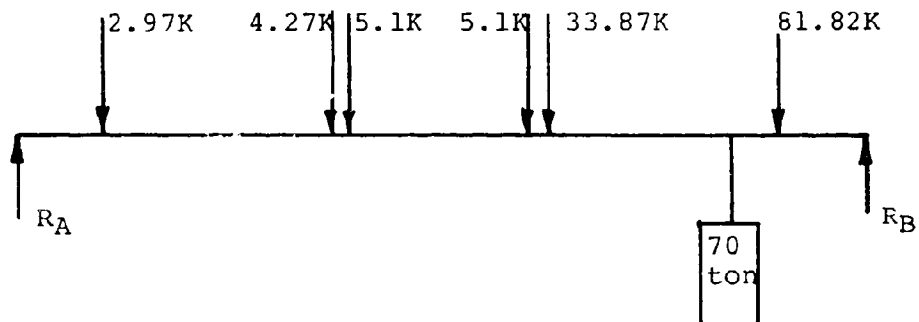


Figure A

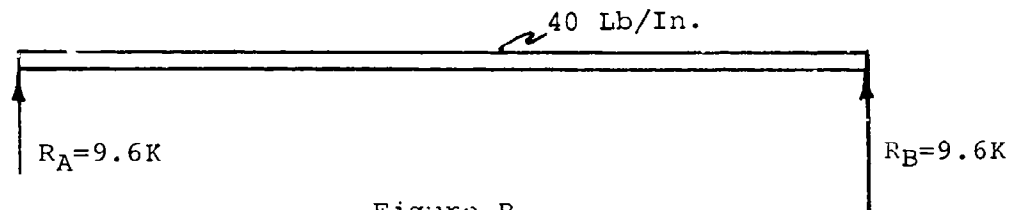


Figure B

Figure A

$$\sum M_{RA} = 128.45 + 788.88 + 1028.93 + 1419.08 + 33.87(295.25) \\ (10000.12)$$

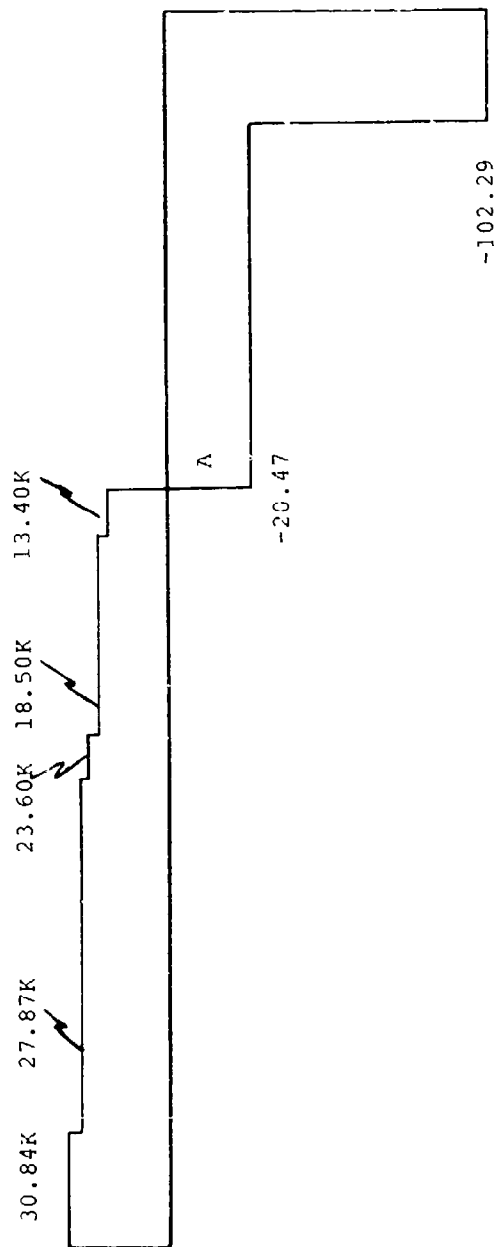
$$+ 81.82(436.75) - 480R_B \\ (35734.89)$$

$$R_B = \frac{49,100.34}{480} = 102.29K$$

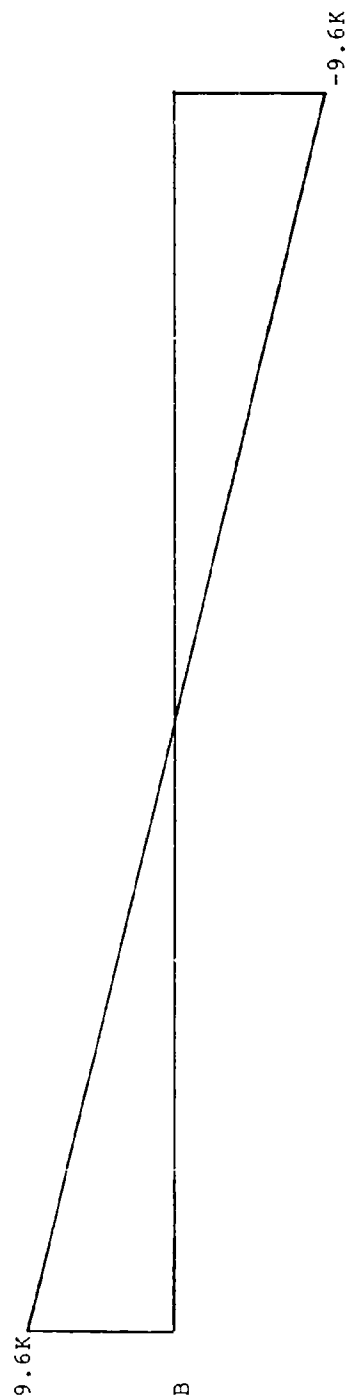
$$R_A = 30.84K$$

Shear and Moment Diagram on  
 Page 100.

SK301-11302-5 at 26' location  
70-ton proof load



V  
Fig.A



V  
Fig.B

SK301-11302-5

Failure condition  
at 26' position  
70-ton proof load

Moment is Max. at PT. "A"

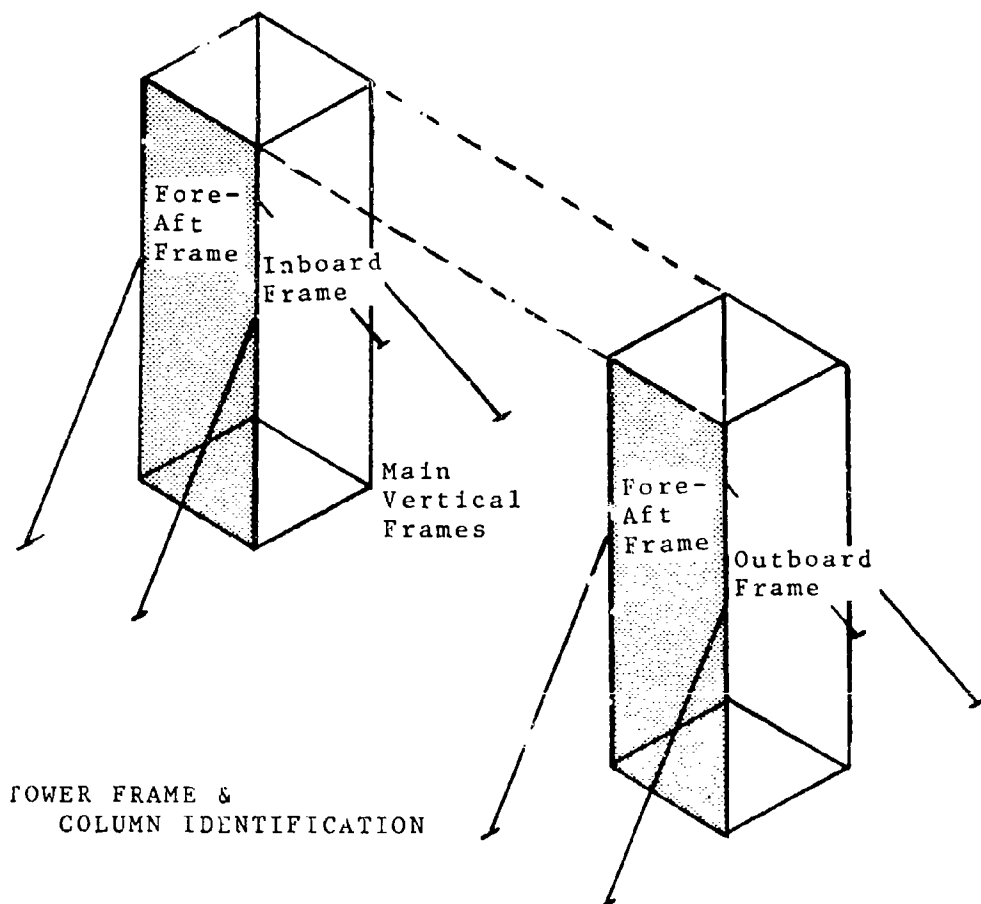
$$M_{MAX} = -102.29(184.75) - 9.6(184.75) + 81.82(141.5) + 400.45$$

$$= \frac{8693.72}{528.2} = 16.45 \text{ KSI}$$

SK301-11304-1  
Tower Assembly

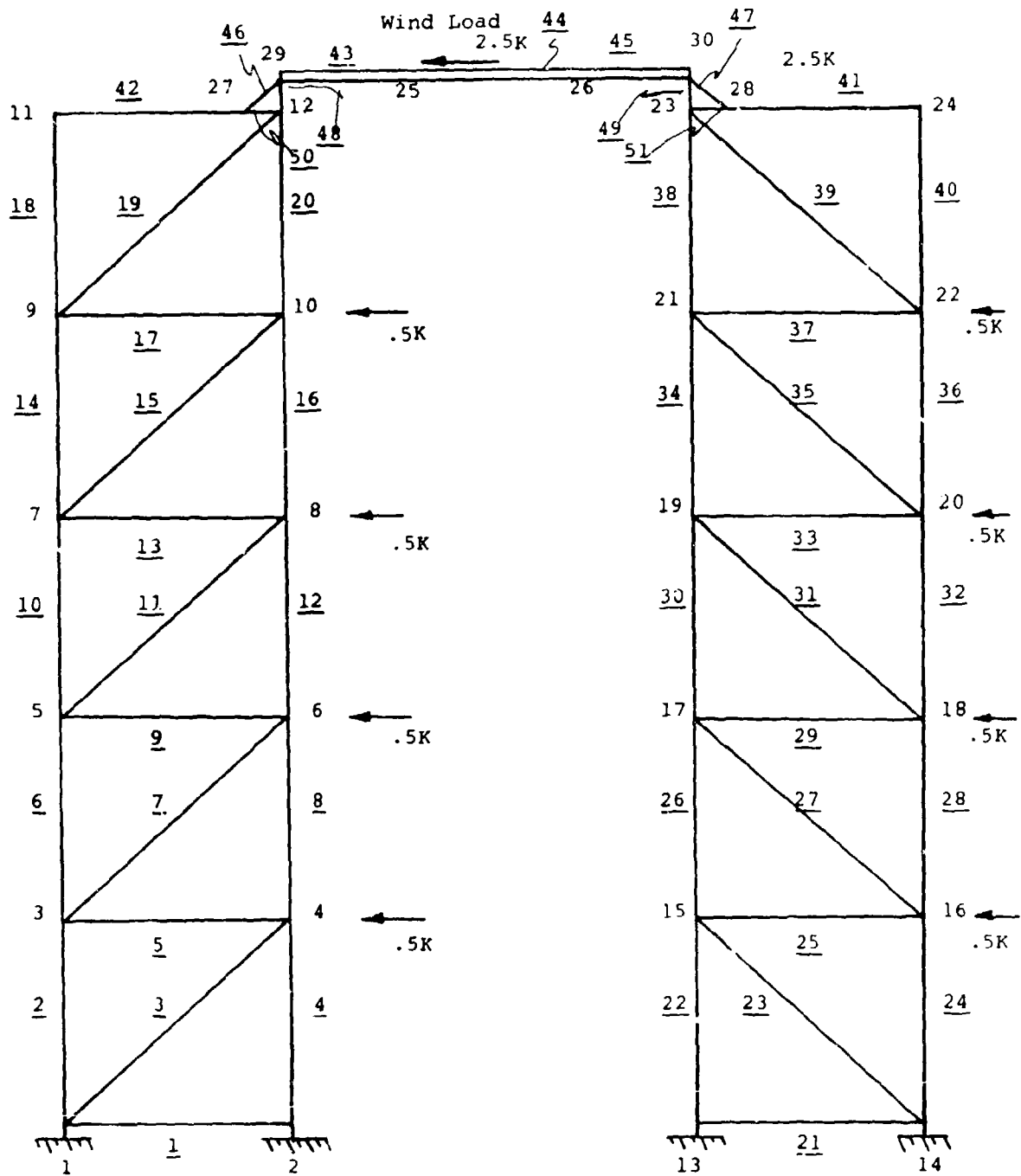
The tower assembly is analyzed using the plain frame computer program.\* The fore-aft frame is analyzed using three separate loads: wind loads, test loads, and deadweight. The total load from the three combined loads and stress is computed for each member. The inboard-outboard frame is analyzed in the same way. To obtain the maximum stress in the main vertical columns it is necessary to add the stress found in the inboard-outboard frame to the stress of the fore-aft frame found as a result of the horizontal test loads only. The tower component identification is shown below.

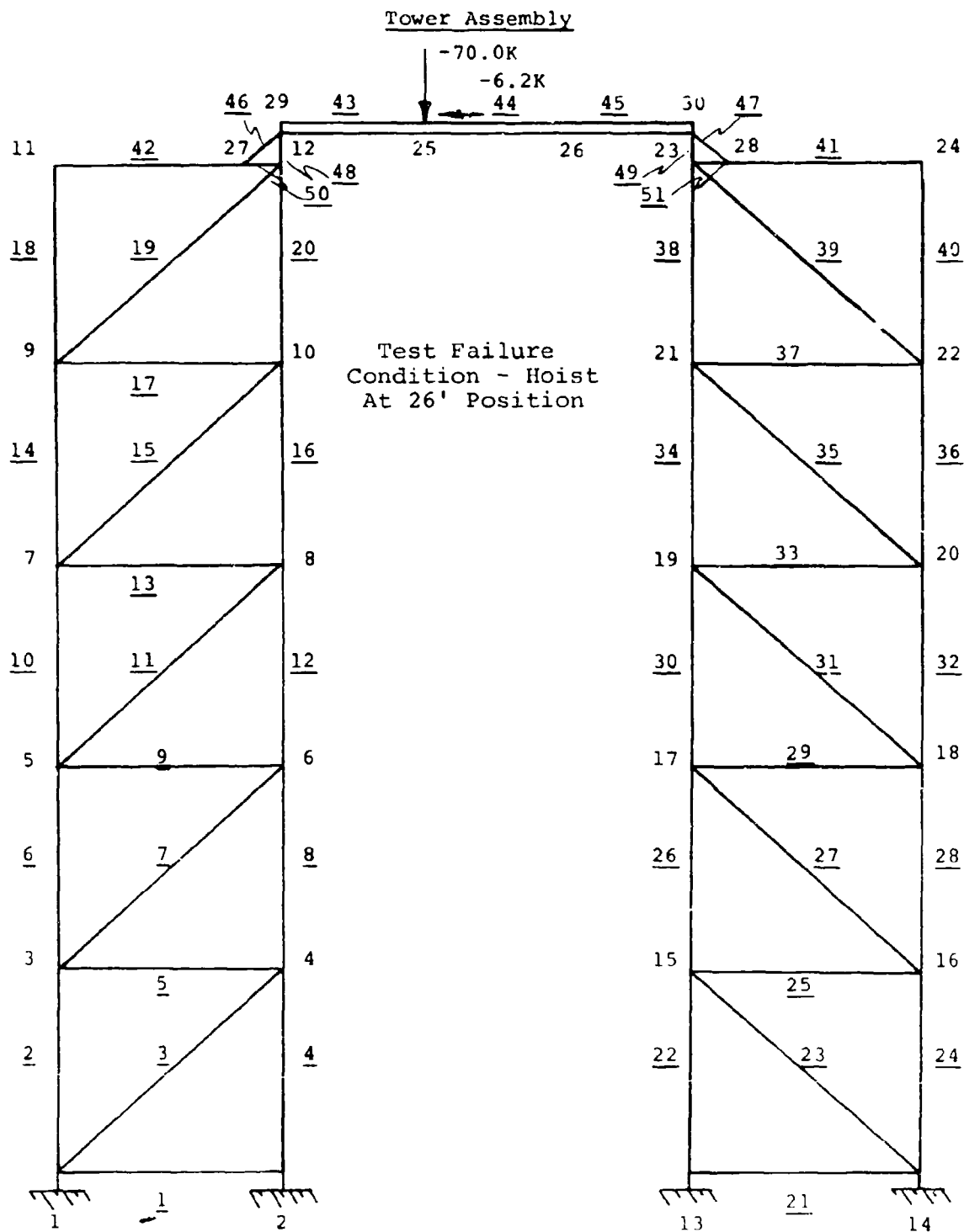
(\*Boeing-Wichita Watfor computer program.)

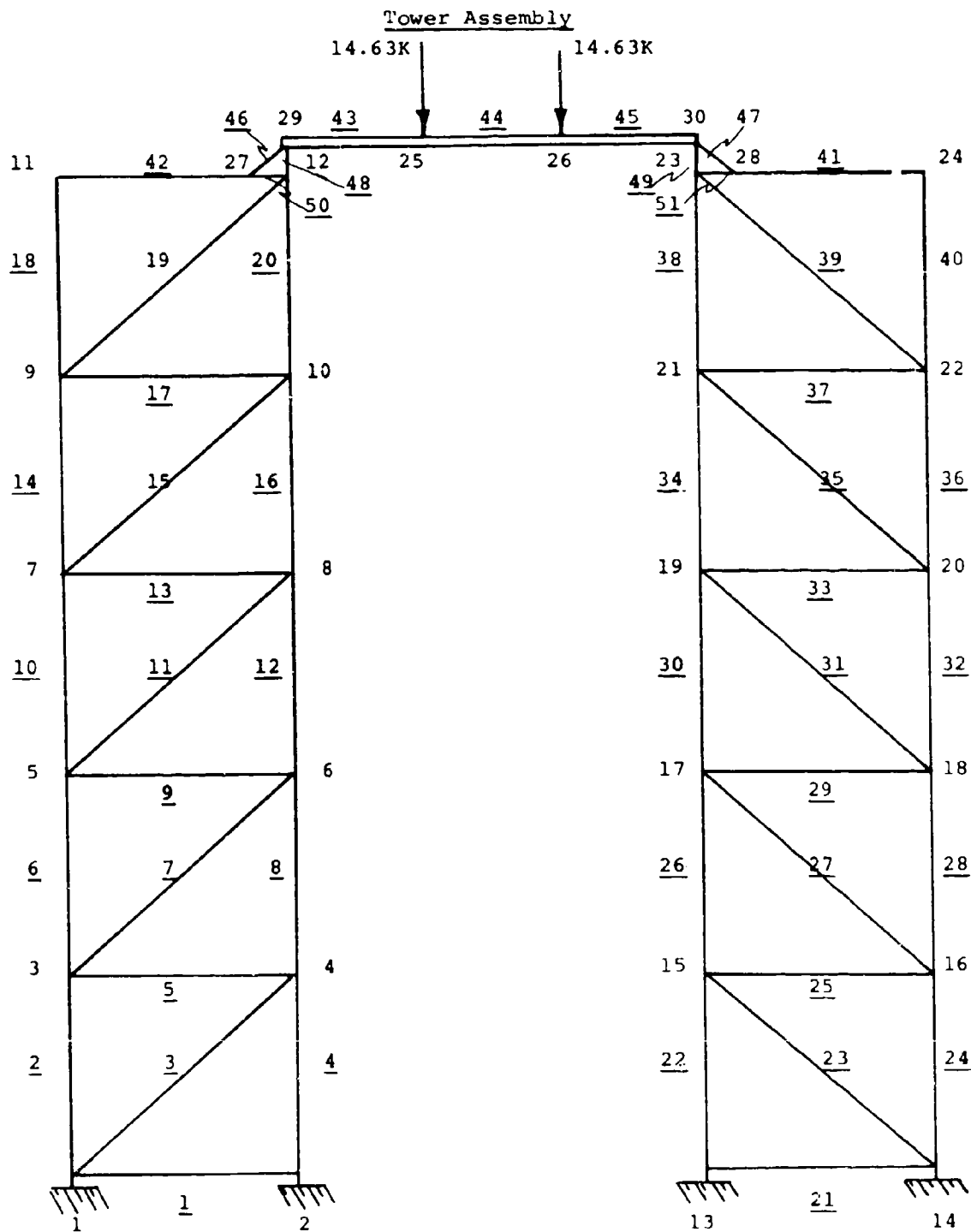




# Tower Assembly - Fore-Aft Frame



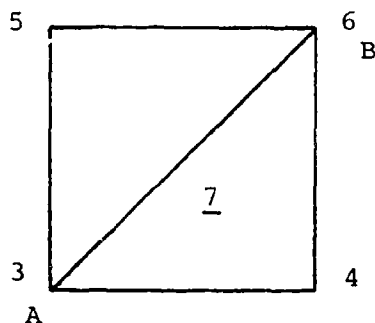




## GLOSSARY - COMPUTER ANALYSIS

Coordinates of Joints - Location of joints of frame using X and Y coordinate axes, the X axis being the line formed by base of towers and the Y axis the left-hand column centerline.

Member End Action - Nodes 1 and 2 define coordinate axis system of each member (see example). Node 1 represents "A" end of member; Node 2 represents "B" end of member. The line 3-6 is the Y axis of the member. The X



axis is 90° to the Y axis. Negative "X" action at the "A" end, and positive "X" action at the "B" end would indicate tension (KIPS) in member. "Y" action indicates shear (KIPS) in member. MZ action indicates moment (In.K.) about X axis of member.

### Member Information:

Area - Cross-sectional area (In.<sup>2</sup>) of member.

Inertia - Moment of inertia (In.<sup>4</sup>) of member.

Modulus - Modulus of elasticity of member.

Length - Length in inches.

Actions applied at joints - load applied at joints.

Support Reactions - foundation loads.

X Reaction - Shear

Y Reaction - Positive (down load)  
- Negative (up load)

Member	Size	Area	Moment	Wind Load	Test Load	Dead-weight	Horiz. Test Only	Total Load
1	(7F) 4x3-1/2x3/8	5.34	.00	.00	.00	.00	.00	.00
2	8 WF 31	9.12	18.73	12.27	25.99	5.92		44.18
3	7F 4x3-1/2x3/8	5.34	.49	5.92	9.13	2.08		17.13
4	8 WF 40	11.76	13.47	-16.24	25.63	7.28	14.56	16.67
5	7F 4x3-1/2x3/8	5.34	1.94	-3.93	-6.74	-1.54		-12.21
6	8 WF 31	9.12	1.82	8.63	19.82	4.52		33.07
7	7F 4x3-1/2x3/8	5.34	1.41	5.41	9.18	2.09		16.68
8	8 WF 40	11.76	6.29	-12.26	31.77	8.68	11.75	28.19
9	7F 4x3-1/2x3/8	5.34	1.58	-3.53	-6.87	-1.57		-11.97
10	8 WF 31	9.12	6.50	5.45	13.61	3.10		22.16
11	7F 4x3-1/2x3/8	5.34	1.48	4.73	9.23	2.10		16.06
12	8 WF 40	11.76	4.35	-8.62	37.94	10.08	8.97	39.40
13	7F 4x3-1/2x3/8	5.34	1.95	-2.93	-6.64	-1.51		-11.08
14	8 WF 31	9.12	6.43	2.76	7.53	1.72		12.01
15	7F 4x3-1/2x3/8	5.34	.44	4.00	9.05	2.06		15.11
16	8 WF 40	11.76	35.35	-5.44	44.15	11.50	6.16	50.21
17	7F 4x3-1/2x3/8	5.34	1.42	-2.75	-7.51	-1.71		-11.97
18	8 WF 31	9.12	36.70	.36	.98	.22		1.56
19	7F 4x3-1/2x3/8	5.34	6.71	3.55	9.72	2.22		15.49
20	8 WF 40	11.76	115.96	-2.75	50.22	12.88	3.39	60.35
21	7F 4x3-1/2x3/8	5.34	.00	.00	.00	.00		.00

TABLE VI. Continued.

Member	Size	Area	Moment	Wind Load	Test Load	Dead-weight	Horiz. Test Only	Total Load
22	8 WF 40	11.76	-10.75	16.08	9.21	7.28		32.57
23	7F 4x3-1/2x3/8	5.34	-.28	-5.87	.88	2.08		-2.91
24	8 WF 31	9.12	-12.05	-12.15	2.45	5.92		-3.78
25	7F 4x3-1/2x3/8	5.34	1.05	4.39	-.64	-1.54		-2.21
26	8 WF 40	11.76	-2.48	12.13	9.80	8.68		30.61
27	7F 4x3-1/2x3/8	5.34	.49	-5.36	.86	-2.09		-2.41
28	8 WF 31	9.12	1.60	-8.53	1.87	4.52		-6.66
29	7F 4x3-1/2x3/8	5.34	.75	3.99	-.65	-1.57		1.77
30	8 WF 40	11.76	1.62	8.52	10.38	10.08		28.98
31	7F 4x3-1/2x3/8	5.34	.45	-4.68	.87	2.10		-1.71
32	8 WF 31	9.12	-2.31	-5.38	1.29	3.10		-.99
33	7F 4x3-1/2x3/8	5.34	.77	3.40	-.63	-1.51		1.26
34	8 WF 40	11.76	-7.14	5.37	10.96	11.50		27.83
35	7F 4x3-1/2x3/8	5.34	.23	-3.96	.85	2.06		-1.05
36	8 WF 31	9.12	2.39	-2.72	.72	1.72		-.28
37	7F 4x3-1/2x3/8	5.34	.73	3.21	-.71	-1.71		.79
38	8 WF 40	11.76	-14.64	2.71	11.53	12.88		27.12
39	7F 4x3-1/2x3/8	5.34	1.83	-3.50	.93	2.22		-.35
40	8 WF 31	9.12	13.57	-.36	.09	.22		
41	8 WF 31	9.12	34.21	-.11	.02	.06		
42	8 WF 31	9.12	149.98	.12	.29	.06		.47

TABLE VI. Concluded									
Member	Size	Area	Moment	Wind Load	Test Load	Dead-weight	Horiz. Test Only	Total Load	
43	30 WF 172	50.65	See Page 99						
44	30 WF 172	50.65	See Page 99						
45	30 WF 172	50.65	See Page 99						
46	8 WF 31	9.12	-27.86	2.36	7.91	1.84		12.11	
47	8 WF 31	9.12	-16.40	-2.33	.92	1.84			
48	8 WF 40	11.76	-34.44	-1.33	52.70	13.41		64.78	
49	8 WF 40	11.76	-17.11	1.31	11.60	13.41		26.32	
50	8 WF 31	9.12	116.95	-1.85	-5.81	-1.34		-9.00	
51	8 WF 31	9.12	50.44	1.83	-.64	-1.34			

SK301-11304-1  
Tower Assembly - Fore-Aft Frame

The following calculations are for stress in the members of the frame (see page 103 for member numbers).

General Equation:

$$f = \frac{\text{Moment} \times 1.5}{\text{Section Modulus}} + \frac{\text{Total Axial Load} \times 1.5}{\text{Area}}$$

Member 19

$$f = \frac{6.71 \times 1.5}{3.0} + \frac{15.49 \times 1.5}{5.34} = 3.36 + 4.36 = 7.72 \text{KSI}$$

$$\frac{L}{r} = \frac{226.5}{1.25} = 181 \quad Fa = 6.56$$

Member 20

$$f = \frac{115.96 \times 1.5}{35.5} + \frac{60.35 \times 1.5}{11.76} = 4.92 + 7.70 = 12.62 \text{KSI}$$

$$\frac{L}{r} = \frac{152}{2.04} = 74.5 \quad Fa = 14.89 \text{ KSI}$$

Member 42

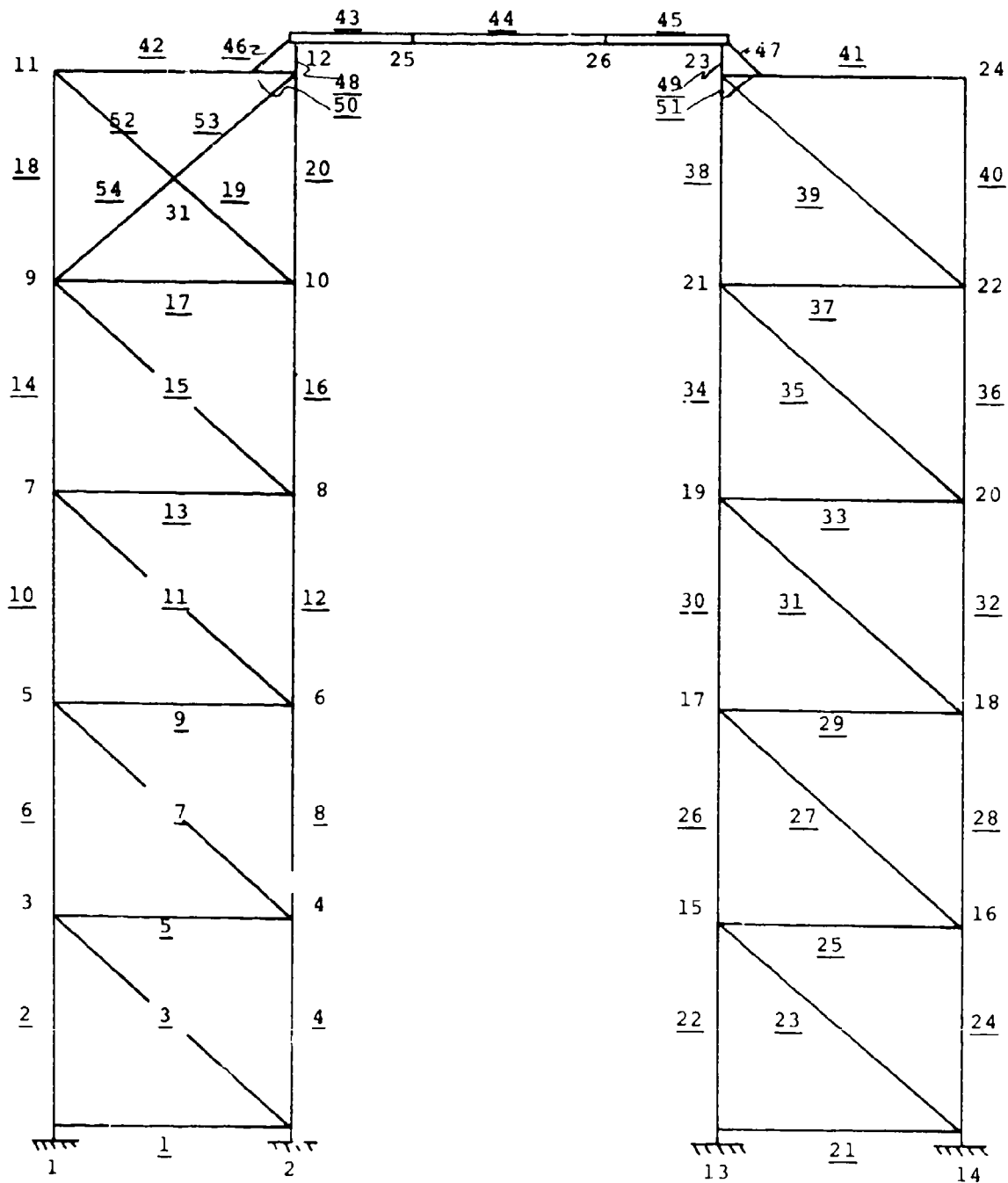
$$f = \frac{149.98 \times 1.5}{27.4} + \frac{.47 \times 1.5}{9.12} = 8.23 + .08 = 8.31 \text{KSI}$$

$$\frac{L}{r} = \frac{168}{2.01} = 83.5 \quad Fa = 14.03 \text{ KSI}$$

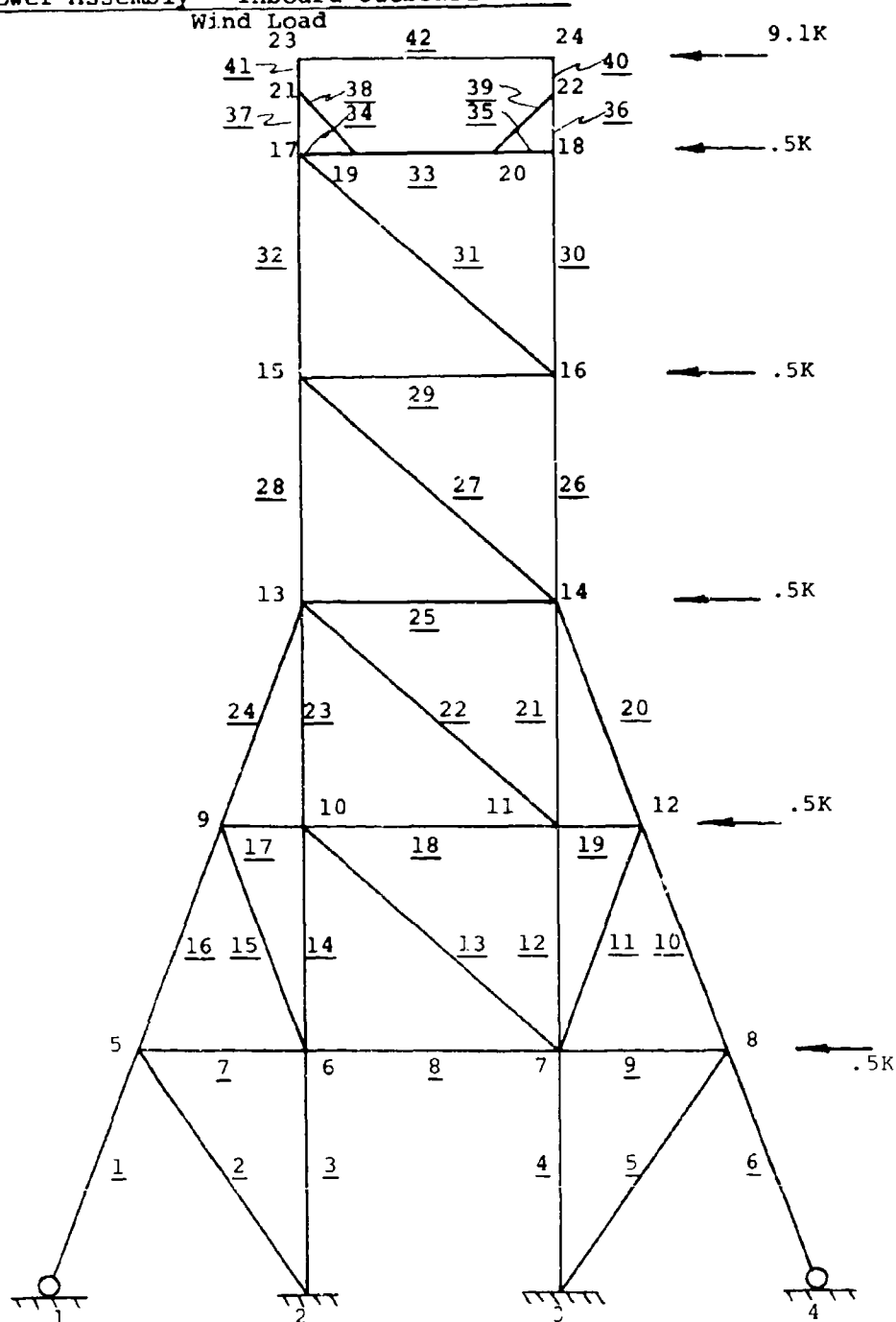




Tower Assembly  
(Optional Construction)



# Tower Assembly - Inboard-Outboard Frame



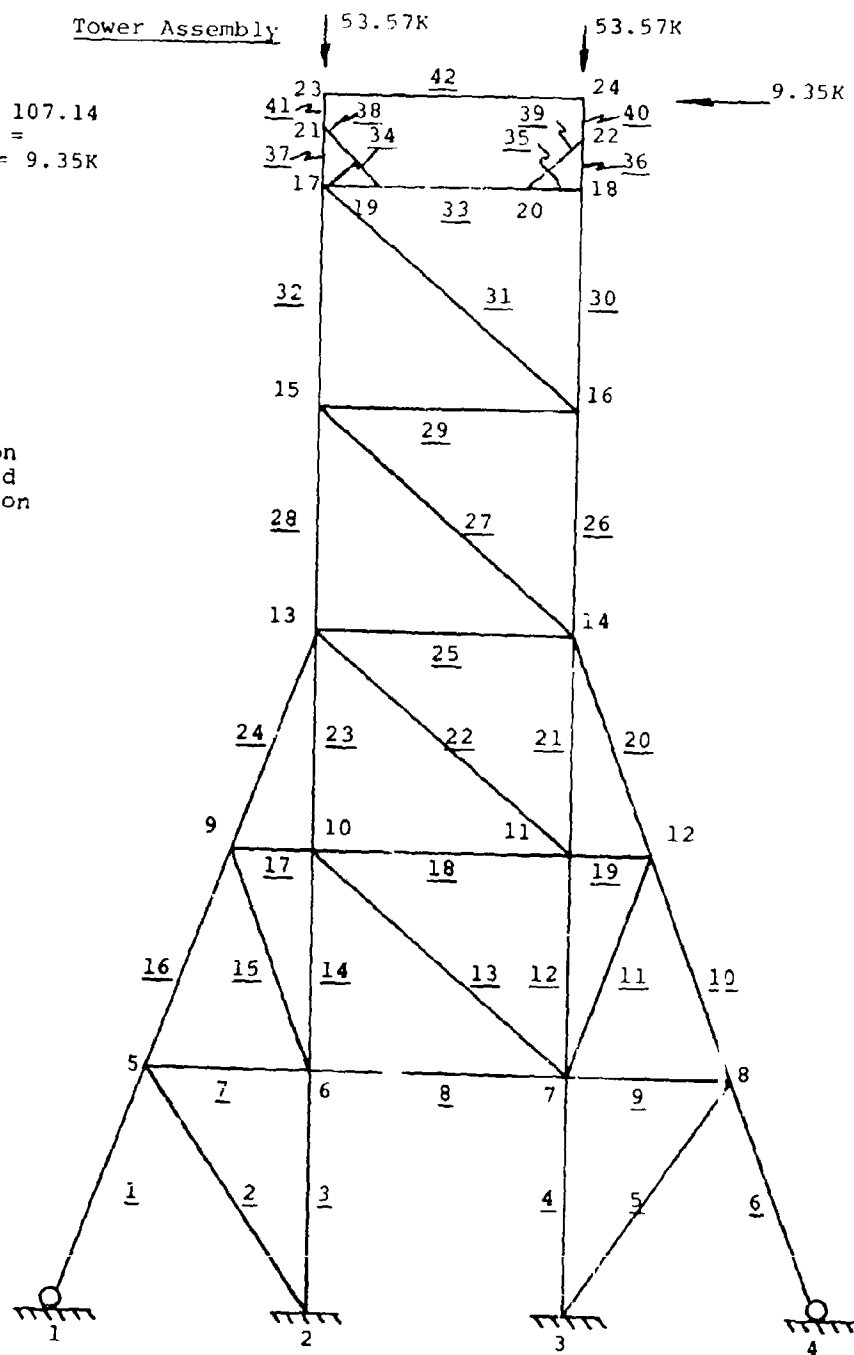
Tower Assembly

$$53.57 + 53.57 = 107.14$$

$$\sin 5^\circ (107.14) =$$

$$.0871 (107.14) = 9.35K$$

Failure Condition  
70-ton proof load  
26' hoist position



Deadweight Load

Tower Assembly

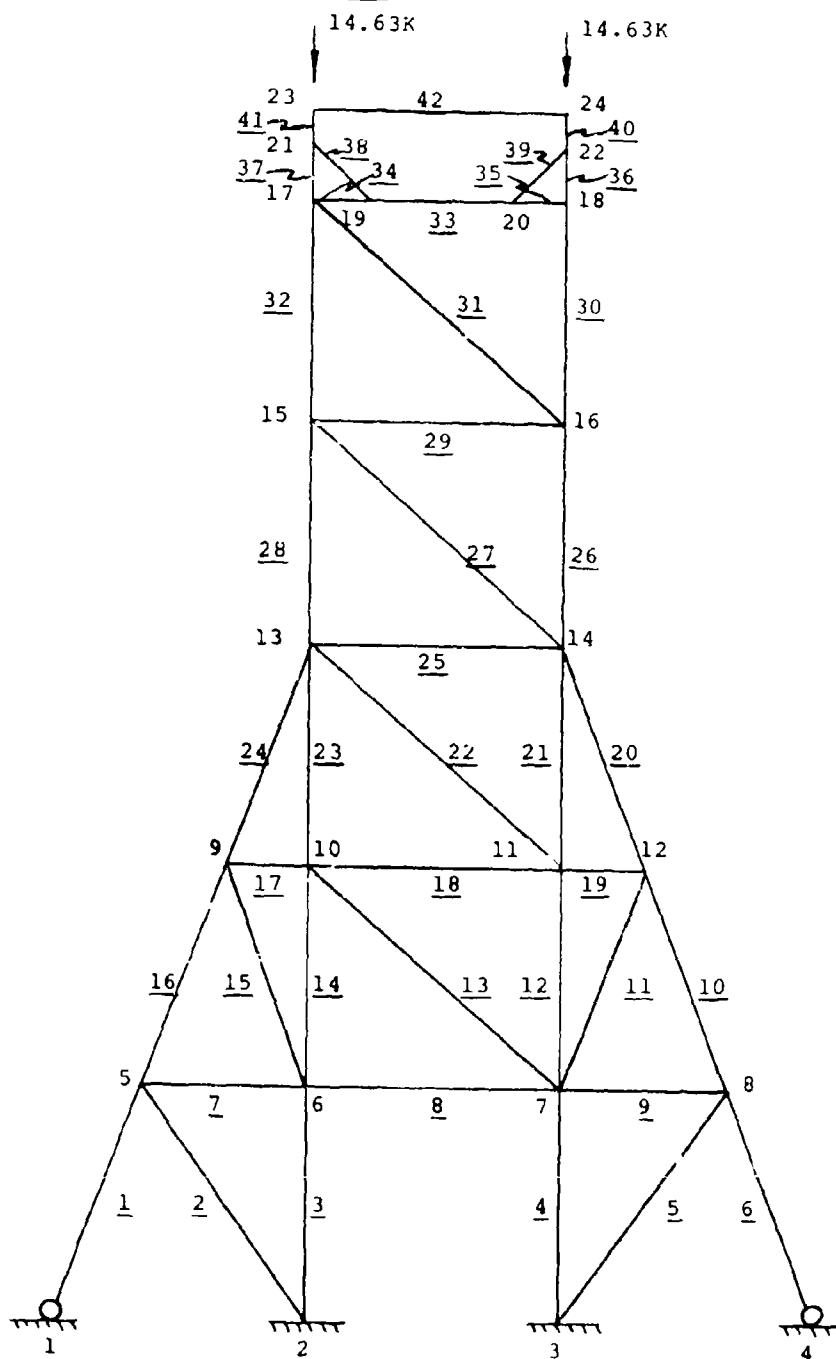


TABLE VII. TOWER ASSEMBLY - INBOARD/OUTBOARD FRAME.									
Member	Size	Area	Sect.Mod.	Moment	Wind Ld	Test Ld	Dead-weight	Total	
1	10 WF 33	9.71	9.2	Very small	+14.94	31.03	4.85	50.86	
2	7F 4x3-1/2x3/8	5.34	3.0		-.68	4.38	1.24	4.94	
3	8 WF 40	11.76	12.1		+8.72	40.27	8.65	57.64	
4	8 WF 40	11.76	12.1		-8.35	28.28	9.92	29.85	
5	7F 4x3-1/2x3/8	5.34	3.0		1.02	3.92	1.02	5.96	
6	10 WF 33	9.71	9.2		-15.63	3.04	4.50	-8.09	
7	7F 4x3-1/2x3/8	5.34	3.0		.60	-3.90	-1.10	-4.40	
8	7F 4x3-1/2x3/8	5.34	3.0		.65	-2.01	-.62	-1.98	
9	7F 4x3-1/2x3/8	5.34	3.0		-.90	-3.48	-.90	-4.78	
10	10 WF 33	9.71	9.2		-14.75	6.40	5.37	-2.98	
11	7F 4x3-1/2x3/8	5.34	3.0		-.19	4.70	1.46	5.97	
12	8 WF 40	11.76	12.1		-7.16	23.75	8.36	24.95	
13	7F 4x3-1/2x3/8	5.34	3.0		-1.52	.19	.03	-1.30	
14	8 WF 40	11.76	12.1		8.61	35.08	7.34	51.03	
15	7F 4x3-1/2x3/8	5.34	3.0		.13	5.55	1.41	7.09	
16	10 WF 33	9.71	9.2		14.35	34.77	5.95	55.07	
17	7F 4x3-1/2x3/8	5.34	3.0		-.07	-3.79	-.97	-4.83	
18	7F 4x3-1/2x3/8	5.34	3.0		1.09	-3.86	-1.17	-3.94	
19	7F 4x3-1/2x3/8	5.34	3.0		.62	-3.23	-1.00	-3.61	
20	10 WF 33	9.71	9.2		-14.90	11.08	6.81	2.99	
21	8 WF 40	11.76	12.1		-6.74	23.21	6.21	24.68	

TABLE VII. Concluded.									
Member	Size	Area	Sect.Mod.	Moment	Wind Ld	Test Ld	Dead-weight	Total	
22	7/4x3-1/2x3/8	5.34	3.0	1.11	- .68	.84	.25	.41	
23	8 WF 40	11.76	12.1	9.37	7.64	35.29	7.55	50.48	
24	10 WF 33	9.71	9.2	1.29	14.44	40.23	7.34	62.01	
25	7/4x3-1/2x3/8	5.34	3.0	3.71	5.42	13.16	2.34	20.92	
26	8 WF 40	11.76	12.1	12.29	-11.67	42.01	14.60	44.94	
27	7/6x3-1/2x3/8	6.84	6.5	3.50	-13.47	-12.49	.01	-25.96	
28	8 WF 40	11.76	12.1	9.06	20.71	73.58	14.60	108.89	
29	7/4x3-1/2x3/8	5.34	3.0	2.87	10.10	9.36	-.01	19.45	
30	8 WF 40	11.76	12.1	22.95	-2.77	50.68	14.58	63.49	
31	7/6x3-1/2x3/8	6.84	6.5	8.17	-13.20	-12.86	.02	-26.04	
32	8 WF 40	11.76	12.1	12.05	11.66	65.16	14.60	91.42	
33	8 WF 31	9.12	27.4	246.90	5.15	4.98	.05	10.18	
34	8 WF 31	9.12	27.4	5.01	11.80	11.30	-.11	21.99	
35	8 WF 31	9.12	27.4	205.44	-1.84	-2.96	-.15	4.95	
36	8 WF 40	11.76	12.1	112.70	-6.03	46.58	14.38	54.93	
37	8 WF 40	11.76	12.1	97.71	5.91	58.95	14.40	79.81	
38	8 WF 31	9.12	27.4	24.51	-8.58	-7.96	.25	-16.29	
39	8 WF 31	9.12	27.4	35.46	8.92	10.27	.29	29.48	
40	8 WF 40	11.76	12.1	143.68	-.49	53.09	14.60	67.20	
41	8 WF 40	11.76	12.1	133.90	.49	54.11	14.60	69.20	
42	8 WF 31	9.12	27.4	83.72	4.55	4.52	-.05	9.02	

SK301-11304-1  
Tower Assembly - Inboard/Outboard Frame

$$f = \frac{M \times 1.5}{S} + \frac{P \times 1.5}{A}$$

Member 1  $f = \frac{50.86 \times 1.5}{9.71} = 7.86 \text{ KSI}$

$$\frac{L}{r} = \frac{162.0}{1.94} = 83.5 \quad F_A = 14.03 \text{ KSI}$$

Member 2  $f = \frac{.90 \times 1.5}{3.0} + \frac{4.94 \times 1.5}{5.34} = .45 + 1.39 = 1.84 \text{ KSI}$

$$\frac{L}{r} = \frac{189}{1.25} = 151 \quad F_A = 7.69$$

Member 3  $f = \frac{3.28 \times 1.5}{12.1} + \frac{57.64 \times 1.5}{11.76} = .41 + 7.36 = 7.77 \text{ KSI}$

$$\frac{L}{r} = \frac{152}{2.04} = 74.5 \quad F_A = 14.89$$

Member 15  $f = \frac{1.03 \times 1.5}{3} + \frac{7.09 \times 1.5}{5.34} = .52 + 1.99 = 2.51 \text{ KSI}$

$$\frac{L}{r} = \frac{162}{1.25} = 129.6 \quad F_A = 9.30 \text{ KSI}$$

Member 23  $f = \frac{9.37 \times 1.5}{12.1} + \frac{50.98 \times 1.5}{11.76} = 1.16 + 6.44 = 7.60 \text{ KSI}$

$$\frac{L}{r} = 74.5 \quad F_A = 14.89 \text{ KSI}$$

Member 24  $f = \frac{1.29 \times 1.5}{9.2} + \frac{62.01 \times 1.5}{9.71} = .21 + 9.58 = 9.79$

$$\frac{L}{r} = 83.5 \quad F_A = 14.03$$



$$\text{Member 25 } f = \frac{3.71 \times 1.5}{3.0} + \frac{20.92 \times 1.5}{5.34} = 1.85 + 5.88 = 7.73 \text{ KSI}$$

$$\frac{L}{r} = \frac{168}{1.25} = 134 \quad F_A = 8.94$$

$$\text{Member 28 } f = \frac{9.06 \times 1.5}{12.1} + \frac{108.89 \times 1.5}{11.76} = 1.12 + 13.90 = 15.02 \text{ KSI}$$

$$\frac{L}{r} = \frac{152}{2.04} = 74.5 \quad F_A = 15.90 \text{ KSI (A-36 steel)}$$

$$\text{Member 31 } f = \frac{8.17 \times 1.5}{6.5} + \frac{26.04 \times 1.5}{6.84} = 1.88 + 5.50 = 7.38 \text{ KSI}$$

$$\frac{L}{r} = \frac{227}{1.39} = 163 \quad F_A = 7.16 \text{ KSI} \quad \text{Over}$$

$$\text{Member 32 } f = \frac{12.05 \times 1.5}{12.1} + \frac{91.42 \times 1.5}{11.76} = 1.49 + 11.65 = 13.14 \text{ KSI}$$

$$\frac{L}{r} = 74.5 \quad F_A = 15.90 \text{ KSI}$$

$$\text{Member 35 } f = \frac{205.4 \times 1.5}{27.4} + \frac{4.95 \times 1.5}{9.12} = 11.24 + .81 = 12.05 \text{ KSI}$$

$$\frac{L}{r} = \frac{38.0}{1.61} = 23.6 \quad F_A = 20.35 \text{ KSI}$$

#### Maximum Stress in Main Vertical Columns

$$f = f(\text{Inbd-Outbd Frame}) + f(\text{Fwd-Aft Frame Horiz. Test Only})$$

$$\text{Member 28 } f = 15.02 + \frac{6.16 \times 1.5}{11.76} = 15.80 \text{ KSI}$$

$$\frac{L}{r} = \frac{152.0}{2.01} = 75.62 \quad F_A = 15.90 \text{ KSI}$$

SK301-11304-1  
Tower Assembly - Column Splice - Friction

Design splice load =  $70.98 \times 1.5 = 106.4K$   
(Max.load)       $70.98 = 8.97 + 62.01$

16 Ea. 3/4 Dia. Bolts/Splice

Friction Load/Bolt =  $19.2K(.35) = 6.73K$

$$16 \times 6.73K = 107.6K$$

(2) 3/4 x 9.0 Plate     $A = 13.5 \text{ In.}^2$

$$f = \frac{P}{A} = \frac{106.4}{13.5} = 7.88 \text{ KSI}$$

32 In. of 3/8 Fillet Weld ( $3.6 \text{ K/In.}$ ) = 115K

Diagonal Strut Splice - Friction

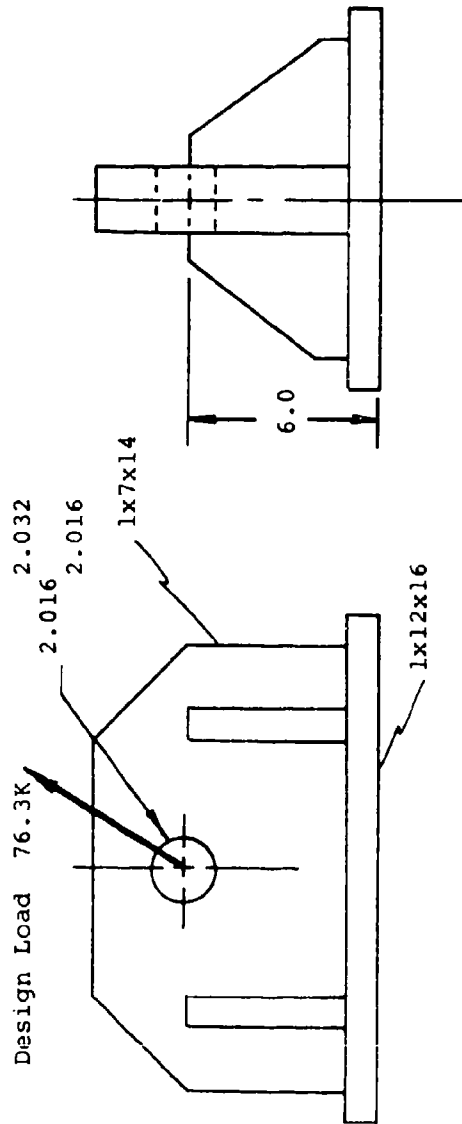
Design Splice Load =  $16.06 \times 1.5 = 24.1K$   
(Max.load)

(2) Ea. 3/4 Dia. Bolts/Splice - Double Friction

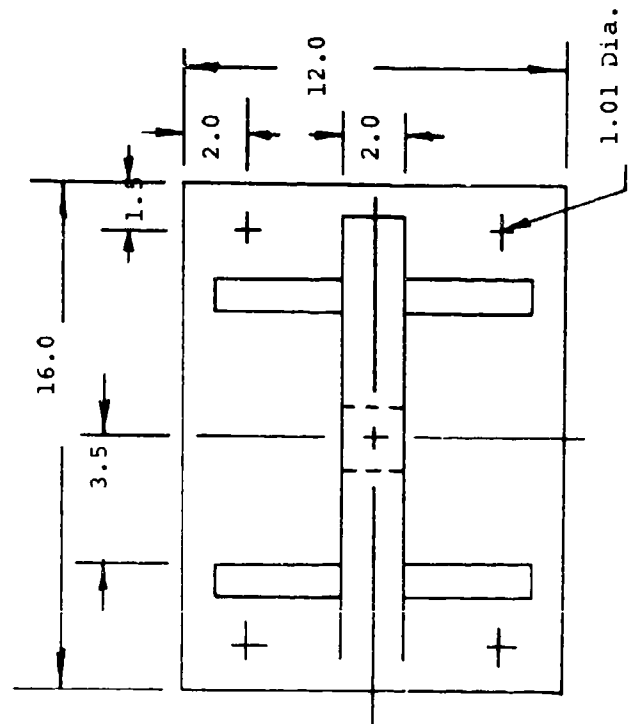
Friction Load/Bolt =  $2 \times 6.73 = 13.46K$

$$2 \text{ Bolts} = 26.92K$$

SK301-11304-10  
 Outrigger Base Plate Assembly



All Weld 3/8 Fillet



SK301-11304-10 Assembly  
-82 Assembly  
Outrigger Tiedown Assembly

-1C Assembly Shear Out

$$\text{Shear Area} - 1.5 \times 2 \times 2 = 6.0 \text{ In.}^2$$

$$\text{Shear Stress} = \frac{P}{A} = \frac{76.3}{6} = 12.7 \text{ KSI}$$

Tension at 2.0 Diameter Hole -82 Assembly

$$\text{Area} = 2 \times 3 = 6.0 \text{ In.}^2$$

$$f_t = \frac{P}{A} = \frac{76.3}{6.0} = 12.7 \text{ KSI}$$

Double Shear Pin 2.0 Diameter A307

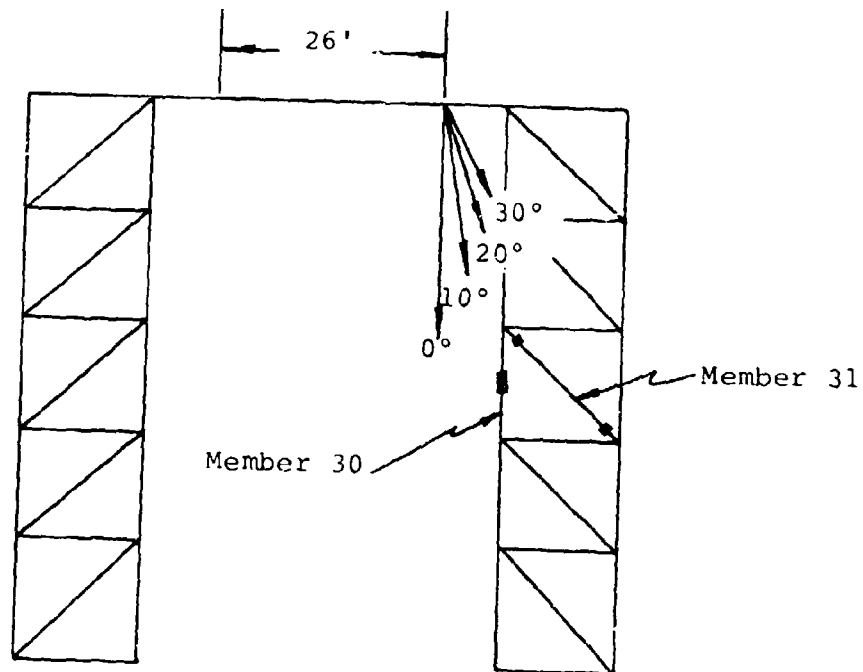
$$A = 2 \quad r^2 = 6.28 \text{ In.}^2$$

$$S = \frac{P}{A} = \frac{76.3}{6.28} = 12.14 \text{ KSI}$$

Inches of 3/8 Inch weld required.

$$\frac{76.3}{3.6} = 21.2 \text{ In.}$$

Failure Loads @ 0°, 10°, 20° & 30°  
Single-Point - 26-Ft Hoist Position



Failure Load 0° -- 537.0K

Failure Load 10° -- 297.0K

Failure Load 20° -- 180.0K

Failure Load 30° -- 135.0K

Location of Failure

Member 30  
Friction splice in  
main vertical column  
(8 WF 40)

Member 31  
Friction splice in  
diagonal brace

Member 31  
Friction splice in  
diagonal brace

Member 31  
Friction splice in  
diagonal brace

Towers would not collapse from these failure conditions.

### Failure Loads

$$F_{cr} = \frac{C \pi^2 E}{\left(\frac{L}{r}\right)^2} = \text{Critical stress}$$

Use a value for c of 1.5 which corresponds to an effective length of .815L in the x-x axis.

K = 1.0 in the y-y axis

#### x-x Axis

$$F_{cr} = \frac{286 \times 10^6}{\left[\frac{.815(226)}{1.25}\right]^2} = 13 \text{ KSI}$$

#### y-y Axis

$$F_{cr} = \frac{286 \times 10^6}{\left[\frac{226}{1.56}\right]^2} = 13.6 \text{ KSI}$$

x-x Axis is critical  $F_{cr} = 13.0 \text{ KSI}$  for double angle struts

### 30° Load Angle

Moment in diagonal member - 9.10 In. K

Axial load in diagonal member - 20.92K

Above values are member loads as a result of a 50K load applied on fwd-aft frame.

$$f_a = \frac{P}{A} = \frac{20.92}{5.34} = 3.92 \text{ KSI}$$

$$f_b = \frac{M}{S} = \frac{9.10 \text{ In. K}}{3.0 \text{ In}^3} = 3.03 \text{ KSI}$$

$$(3.03 + 3.92) \times = 13.0 \text{ KSI}$$

$$x = \frac{13.0}{6.95} = 1.87$$

$$50K(1.87) = 93.40K/\text{Frame}$$

$$93.40K(2) = 186.80K \text{ total failure load}$$

### 20° Load Angle

Member loads as a result of a 50K load applied on fore-aft frame:

$$M = 6.82 \text{ In. K}$$

$$P = 15.65 \text{ K}$$

$$f_b = \frac{M}{S} = \frac{6.82}{3.0} = 2.27 \text{ KSI}$$

$$f_a = \frac{P}{A} = \frac{15.65}{5.34} = 2.94 \text{ KSI}$$

$$(2.27 + 2.94)x = 13.0 \text{ KSI}$$

$$x = \frac{13.0}{5.21} = 2.49$$

$$50K(2.49) = 125K/\text{Frame}$$

250K total failure load

### 10° Load Angle

Member loads as a result of a 50K load applied on fore-aft frame:

$$M = 4.12 \text{ In. K}$$

$$P = 9.5 \text{ In. K}$$

$$f_b = \frac{M}{S} = \frac{4.12}{3.0} = 1.37 \text{ KSI}$$

$$f_a = \frac{P}{A} = \frac{9.5}{5.34} = 1.78 \text{ KSI}$$

$$(1.37 + 1.78)x = 13.0 \text{ KSI}$$

$$x = \frac{13.0}{3.15} = 4.12$$

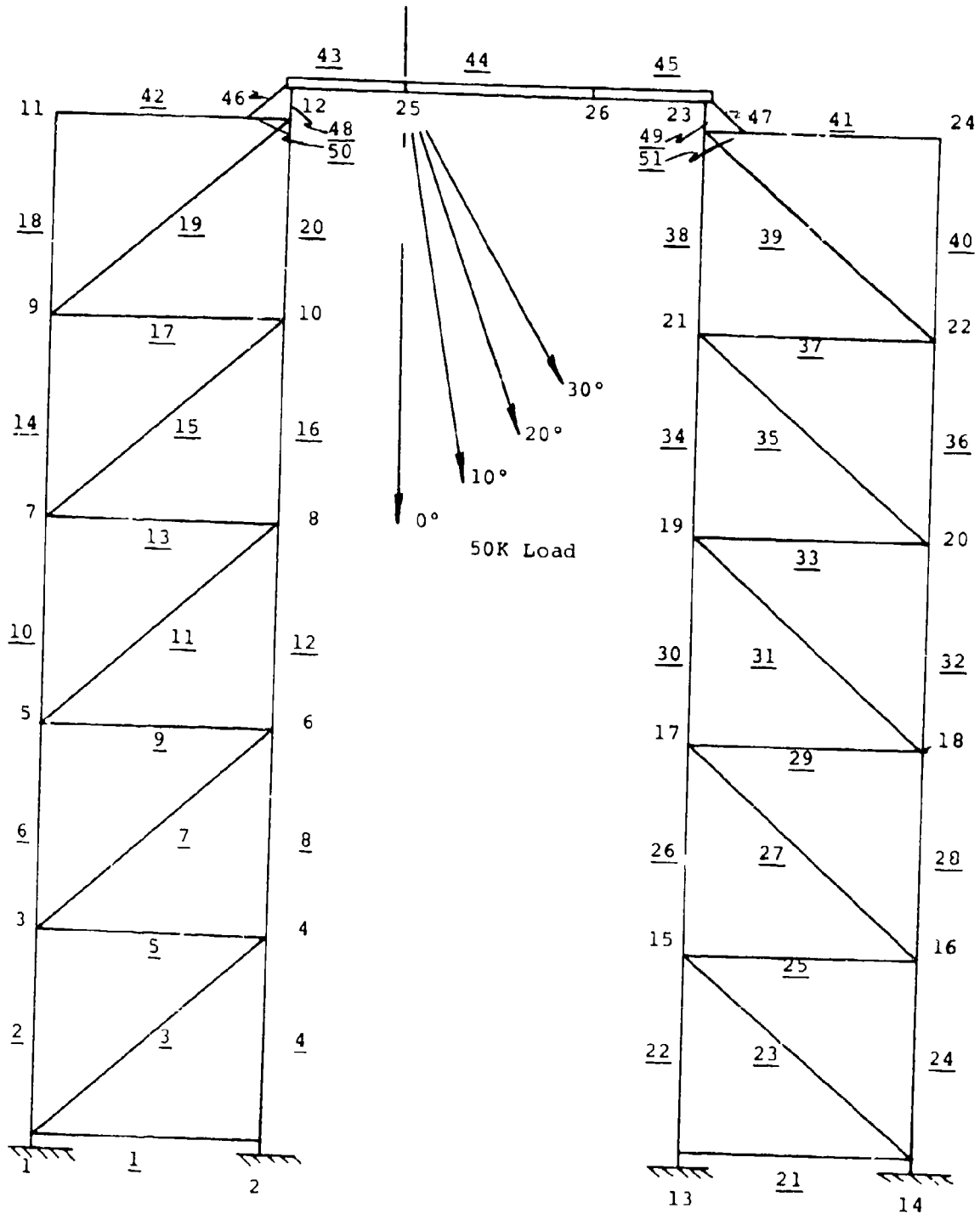
$$50K \times 4.12 = 206 \text{ K/Frame}$$

412K Total failure load

### 0° Load Angle

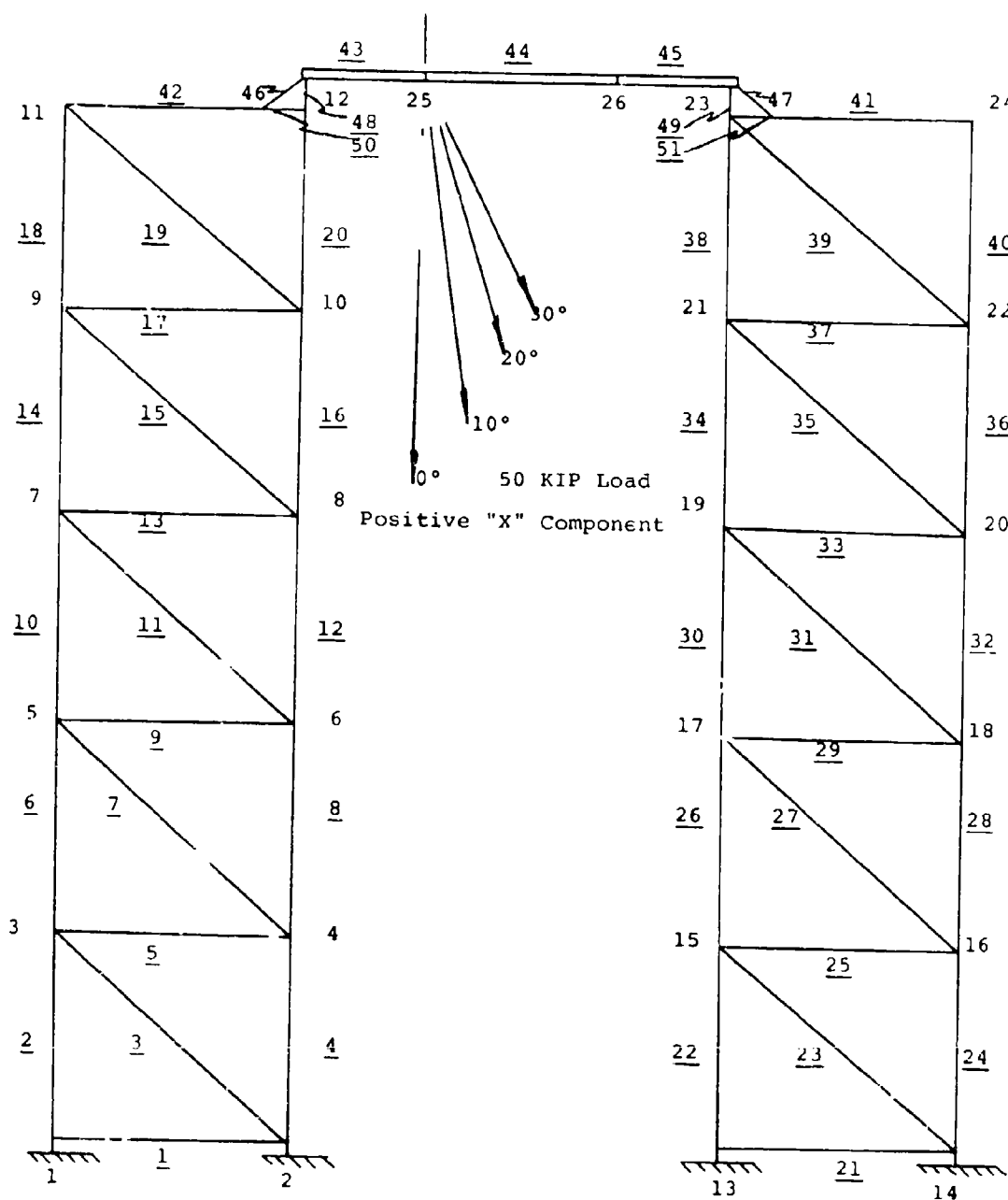
412K total failure load

# Test Tower Assembly





Test Tower Assembly  
(Optional Construction)



### Foundation Requirements

The following pages show foundation requirements in KIPS. In the load column, five separate load requirements are listed:

1. Foundation requirements as a result of side wind loads.
2. Foundation requirements as a result of test failure loads.
3. Foundation requirements as a result of positive or negative horizontal component of test load in the fore-aft direction.
4. Foundation requirements as a result of deadweight of overhead structure.
5. Foundation requirements as a result of deadweight of tower.

All horizontal loads are considered to be reversible, and therefore maximum foundation requirements are symmetrical about  $\bar{C}$  in both X and Y planes.

Load Table VIII is the same as load Table IX except that the horizontal component of test load is reversed.

Table X shows foundation requirements as a result of the empty load container swinging into the tower at maximum possible height of 58 feet. This condition results in maximum uplift requirement.

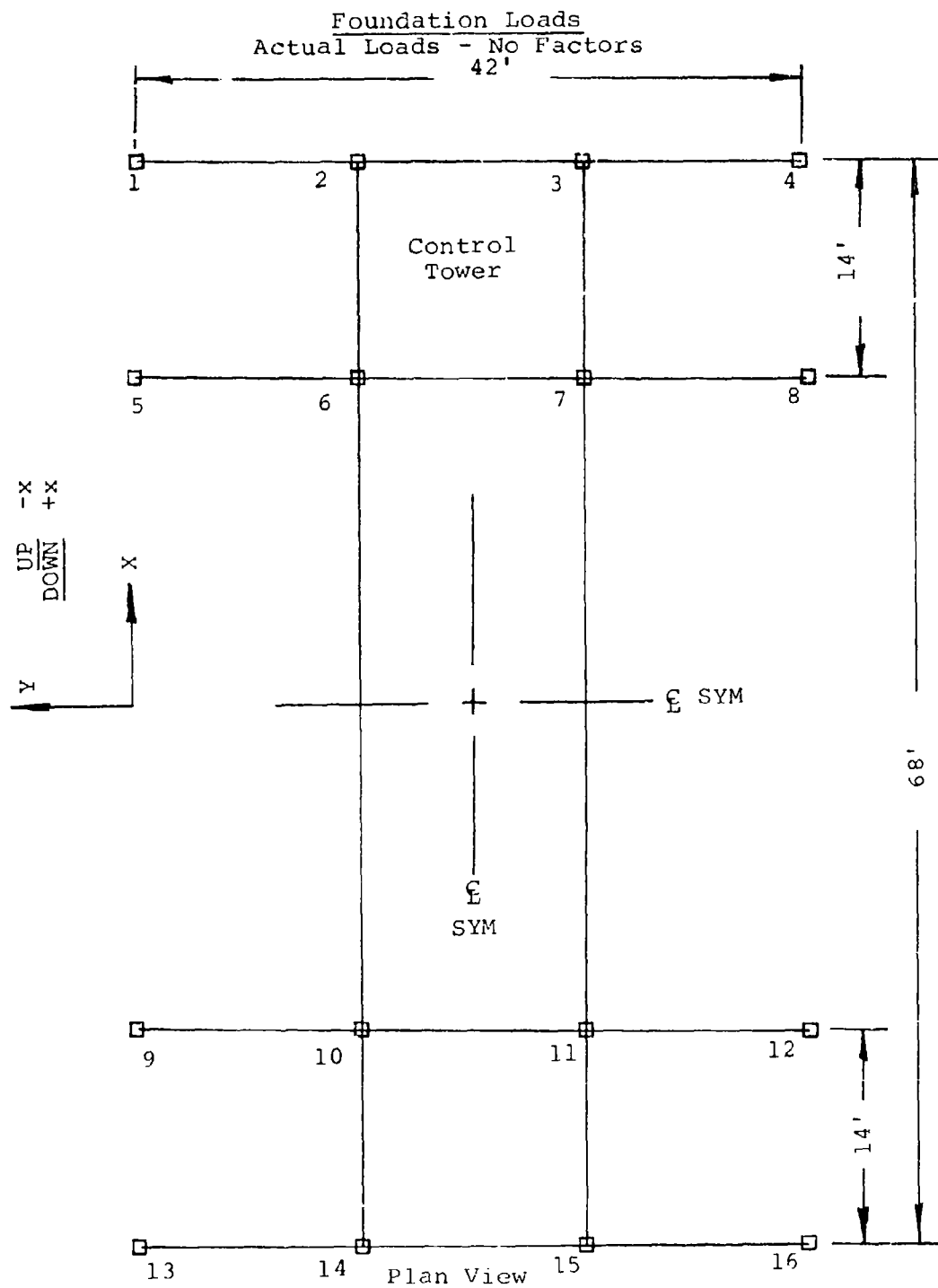


TABLE VIII. REACTION LOADS - 70-TON FAILURE CONDITION.								
	Column No. ○ (KIPS)							
	①		②		③		④	
	X	Z	X	Z	X	Z	X	Z
(1) Wind Load	2.57	6.99	.19	3.88	.24	-3.30	2.79	-7.57
(2) Test Load		4.47		14.01		10.05		3.22
(3) + Horiz.Comp.Test			-3.09	14.56	-3.09	14.56		
(4) Deadwt. Overhead				4.28		4.48		
(5) Deadwt. Tower				10.70		10.70		
TOTAL	2.57	11.46	-2.90	18.31	-2.85	7.37	2.79	-4.35
	⑤		⑥		⑦		⑧	
	X	Z	X	Z	X	Z	X	Z
	X	Z	X	Z	X	Z	X	Z
(1)	5.17	14.02	.42	8.18	.62	-7.53	5.40	-14.67
(2)	10.73	29.12	-2.59	29.80	2.33	21.40	-1.05	2.85
(3)			.01	14.56	-.01	14.56		
(4)		2.85		7.12		7.48		2.99
(5)				10.70		10.70		
TOTAL	15.90	45.99	-1.96	70.36	2.94	46.61	4.35	-8.83

TABLE IX. REACTION LOADS - 70-TON FAILURE CONDITION.								
	①		②		③		④	
	X	Z	X	Z	X	Z	X	Z
	X	Z	X	Z	X	Z	X	Z
(1) Wind Load	2.57	6.99	.19	3.88	.24	-3.30	2.79	-7.57
(2) Test Load		4.47		14.01		10.05		3.22
(3) -Horiz.Comp.Test			3.09	14.56	3.09	14.56		
(4) Deadwt. Overhead				4.28		4.48		
(5) Deadwt. Tower				10.70		10.70		
TOTAL	2.57	11.46	3.28	47.43	3.33	36.49	2.79	4.35
	⑤		⑥		⑦		⑧	
	X	Z	X	Z	X	Z	X	Z
	X	Z	X	Z	X	Z	X	Z
(1)	5.17	14.02	.42	8.18	.62	-7.53	5.40	-14.67
(2)	10.73	29.12	-2.59	29.80	2.33	21.40	-1.05	2.85
(3)			.01	14.56	-.01	14.56		
(4)		2.85		7.12		7.48		2.99
(5)				10.70		10.70		
TOTAL	15.90	45.99	-2.16	41.24	2.94	17.49	3.35	-8.83

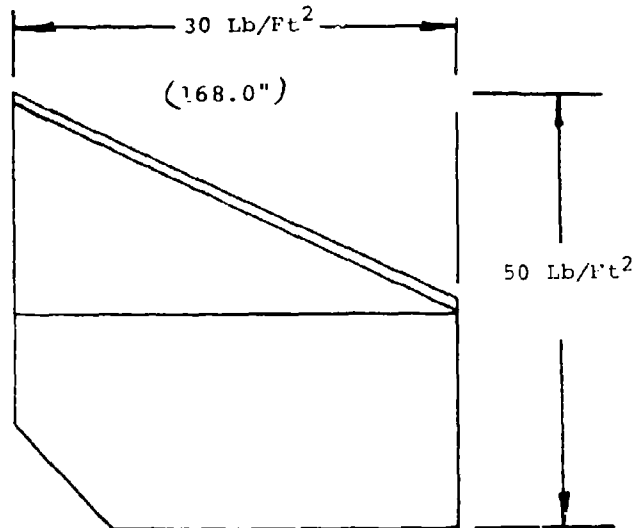
TABLE X. REACTION LOADS - IMPACT OF LOAD CONTAINER ON TOWER.

	Column No. <span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">1</span> KIPS							
	<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">1</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">2</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">3</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">4</span>	
	X	Z	X	Z	X	Z	X	Z
(1) Impact Load		22.6		52.7		52.7		22.6
(2) Deadweight		15.5		11.0		11.0		5.5
TOTAL		28.1		63.7		63.7		28.1
	<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">5</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">6</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">7</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">8</span>	
(1)		-22.6		-52.7		-52.7		-22.6
(2)		5.5		11.0		11.0		5.5
TOTAL		-17.1		-41.7		-41.7		-17.1
	<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">9</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">10</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">11</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">12</span>	
(1)		22.6		52.7		52.7		22.6
(2)		5.5		11.0		11.0		5.5
TOTAL		28.1		63.7		63.7		28.1
	<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">13</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">14</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">15</span>		<span style="border: 1px solid black; border-radius: 50%; padding: 0 5px;">16</span>	
(1)		-22.6		-57.7		-52.7		-22.6
(2)		5.5		11.0		11.0		5.5
TOTAL		-17.1		-41.7		-41.7		-17.1
<p>(1) 8.0K weight moving horizontally strikes tower at a velocity of 20.6 Ft/Sec. Resulting reaction at base of tower is shown in KIPS.</p> <p>(2) Deadweight of overhead steel and deadweight of tower acting at base of tower in KIPS.</p>								

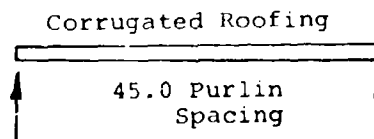
TABLE XI DEADWEIGHTS.

Dwg. No.	Nomenclature	Qty.	Dead Wt/Assy.	Total Dead Wt.
SK301-11304-1	Tower Assy.	1	38723	38723
SK301-11304-2	Tower Assy.	1	42750	42750
SK301-11302-1	Hoist Module	2	2470	4940
-2	Davit Mount	1	6207	6207
-3	Module Support	2	6274	12548
-4	Platform Assy.	1	831	831
-6	Platform Assy.	1	831	831
SK301-11302-5	Beam Assy.	2	8081	16162
—	—	—	—	—
Test Hoist		2	1700	3400
Aux. Hoist		1	3365	3365
Mast		1	3965	3965
—	—	—	—	—
TOTAL				133,722#

# Control Room Shelter



Control room shelter designed for a 50-Lb/Ft<sup>2</sup> wind load on projected frontal area. Roof is designed for 30-Lb/Ft<sup>2</sup> snow load on vertical projected area.



$$M = \frac{wl^2}{8} = \frac{2.5 \text{ Lb/In.} (45)^2}{8}$$

$$M = .634 \text{ In. K}$$

$$f = \frac{M}{S} = \frac{.634}{.0532} = 11.9 \text{ KSI}$$

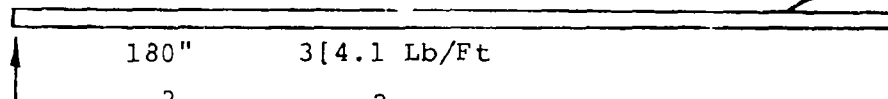
$$S = .0532 \text{ In.}^3 \text{ per ft of width}$$

Standard 2-2/3 x 1/2  
Galvanized Corr. Sheet

# Control Room Shelter

Roof Load/Running In. of Rafter

$$\begin{array}{r} 5.0 \text{ Lb Snow} \\ .6 \\ \hline 5.6 \text{ Lb} \end{array}$$

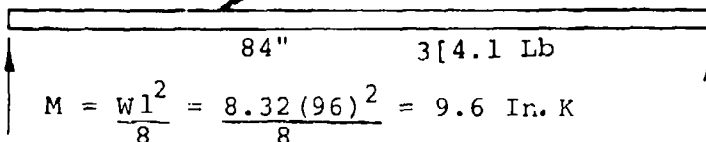


$$M = \frac{Wl^2}{8} = \frac{5.6(180)^2}{8} = 22.7 \text{ In.-Lb}$$

$$f = \frac{M}{S} = \frac{22.7}{1.1} \text{ In. K} = 20.6 \text{ KSI}$$

Wind Load/Stud

$$100 \text{ Lb/Ft} \quad 8.32 \text{ Lb/In.}$$

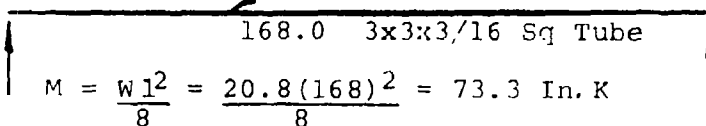


$$M = \frac{Wl^2}{8} = \frac{8.32(96)^2}{8} = 9.6 \text{ In. K}$$

$$f = \frac{M}{S} = \frac{9.6}{1.1} = 8.73 \text{ KSI}$$

Sill Wind Load

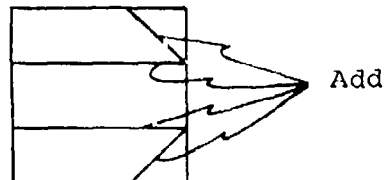
$$250 \text{ Lb/Ft.} \quad 20.8 \text{ Lb/Ft}$$



$$M = \frac{Wl^2}{8} = \frac{20.8(168)^2}{8} = 73.3 \text{ In. K}$$

$$f = \frac{M}{S} = \frac{73.3}{1.732} = 42.3 \text{ KSI}$$

Add Horizontal Braces @ 8-Ft Level





APPENDIX II  
DRAWINGS - DESIGN LAYOUTS

The integrated test rig is described by the following drawings which are provided as the listed figures.

<u>Figure</u>	<u>Drawing No.</u>	<u>Title</u>
49	SK301-11277	Structures Installation - HLH Hoist Test
50	SK301-11304	HLH Hoist Tower Assembly
51	SK301-11302	HLH Overhead Assembly
52	X72-002-AS-YD3/1	Site Plan - Paving and Utilities, Plan and Sections
53	X72-002-AS-YD3/2	Foundations - Plans, Sections and Details
54	X73-003-AS-YD3/2	Pneumatic Power Generator Shelter HLH/ATC Cargo Handling System
55	X73-003-AS-YD3/3	Control Room Plans, Elevations and Section
56	X73-003-AS-YD3/4	Control Room Section and Details
57	V73-G03-AS-YD3	Removable Handrail Details
58	X73-003-M-YD3/1	Piping Arrangement, Test Tower Top Section
59	X73-003-M-YD3/2	Piping Arrangement, Test Tower Base
60	X73-003-M-YD3/3	Fuel Piping Arrangement - PPG Unit
61	V73-001-E-YD3	Electrical Single Line Diagram
62	X73-003-E-YD3/1	Electrical HLH/ATC Cargo Handling Test Rig
63	X73-003-E-YD3/2	Control Room Electrical Layout
64	SK301-11564	Load Controlling Crewman Platform - Integrated Test Rig

<u>Figure</u>	<u>Drawing No.</u>	<u>Title</u>
65	SK301-11676	System Test - Drawing Tree
66	ST40972	Lifting Sling, Hoist/Module
67	ST51273-1	Hoist Lifting Fixture
68	SK301-11694	Integrated Test Rig - System Wiring
69	ST30861	Instrumentation Drawing Tree

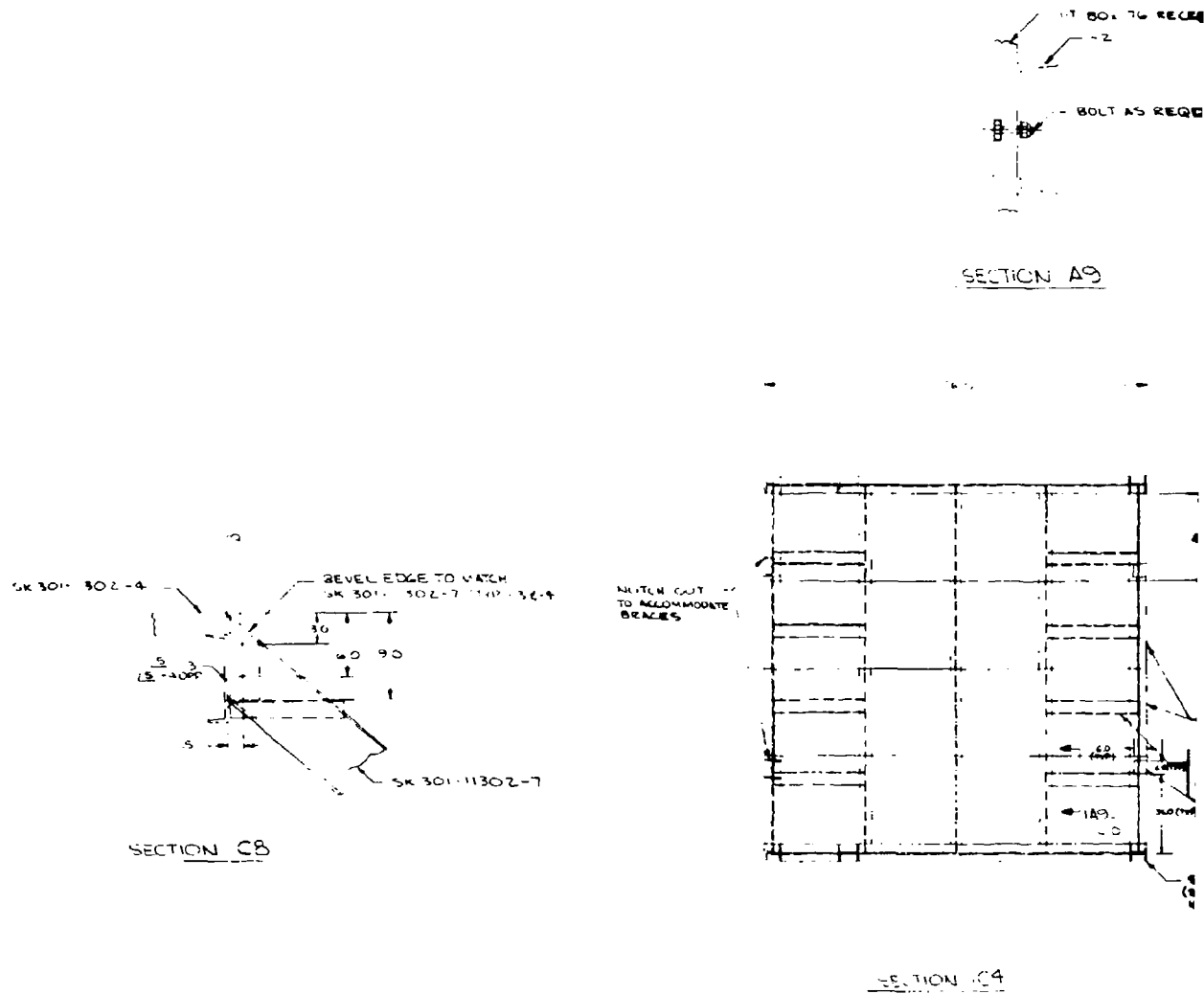
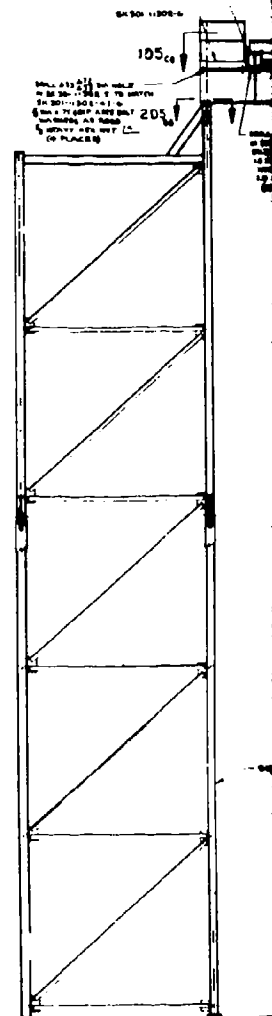
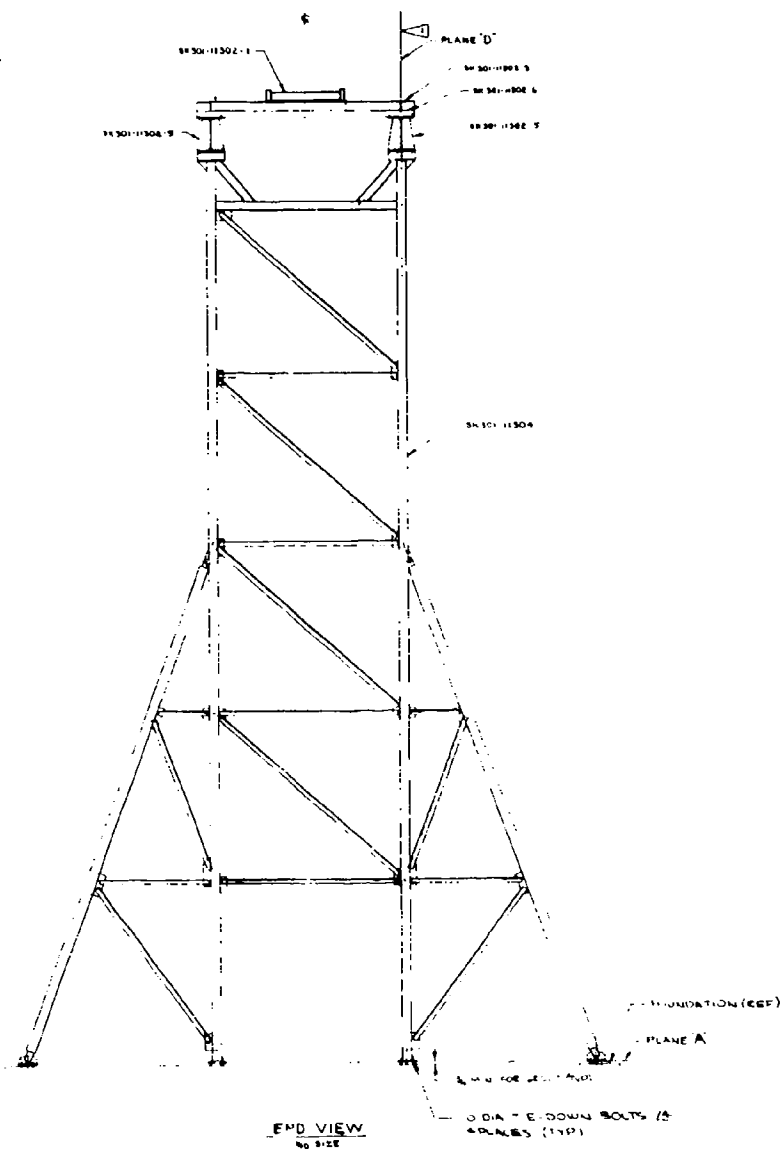
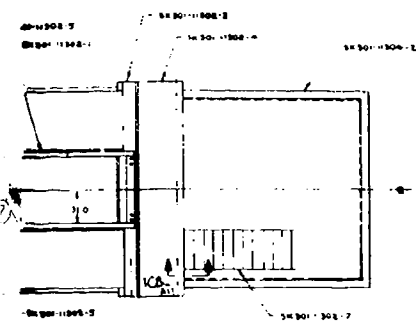


Figure 49. Structures Installation - HLH Hoist Test.

A





✓

2. SEE VERTICAL WORK STATEMENT FOR FINISH PAINT COLOR REQUIREMENTS

L 12. ANGLES, PLANE C, PLANE D TO BE PERPENDICULAR TO PLANE A WITHIN  $\pm 0.15$  DEG

23. CHIM AS REQD TO PROVIDE PROPER ALIGNMENT OF SK 301-11302-1 HOIST MODULE

12 SH.M.F. AS REQD TO INSURE PROPER ALIGNMENT OF SK 201-11302-S

12 TORQUE PER AISI SPECIFICATIONS.

⑤ WELD TO SK 301- 1302-4 AND BOLT TO SK 301- 302-7 WITH  $\frac{1}{2}$ " DIA. BOLTS.

[illegible][illegible][illegible]

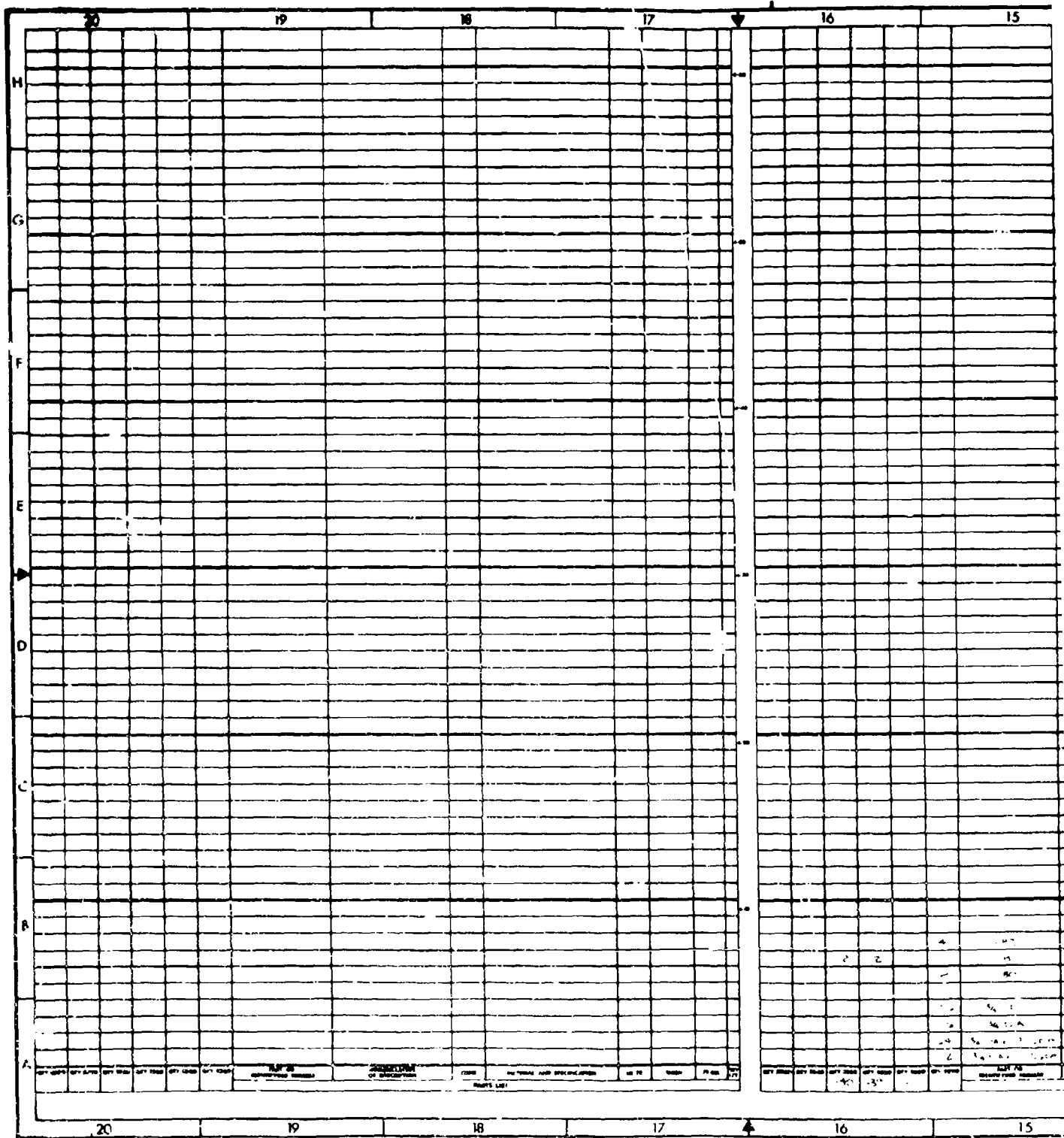


Figure 50. HLH Hoist Tower Assembly (Sheet 1)

A

[illegible]

B



[illegible]







3/4" DIA HOLE THRU  
1/2" DIA. 15'2" R.D.  
A 15'2" STEEL  
CONCRETE BIT  
FOR A HEAVY DUTY  
1/2" DIA FLAT ANCHOR  
(144)

25. 11 24 - 1994

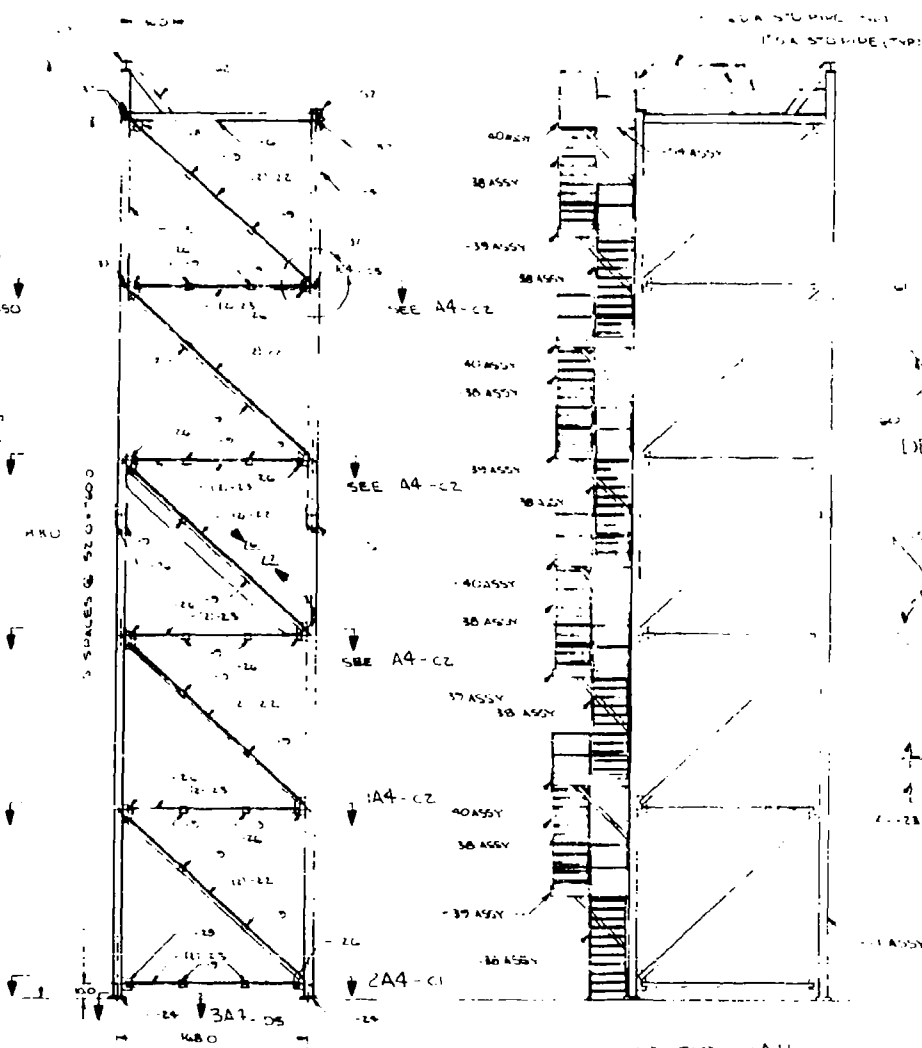
1A6-24

-1 TOWER ASSY

D

[illegible]

DETAIL IC7



DETAIL C

DETAIL IC 2 (TYPE)

SECTION 1A11

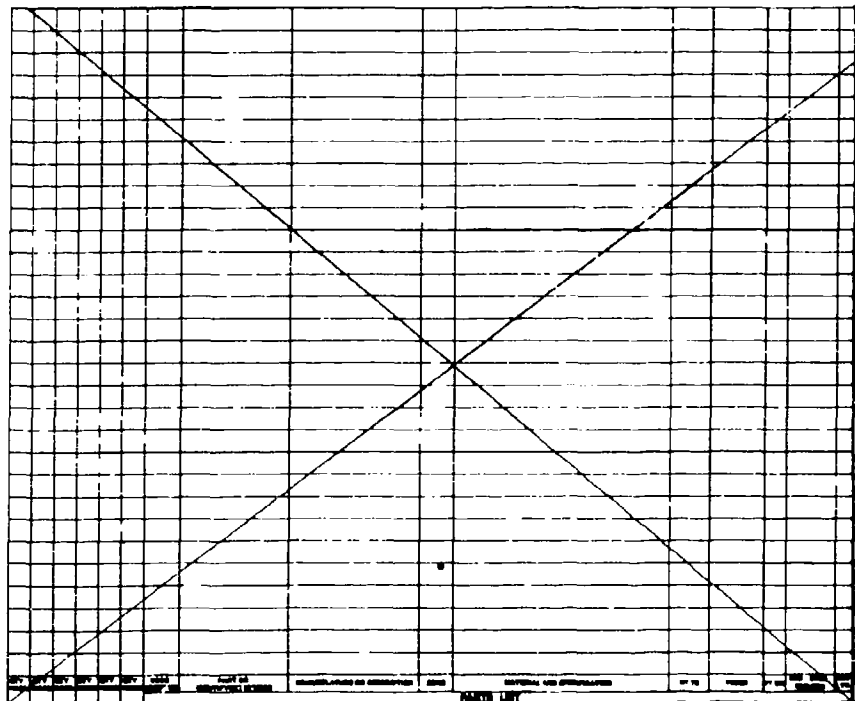
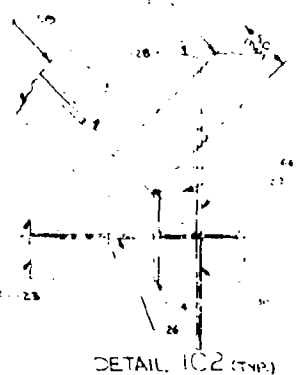
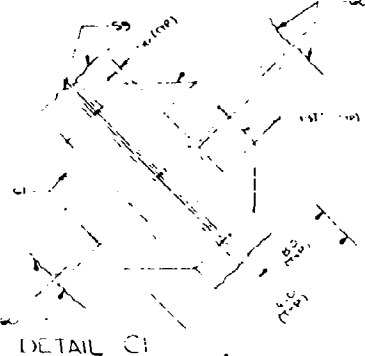
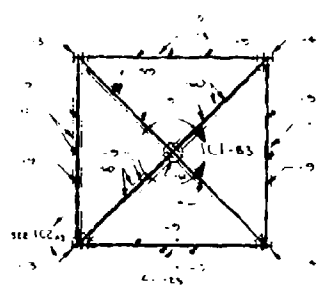
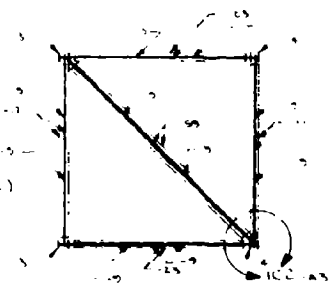
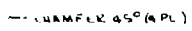
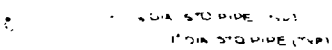
VIEW IAC.

B ( 9, 'R) 7 8551, NOT 'SHOWN  
 | N T U S V I E W F O R C L A R I T Y )

5K-301-11304-12

D

[illegible]

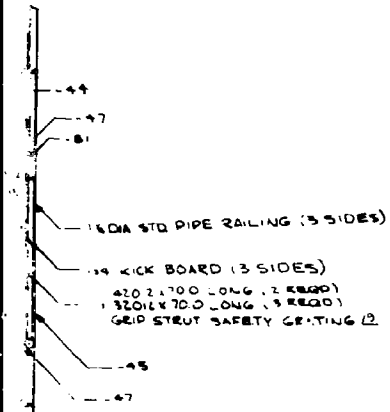
[illegible]

1. NAME OF THE 2. ADDRESS 3. CITY 4. STATE 5. ZIP 6. PHONE 7. FAX 8. E-MAIL 9. WWW 10. OTHER		11. NAME OF THE 12. ADDRESS 13. CITY 14. STATE 15. ZIP 16. PHONE 17. FAX 18. E-MAIL 19. WWW 20. OTHER		21. NAME OF THE 22. ADDRESS 23. CITY 24. STATE 25. ZIP 26. PHONE 27. FAX 28. E-MAIL 29. WWW 30. OTHER		31. NAME OF THE 32. ADDRESS 33. CITY 34. STATE 35. ZIP 36. PHONE 37. FAX 38. E-MAIL 39. WWW 40. OTHER		41. NAME OF THE 42. ADDRESS 43. CITY 44. STATE 45. ZIP 46. PHONE 47. FAX 48. E-MAIL 49. WWW 50. OTHER		51. NAME OF THE 52. ADDRESS 53. CITY 54. STATE 55. ZIP 56. PHONE 57. FAX 58. E-MAIL 59. WWW 60. OTHER		61. NAME OF THE 62. ADDRESS 63. CITY 64. STATE 65. ZIP 66. PHONE 67. FAX 68. E-MAIL 69. WWW 70. OTHER		71. NAME OF THE 72. ADDRESS 73. CITY 74. STATE 75. ZIP 76. PHONE 77. FAX 78. E-MAIL 79. WWW 80. OTHER		81. NAME OF THE 82. ADDRESS 83. CITY 84. STATE 85. ZIP 86. PHONE 87. FAX 88. E-MAIL 89. WWW 90. OTHER		91. NAME OF THE 92. ADDRESS 93. CITY 94. STATE 95. ZIP 96. PHONE 97. FAX 98. E-MAIL 99. WWW 100. OTHER	
---	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	--	---	--

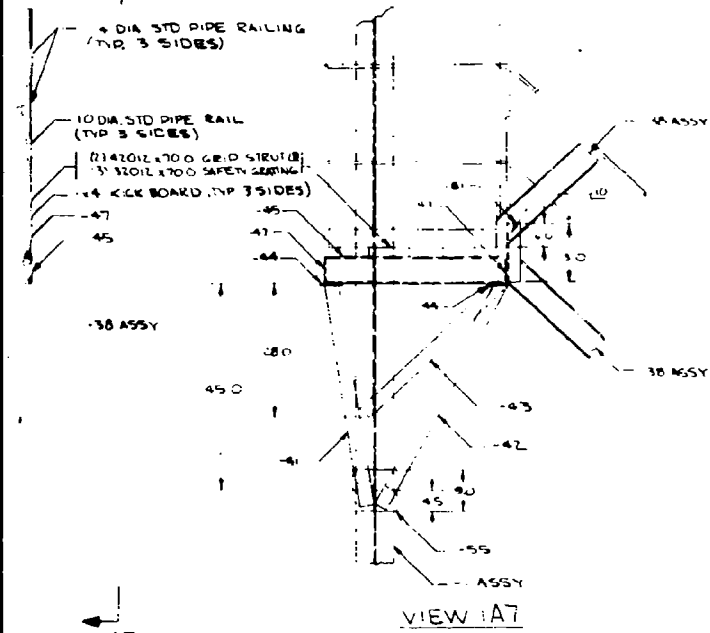
E



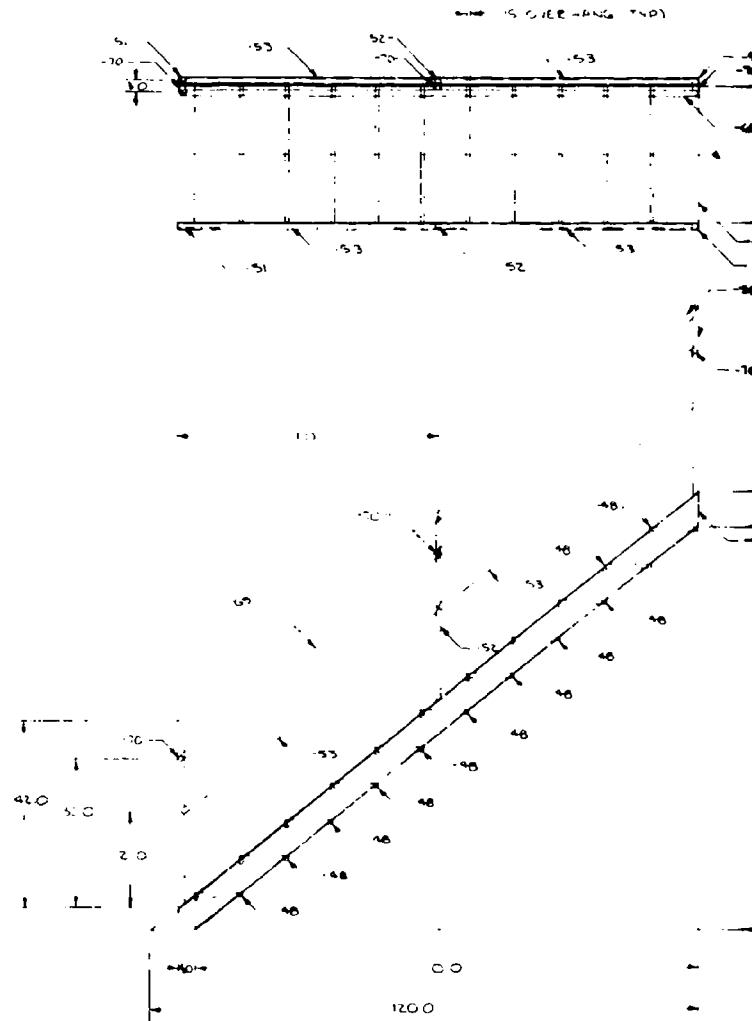




IC7-C1



1A7-AG



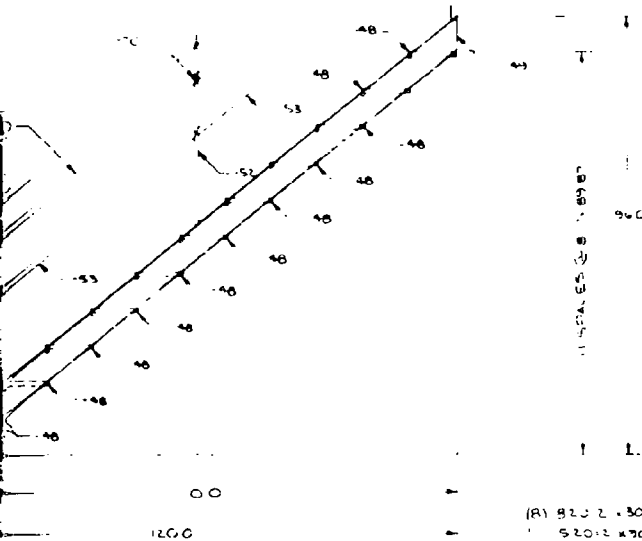
38 STA. RWAY ASSY

ALL WELDED CONSTRUCTION

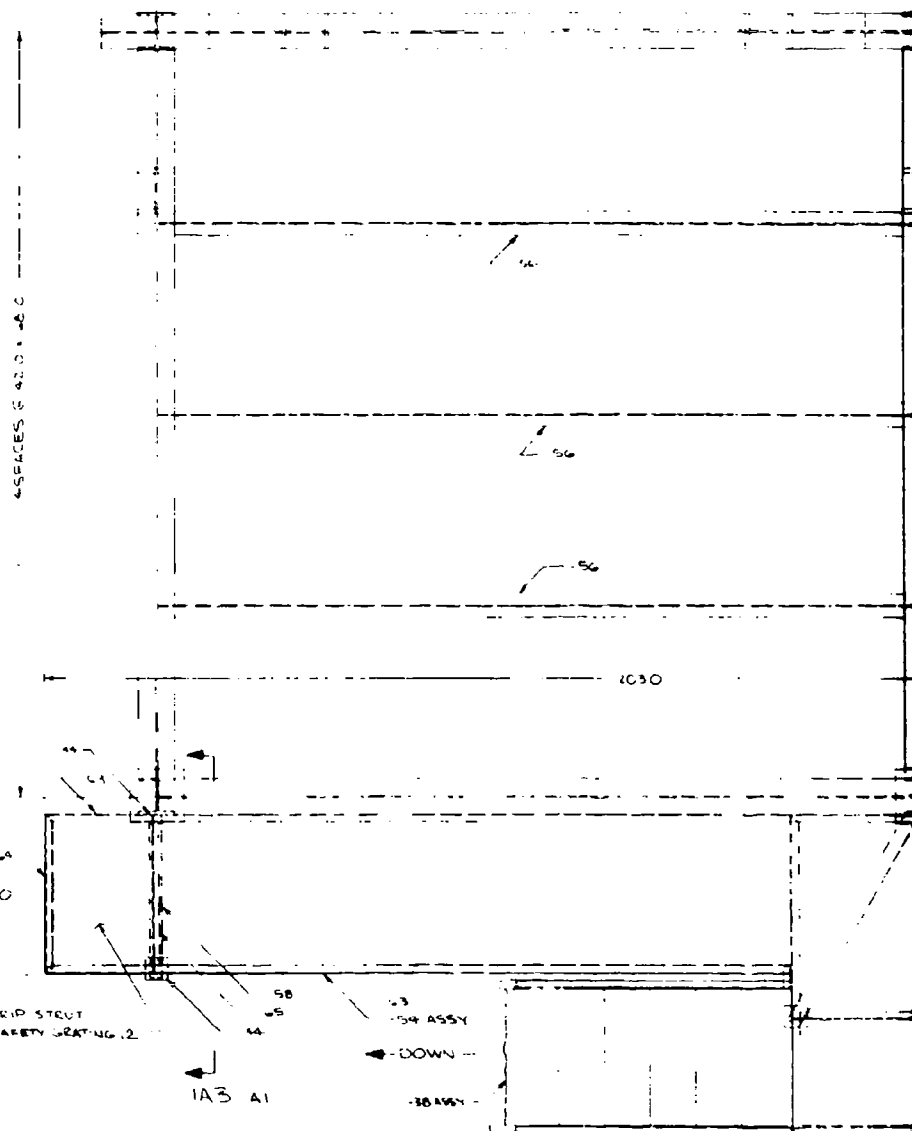
SK-301-11304 13

B

C

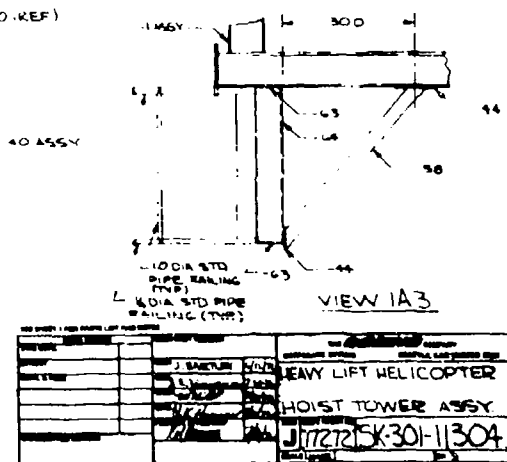
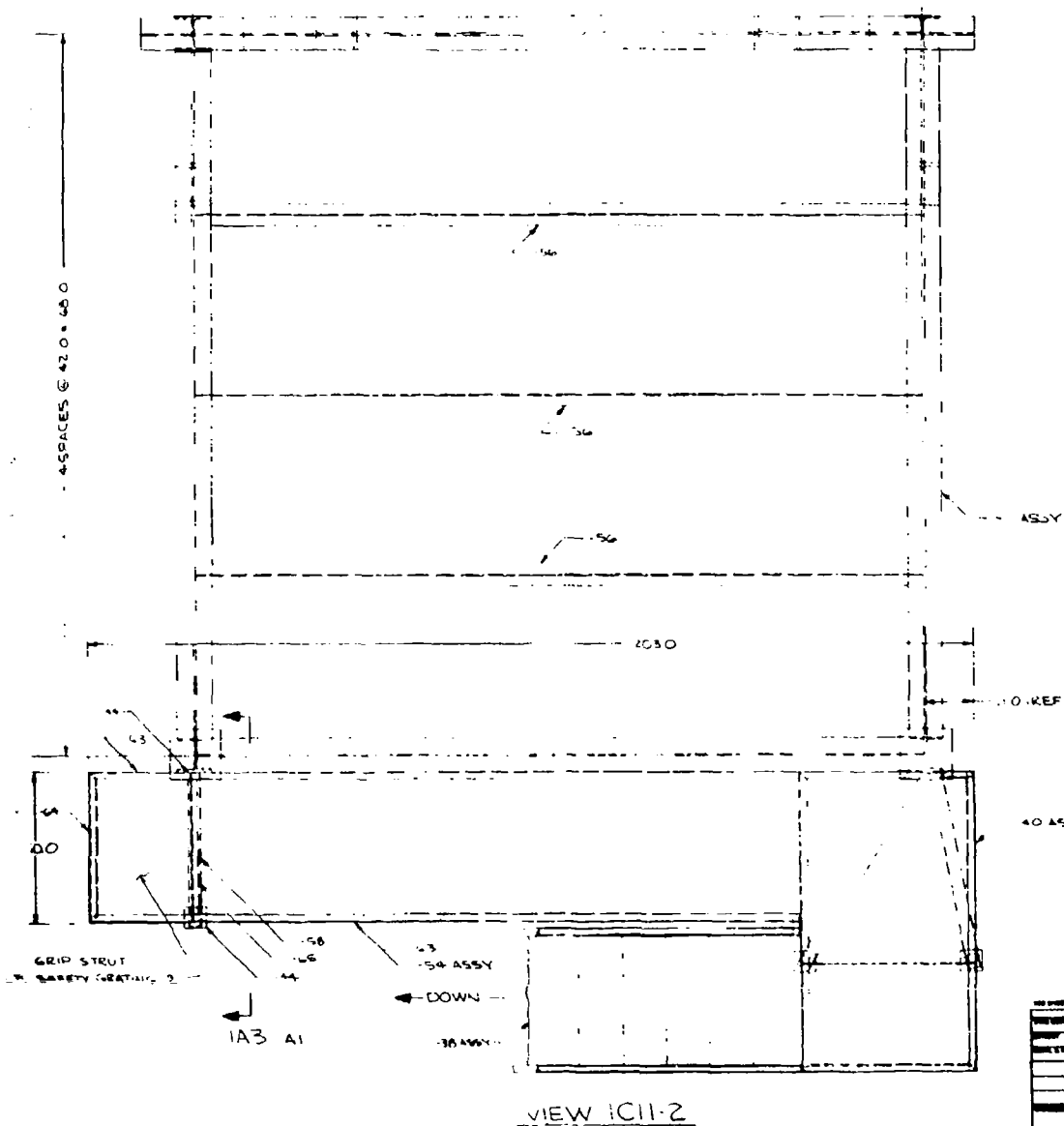


AL - NE - JED CONSTRUCTION



VIEW : C11-2

SK-301-11304-3



REV	DATE	BY	CHKD	APP'D	DESCRIPTION
1		J. BAYLUM			HEAVY LIFT HELICOPTER
2					HOIST TOWER ASSY
3					J1727215K-301-11304



1. PREPARE THE OIL
2. BREAK ALL THE
3. BALL WELDS TO
4. A 1" STEEL W/
5. MAXIMUM STRAIN
6. FLAME CUTTING
7. TO BE FLAME C
8. 1 IN PER A SC
9. AT RECTION OF 1
10. STRUCTURAL TOL

U2 SET FOR KICK

4.

EXFUT, ASSETS)

SP.  
... 6.7 HUND - ONE ? PLACES (TOP PLANE ONLY)

• 4. 11. 1960. 20. 1. 1961. SAFETY MEETING.

1-1-76 - C 11 5227 JAEY GRAY N 10

PLACES FOR RAISE ONLY

501 - 160 BRD 21201 2147 N

112V - VOLTAGE ASSY

24 301-300

BANK	INVT	LODR
------	------	------

~~SECRET~~

**100**

100

—

B

2. SEE DIM. FOR DIM. NOT SHOWN.
3. BREAK ALL SHARP CORNERS.
4. ALL WELDS TO BE MADE IN ACCORDANCE WITH AISC AND AWS STANDARDS.
5. ALL STEEL WILL BE A36 OR APPROVED EQUIVALENT.
6. MAXIMUM STRAIGHTNESS DEVIATION =  $1/1000$  OF THE OVERALL LENGTH.
7. FRAME CUTTING TO BE PERMISSIBLE ON ALL PARTS, EXCEPT THAT THE HOLES ARE NOT TO BE FRAME CUT.
8. PAINT PER AISC SPECIFICATION (FIELD PAINT) SEE VERTICAL WORK STATEMENT FOR PAINT REQUIREMENTS.
9. DETECTION OF BEAM ASSESS ANY EXISTING DAMAGE TO BE UPWARD.
10. STRUCTURAL TOLERANCES TO BE PER AISC STANDARD UNLESS OTHERWISE NOTED ON DWG.

- 
- Hand-drawn sketch of a rectangular structure, possibly a building or container, with dimensions and labels. The structure is 300 units long and 40 units high. It has a flat roof and vertical supports. Labels include "300" for length, "40" for height, and "40" for a small vertical dimension on the right. A note at the bottom right says "TOTAL LENGTH 300 LONG (APPROXIMATE)". There are also some numbers like "40", "30", and "40" scattered around the drawing.

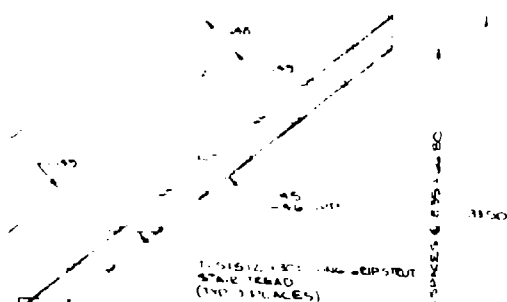
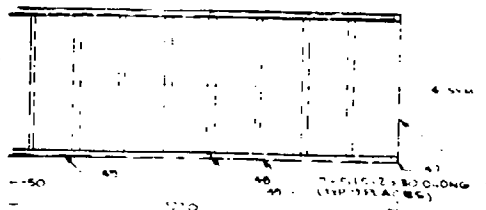
1.51512 13 0 100-EPSTON  
STA-2-READ  
(100 111 MFS)

FAIRWAY ROSSY  
A. GLENN L. COMPTON  
A. GLENN L. COMPTON

					✓	6	STOILY 760	GUSSET
					✓	12	STOILY 480	PLATE
					✓	2	STOILY 960	
					✓	9	STOILY 410	CLIP BRKT
					✓	1	T-SILLYWOOD	CLIP BRKT
					✓	2	1-6 SILLYWOOD	SPR. 7500
					✓	1	1-6 SILLYWOOD	SILLYWOOD
					✓	1	NOIA = 90 FT	STD PIPE
					✓	1	100A = 14 FT	STD PIPE
					✓	2	-60	PLATE
					✓	4	-45	RAILING
					✓	7	-48	RAILING
					✓	2	-47	RAILING
					✓	1	-46	OPD - 48
					✓	1	-46	CHANNEL
					✓	3	-48	MOUNTING
					✓	4	-45	CHANNEL
					✓	2	-45	CHANNEL
					✓	10	-47	GUSSET
					✓	2	-40	CHANNEL
					✓	2	-45	CHANNEL
					✓	2	-36	BEAM
					✓	2	-87	BEAM
					✓	2	-4	BEAM
					✓	2	-95	RAILING
					✓	2	-4	RAILING
					✓	16	-30	GUSSET
					✓	4	-11	CHANNEL
					✓	2	-11	BEAM
					✓	2	-30	PLATE
					✓	4	-79	PLATE
					✓	2	-28	PLATE
					✓	8	-27	GUSSET
					✓	8	-25	GUSSET
					✓	2	-26	BEAM
					✓	4	-24	PLATE
					✓	1	-19	WEB
					✓	4	-8	WEB
					✓	12	-11	GUSSET
					✓	8	-16	GUSSET
					✓	4	-5	GUSSET
					✓	1	-4	PAD
					✓	2	-13	UPPER FLANGE
					✓	2	-1	UPPER FLANGE
					✓	2	-1	BEAM
					✓	7	-	STAIRWAY
					✓	6	-	PLATFORM
					✓	2	-5	OVERHEAD
					✓	4	-	PLATFORM
					✓	2	-3	WORK PLANT
					✓	1	-6	DAINTY MAN
					✓	2	-1	MOIST MOUNT

NAME	LAST	FIRST	MIDDLE	DATE OF BIRTH	DATE OF DEATH	PLACE OF BIRTH	PLACE OF DEATH	CAUSE OF DEATH	DATE OF BURIAL	PLACE OF BURIAL	DATE OF CREMATION	PLACE OF CREMATION
1	SMITH	JOHN	DAVID	1915	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
2	JOHNSON	MARY	ELIZABETH	1920	1990	NEW YORK	NEW YORK	OLD AGE	1990	NEW YORK		
3	WILLIAMS	JOHN	DAVID	1925	1980	NEW YORK	NEW YORK	HEART DISEASE	1980	NEW YORK		
4	BROWN	JOHN	DAVID	1930	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
5	DAVIS	JOHN	DAVID	1935	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
6	GARCIA	JOHN	DAVID	1940	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
7	RODRIGUEZ	JOHN	DAVID	1945	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
8	WILLIAMS	JOHN	DAVID	1950	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
9	WILLIAMS	JOHN	DAVID	1955	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
10	WILLIAMS	JOHN	DAVID	1960	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
11	WILLIAMS	JOHN	DAVID	1965	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
12	WILLIAMS	JOHN	DAVID	1970	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
13	WILLIAMS	JOHN	DAVID	1975	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
14	WILLIAMS	JOHN	DAVID	1980	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
15	WILLIAMS	JOHN	DAVID	1985	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
16	WILLIAMS	JOHN	DAVID	1990	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
17	WILLIAMS	JOHN	DAVID	1995	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
18	WILLIAMS	JOHN	DAVID	2000	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
19	WILLIAMS	JOHN	DAVID	2005	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
20	WILLIAMS	JOHN	DAVID	2010	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
21	WILLIAMS	JOHN	DAVID	2015	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
22	WILLIAMS	JOHN	DAVID	2020	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
23	WILLIAMS	JOHN	DAVID	2025	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
24	WILLIAMS	JOHN	DAVID	2030	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
25	WILLIAMS	JOHN	DAVID	2035	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
26	WILLIAMS	JOHN	DAVID	2040	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
27	WILLIAMS	JOHN	DAVID	2045	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
28	WILLIAMS	JOHN	DAVID	2050	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
29	WILLIAMS	JOHN	DAVID	2055	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
30	WILLIAMS	JOHN	DAVID	2060	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
31	WILLIAMS	JOHN	DAVID	2065	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
32	WILLIAMS	JOHN	DAVID	2070	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
33	WILLIAMS	JOHN	DAVID	2075	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
34	WILLIAMS	JOHN	DAVID	2080	1985	NEW YORK	NEW YORK	HEART DISEASE	1985	NEW YORK		
35	WILLIAMS	JOHN	DAVID									

BOARDS, AS REQD



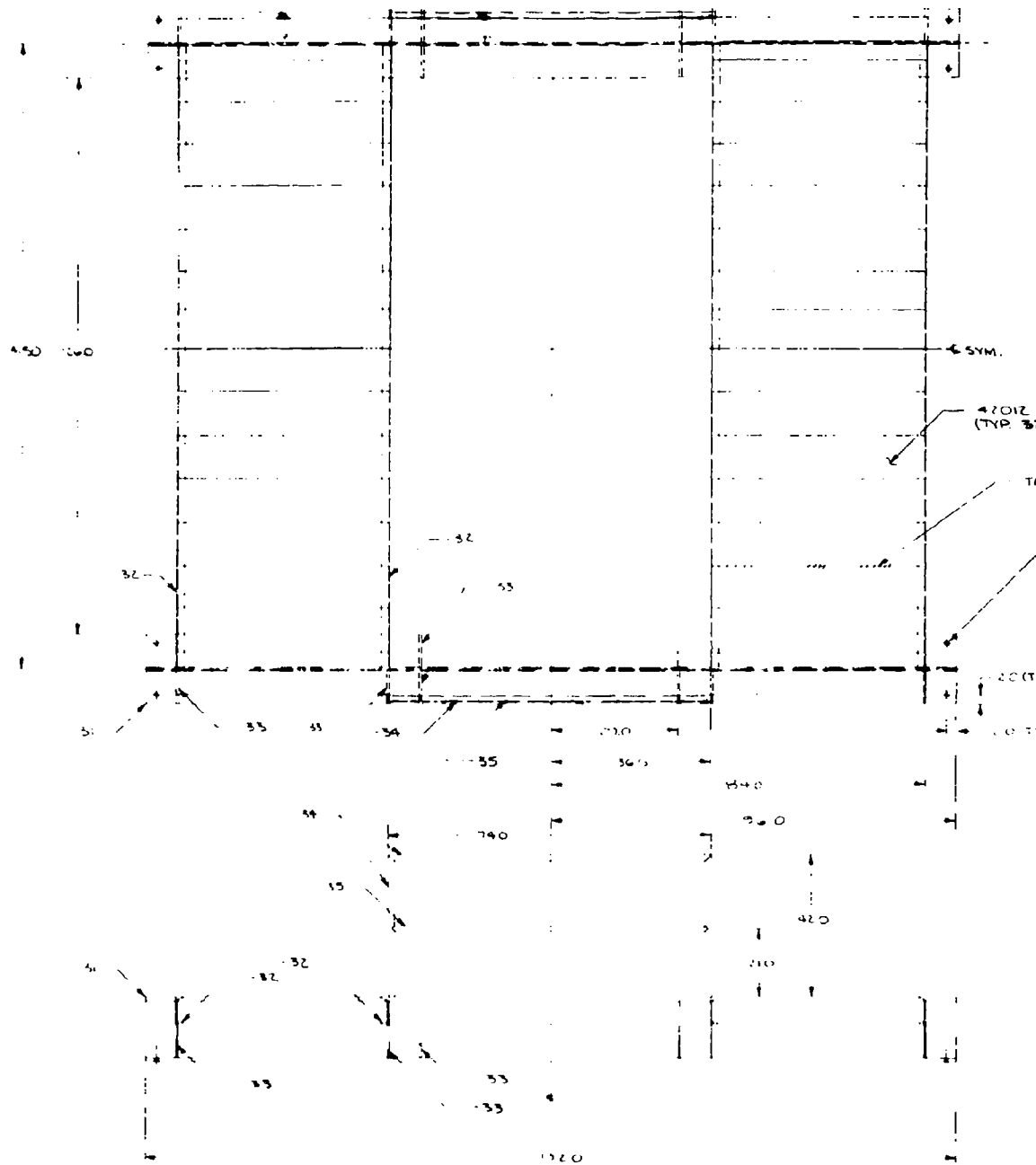
1. AIRWAY LOSS  
A. CLOSED CIRCULATION  
A. MIN. 1000

[illegible][illegible]

OVERHEAD ASSY.  
SK-30H11302



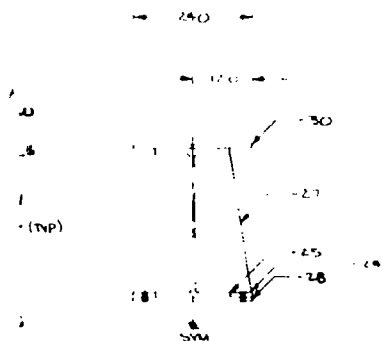




Ø1  
Ø18 DIA HOLE  
THRU LWR FLANGE 12"  
(TYP 4 PLACES EACH END)

47012  
(TYP 3)

20 (TYP)

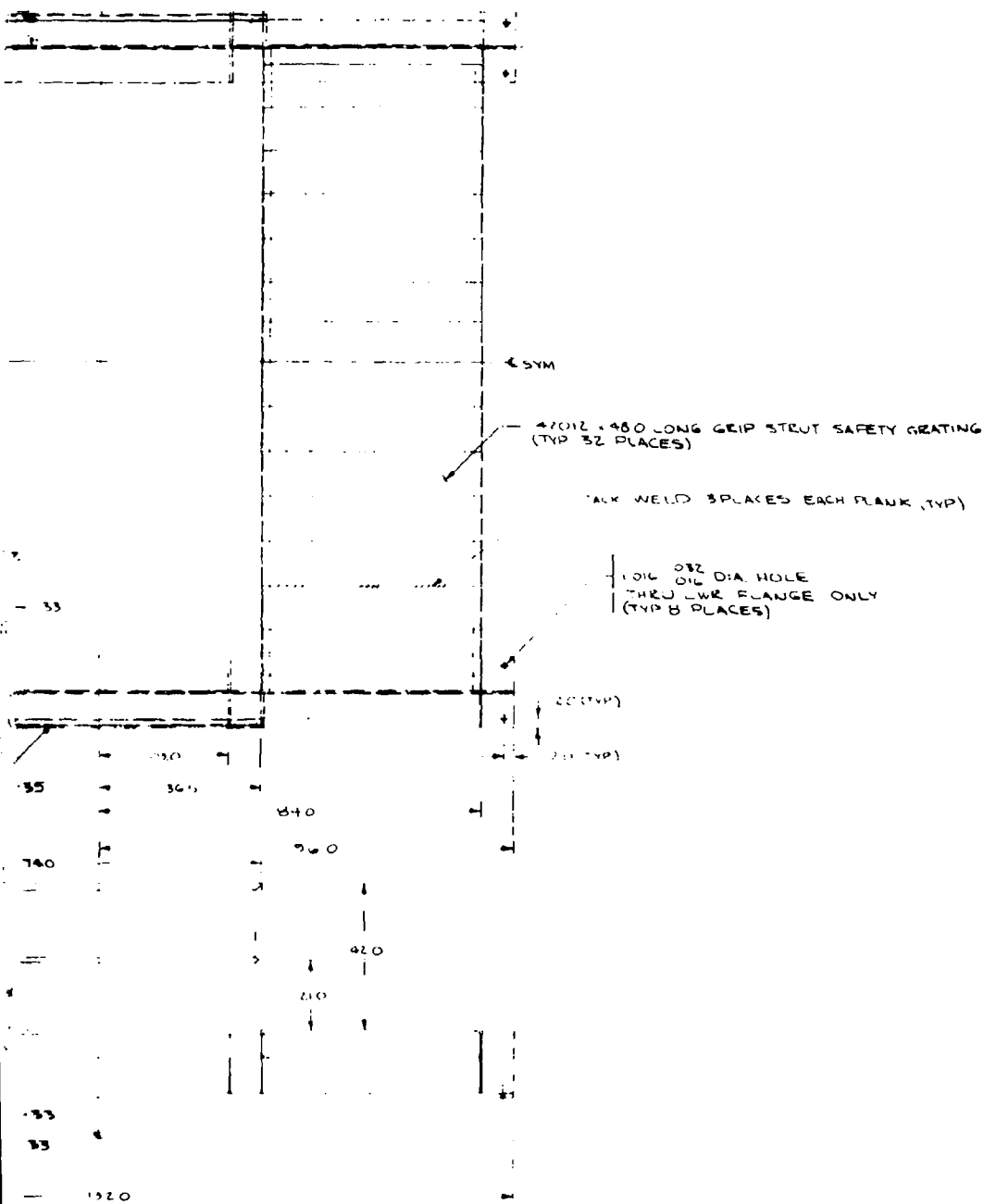


3 WORK PLATFORM ASSY

B

SK-30-11302 2

C

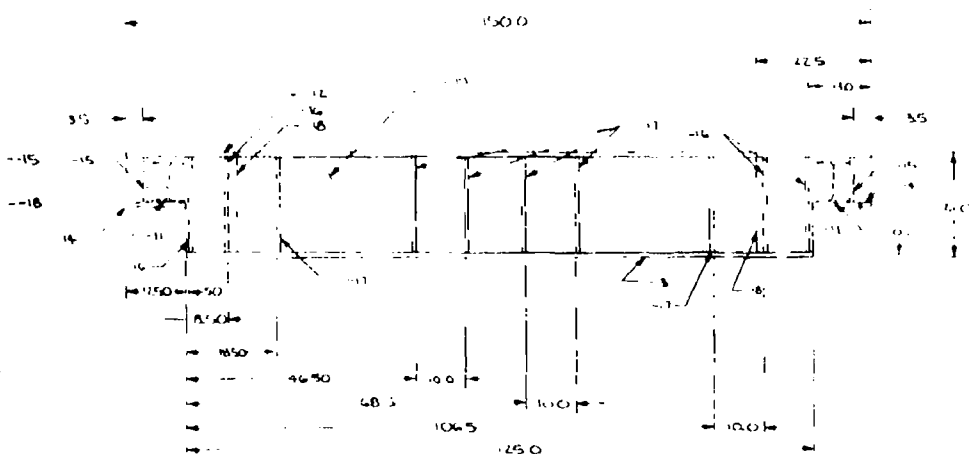
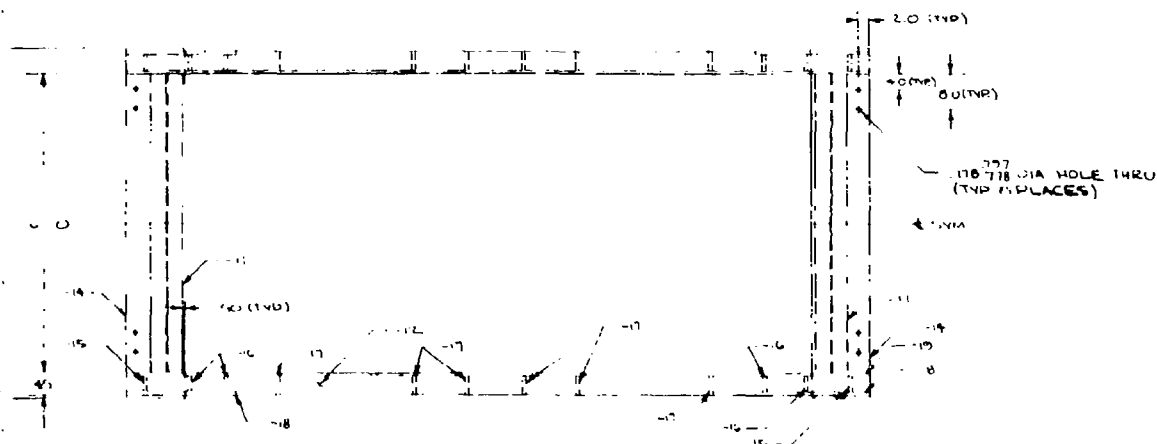


- 3 WORK PLATFORM ASSY

C

D





1 FWD HOIST MOUNTING ASSY  
ALL WELDED CONST 1/4 MM WELD

NO. 1		HEAVY LIFT HELICOPTER	
STALEY		OVERHEAD ASSY	
J77272		SK-301-11302	
1		2	

✓



## Plan and Sections

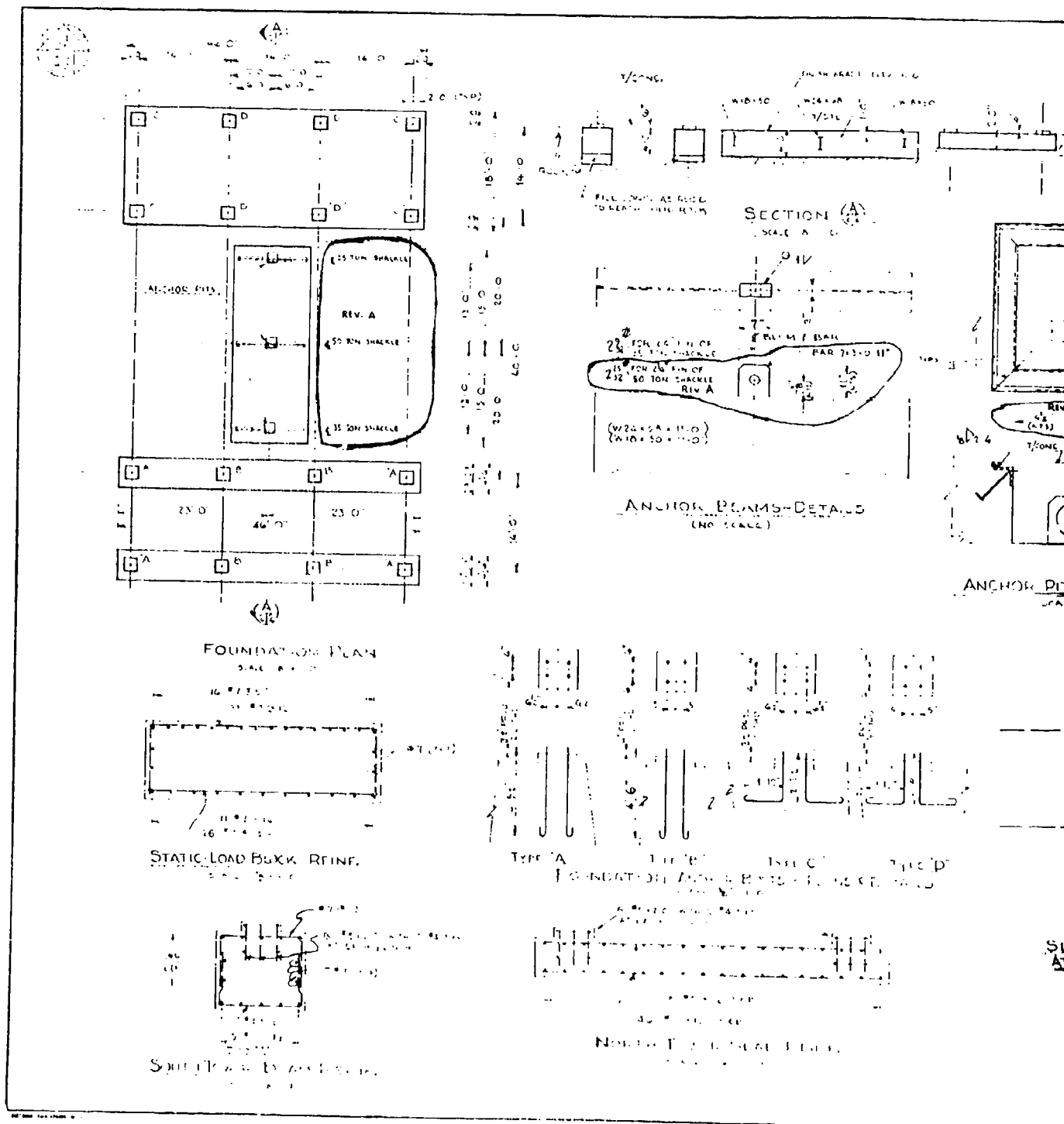
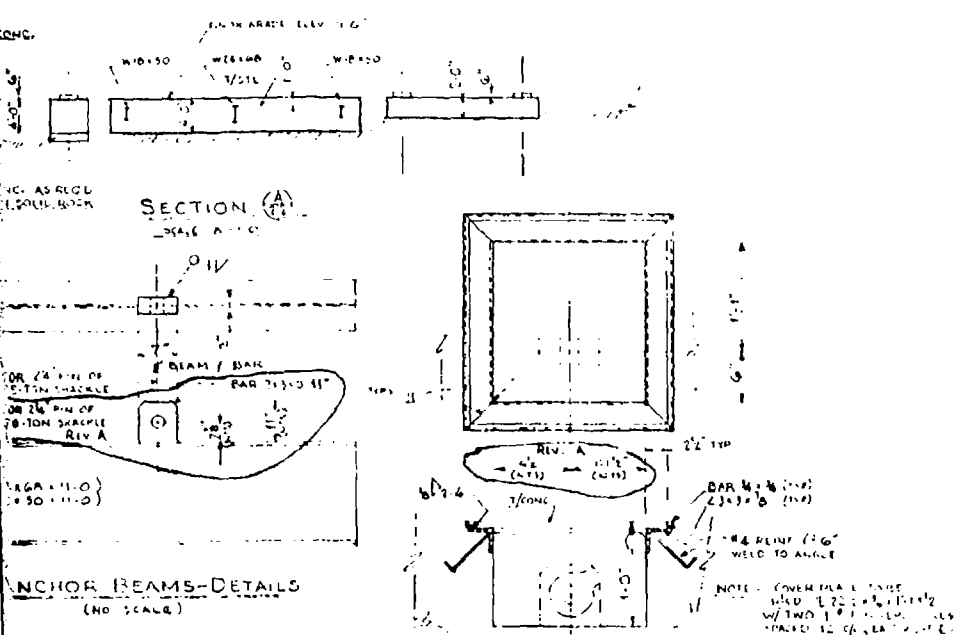


Figure 53. Foundations - Plans, Sections and Details.

Preceding page blank

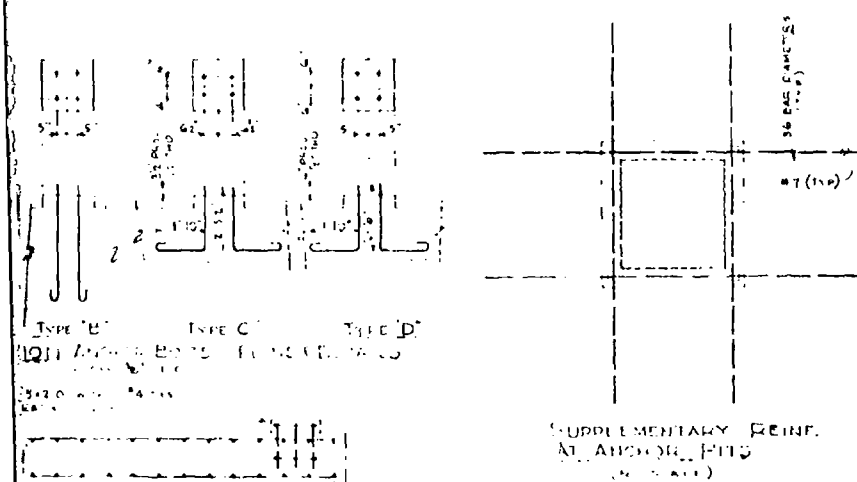
A

# ANCHOR BEAMS - DETAILS



1. ALL REINFORCEMENT SHALL BE CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL, AND THE AISC CODE OF PRACTICE FOR THE CONSTRUCTION OF STEEL STRUCTURES.
2. ALL REINFORCEMENT SHALL BE CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL, AND THE AISC CODE OF PRACTICE FOR THE CONSTRUCTION OF STEEL STRUCTURES.
3. ALL REINFORCEMENT SHALL BE CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL, AND THE AISC CODE OF PRACTICE FOR THE CONSTRUCTION OF STEEL STRUCTURES.
4. ALL REINFORCEMENT SHALL BE CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL, AND THE AISC CODE OF PRACTICE FOR THE CONSTRUCTION OF STEEL STRUCTURES.
5. ALL REINFORCEMENT SHALL BE CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL, AND THE AISC CODE OF PRACTICE FOR THE CONSTRUCTION OF STEEL STRUCTURES.
6. ALL REINFORCEMENT SHALL BE CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL, AND THE AISC CODE OF PRACTICE FOR THE CONSTRUCTION OF STEEL STRUCTURES.
7. ALL REINFORCEMENT SHALL BE CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL, AND THE AISC CODE OF PRACTICE FOR THE CONSTRUCTION OF STEEL STRUCTURES.
8. ALL REINFORCEMENT SHALL BE CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL, AND THE AISC CODE OF PRACTICE FOR THE CONSTRUCTION OF STEEL STRUCTURES.
9. ALL REINFORCEMENT SHALL BE CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL, AND THE AISC CODE OF PRACTICE FOR THE CONSTRUCTION OF STEEL STRUCTURES.
10. ALL REINFORCEMENT SHALL BE CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL, AND THE AISC CODE OF PRACTICE FOR THE CONSTRUCTION OF STEEL STRUCTURES.
11. ALL REINFORCEMENT SHALL BE CONFORM TO THE REQUIREMENTS OF THE LATEST EDITION OF THE AISC SPECIFICATION FOR STRUCTURAL STEEL, AND THE AISC CODE OF PRACTICE FOR THE CONSTRUCTION OF STEEL STRUCTURES.

Reproduced from  
best available copy.



<p>DESIGNED BY: [ ]</p> <p>CHECKED BY: [ ]</p> <p>APP. BY: [ ]</p> <p>DATE: [ ]</p> <p>SCALE: AS NOTED</p>	<p>TITLE: [ ]</p> <p>PROJECT: [ ]</p> <p>SECTION: [ ]</p> <p>PLANT ENGINEERING</p> <p>DATE: [ ]</p> <p>DWG. NO. [ ]</p>
--	---

Details.

B



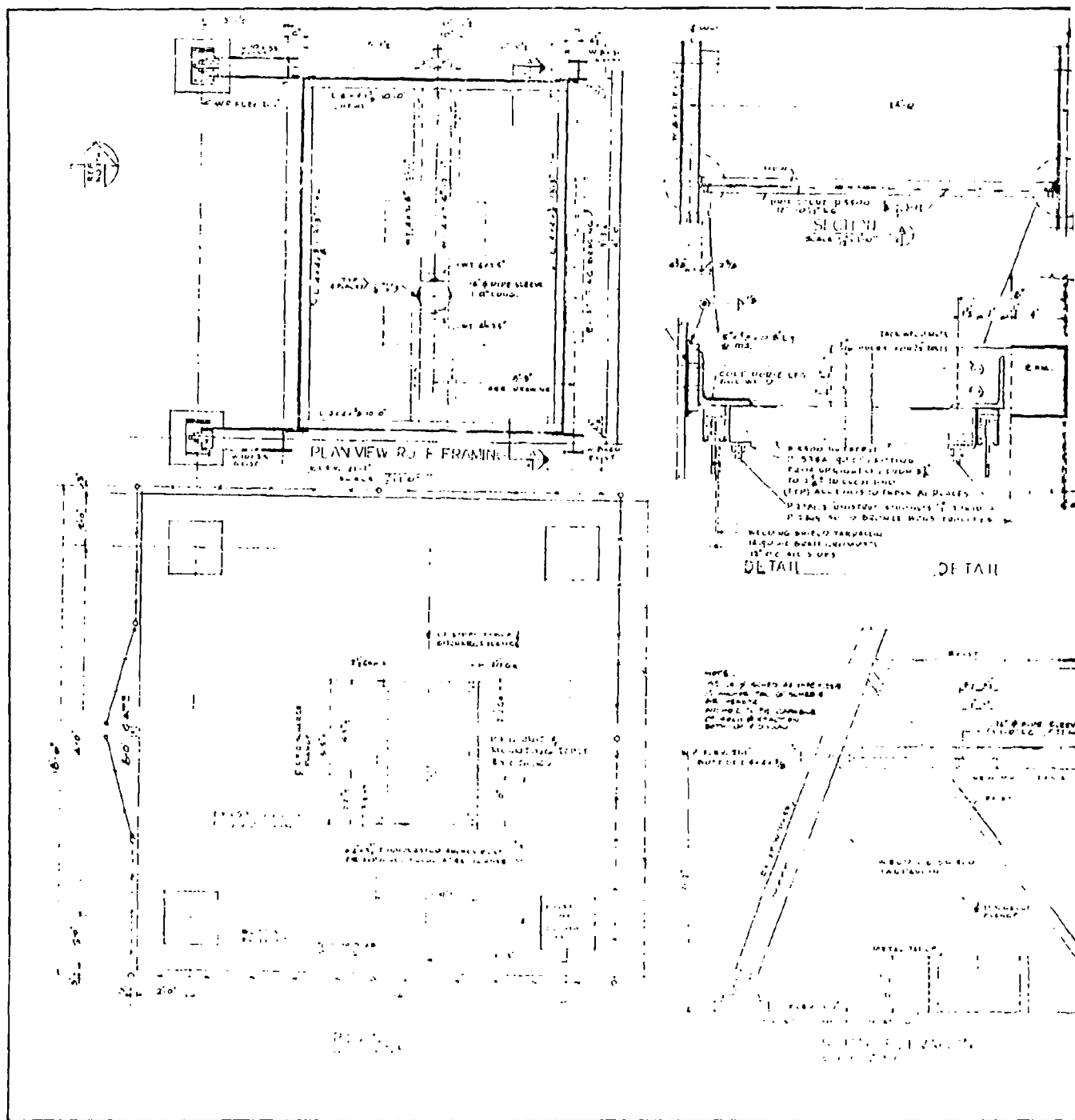


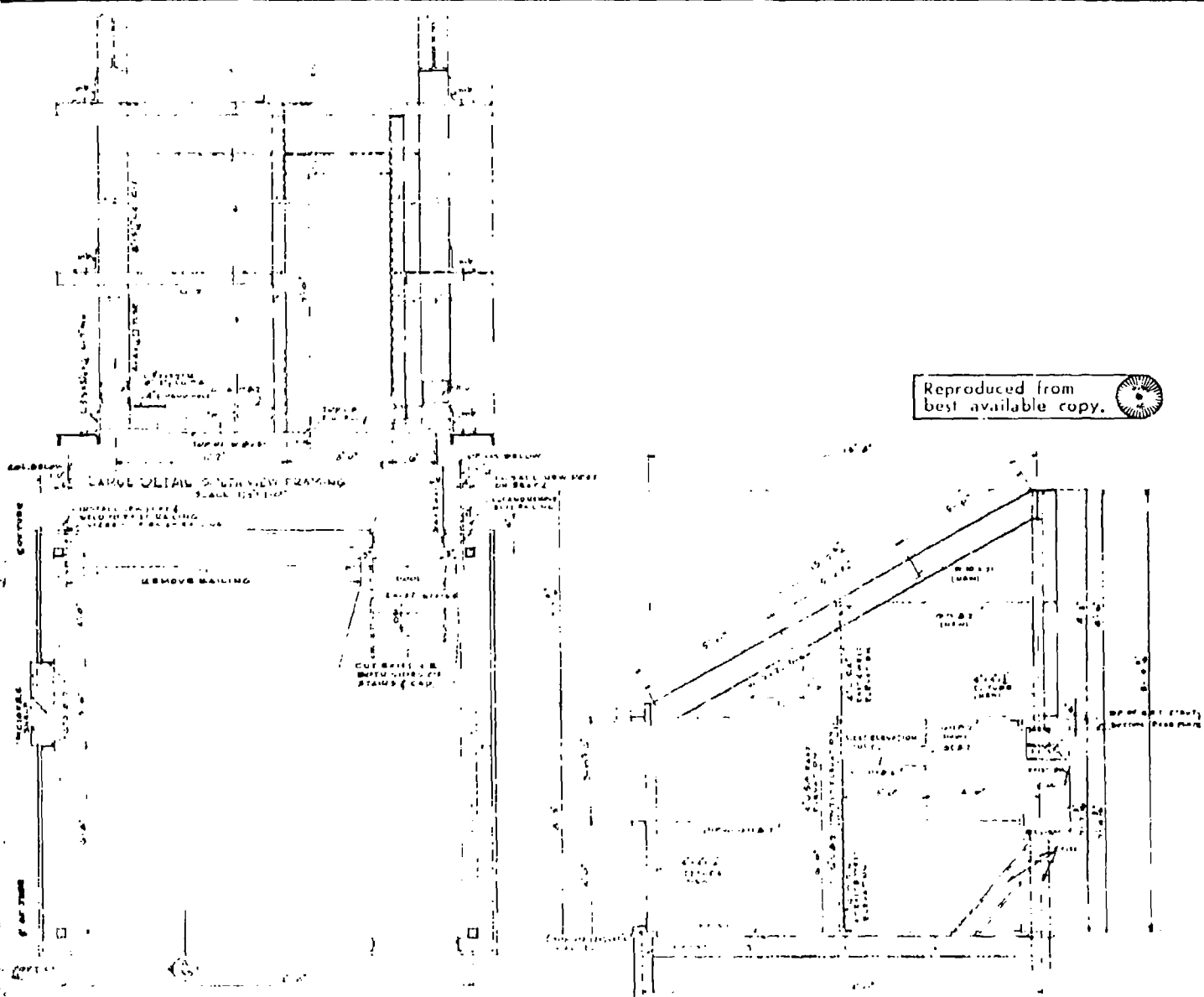
Figure 54. Pneumatic Power Generator Shelter, HLH/ATC Cargo Handling System.

Preceding page blank





Reproduced from  
best available copy.



SECTION  
SCALE 1/4\"/>

DRAWN BY		DATE		TITLE		ED. (REV.)		CUT NO.	
APP. BY		DATE		CONTROL ROOM					
APP. BY		DATE		PLANT ENGINEERING					
APP. BY		DATE		BOEING					
APP. BY		DATE		DOW NO. 13					

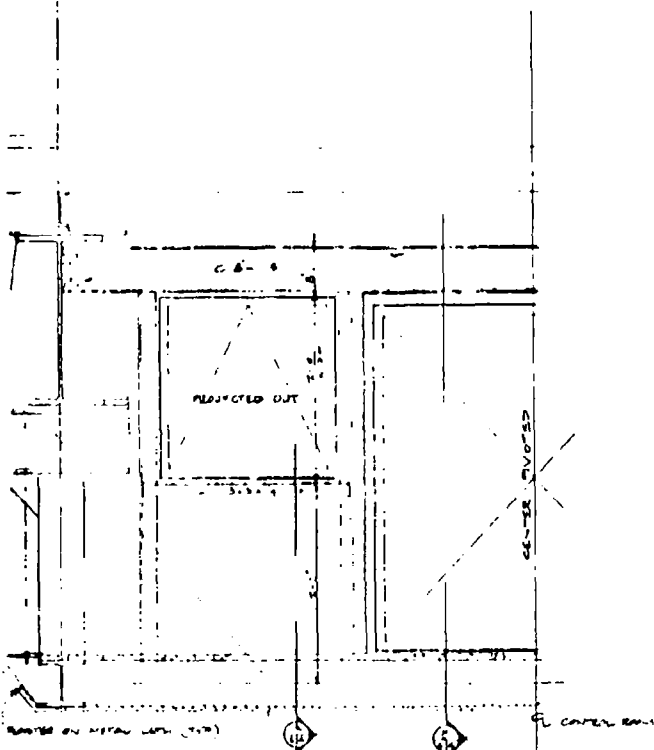
Section.

B



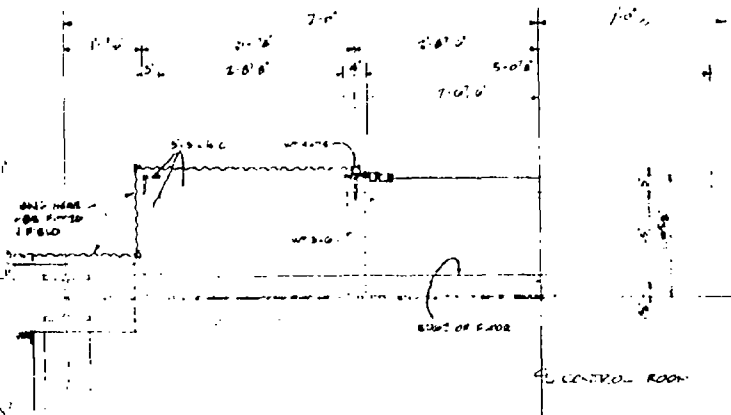
# NOTES:

1. ALL STRUCTURE TO BE 1/4" DIA. GALV. COIL-SPUN STEEL.
2. WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL. WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL. WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL.
3. WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL. WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL. WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL.
4. ALL WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL. WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL. WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL.
5. ALL WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL. WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL. WINDSHIELD MUST BE 1/4" DIA. GALV. COIL-SPUN STEEL.



INTERIOR ELEVATION

SCALE 1/4" = 1'-0"



PARTIAL PLAN

SCALE 1/4" = 1'-0"

DRAWN BY	DATE	1/1/68	1/1/68	1/1/68	1/1/68
CHECK BY	DATE				
APP. BY	DATE				
APP. BY	DATE				
APP. BY	DATE				
APP. BY	DATE				
SCALE	1/4" = 1'-0"				
BOEING CORPORATION		PLANT ENGINEERING		DWG. NO. 1/1/68-1/1/68	

tails.

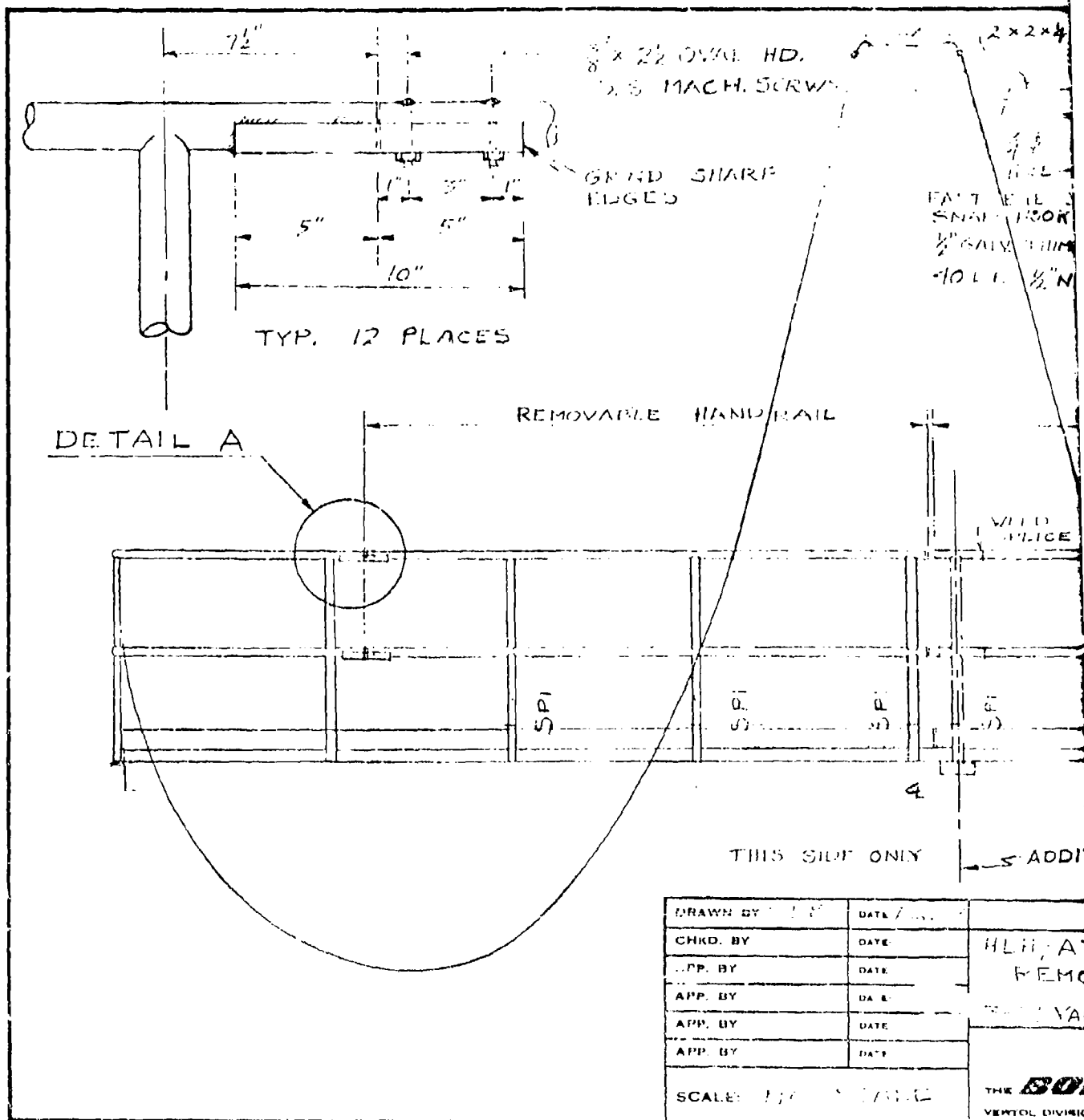


Figure 57. Removable Handrail Details.

DRAWN BY	DATE	TITLE			
CHKD. BY	DATE	HELICOPTER CARGO HANDLING SYS			
APP. BY	DATE	REMOVABLE HANDRAIL			
APP. BY	DATE	DETAILS			
APP. BY	DATE	CTB 7			
APP. BY	DATE				
APP. BY	DATE				
SCALE: 1/4" = 1'-0"		PLANT ENGINEERING	REV	DATE	DESCRIPTION
		THE <b>BOEING</b> COMPANY	DWG. NO. 73-07250		
		VERTOL DIVISION			





50" S FLANGE  
THREADED

SIGNAL  
CONDUCTOR  
REEL

FLEX MET HOSE  
EVERPLEK  
S.S. HOSE ASSEMBLY  
5" H.W. 24"  
1.5 DRIP/ 3 FITTINGS

TOWER

EXHAUST  
110" S.S.

TWO HOIST



NOTE 1

1. ALL PIPING TO BE SCH 40 S.S.
2. ALL INSULATION TO BE 2" MIN (CARB) CARB TEMP 1500 WITH 616 WEATHERPROOF ALUM JACKET.
3. ALL INSULATION BUT JOINTS TO BE SEALED WITH ALUMINUM BANDS.
4. ALL METAL HOSE TO BE RATED AT 60 PSI, 343°F.
5. INSTALL SPRING HANGERS IN THEIR FULL PRESTRESSED POSITION.
6. ALL FITTINGS TO BE "SPEEDLINE" BELLED END TYPE 304 SS SCH 5.
7. ALL STAINLESS STEEL PIPE & FITTINGS TO BE HELIARC WELDED, SEE WELDING SPECIFICATIONS.

# PART PLAN TOWER

CT  
A STD CURF (2)  
P A 2" O.D.  
INC. HASTINGS MICH

AB  
175A  
HALL

HA

P.W. HOIST

EXHAUST  
110" S.S.

2" DIA. WASTEWATER  
4500-2808

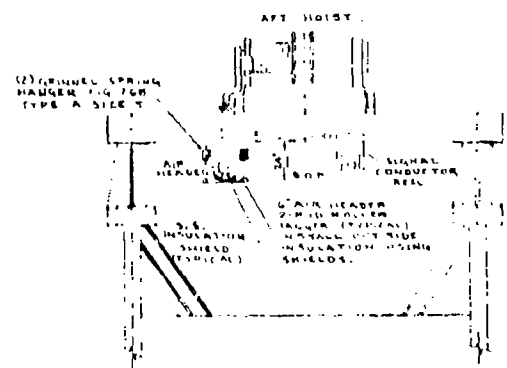
3" TEE

SINGLE ROD HOIST  
HANGER W/ 6" WHEEL  
SPRING HANGER  
FIG 1-60 TYPE A  
WELDED

SCH 40 S.S. 1"  
ON 6" S.S. TWR 10"  
IN 6" API

WELD  
ALL AROUND

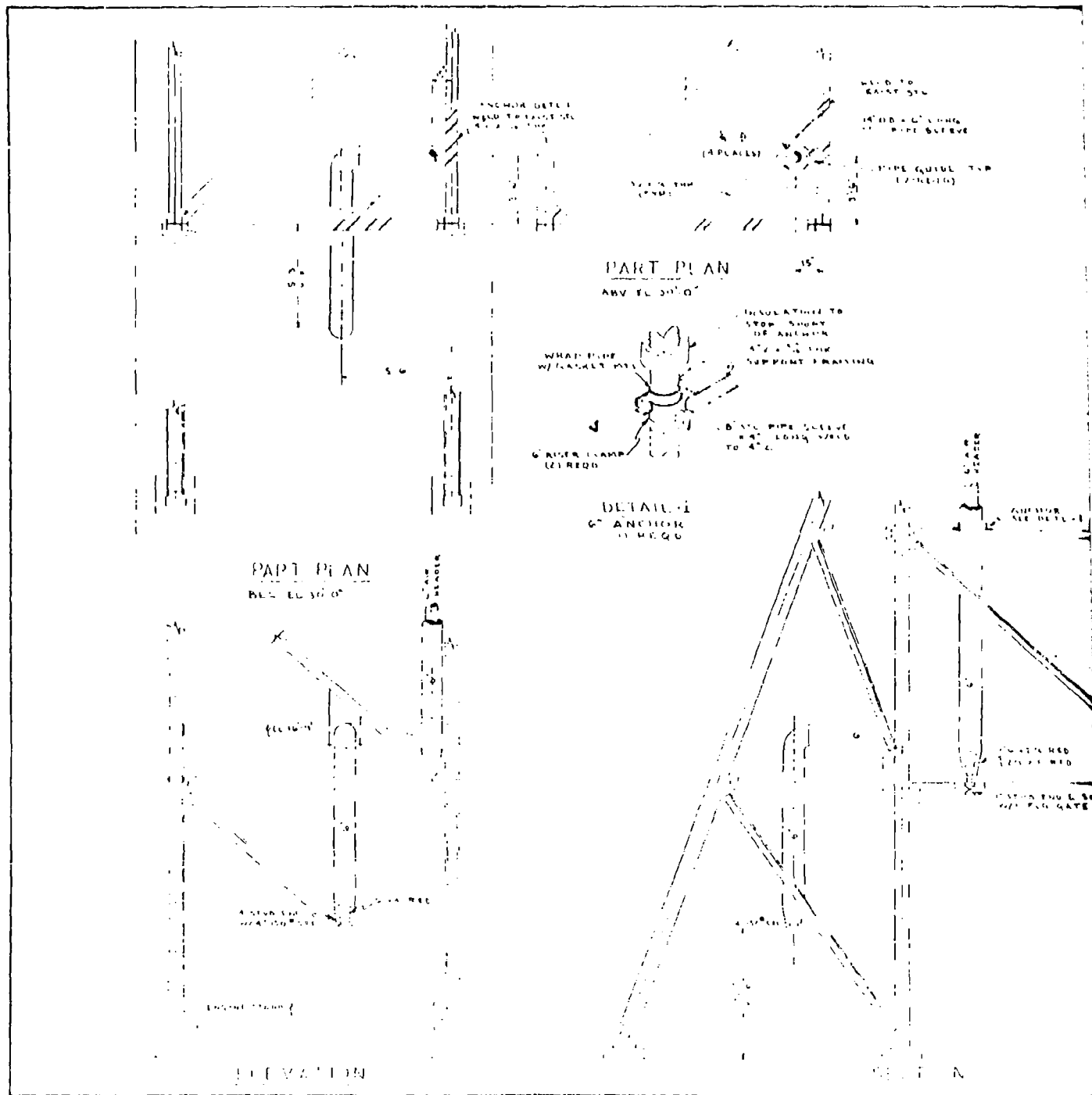
ELEVATION



## SECTION B-B

FR 213660		SHEET 1 OF 5	
DESIGNED BY	DATE	TITLE	REV
DRAWN BY	DATE	HEATING (ARGON) HES SYS	
APP BY	DATE	1ST TOWER TOP SECTION	
APP BY	DATE	PIPING ARRGT	
APP BY	DATE	PLANT ENGINEERING	
SCALE 3/4" = 1'-0"		BOEING	
		DATE 11/15/55	

p Section.



A

Figure 59. Piping Arrangement, Test Tower Base.

# NOTES

1. ALL WELDING TO BE DONE BY AWS C-100
2. ALL WELDING TO BE DONE BY AWS C-100
3. ALL WELDING TO BE DONE BY AWS C-100
4. ALL GASKETS TO BE OF COMPRESSION RESISTANT TYPE TO GASKET REQUIREMENTS
5. ALL STAINLESS STEEL PIPE FITTINGS TO BE HELIUM WELDED SEE WELDING SPECIFICATIONS
6. ALL INSULATION PLY JOINTS TO BE SEALED WITH ALUMINUM BANDS



WELD TO  
EXISTING STEEL  
14" O.D. LONG  
STEEL PIPE SLEEVE  
PIPE GUIDE TOP  
(12" HIGHER)

## PLAN

INSULATION TO  
TOP SURFACE  
OF ANCHOR  
4" x 4" x 1/2" THK  
SUPPORT FRAMING

8" O.D. PIPE SLEEVE  
4' x 4" LONG WELD  
TO 4" x 4"

ANCHOR  
SEE DETAIL 1

WELDED  
TO ANCHOR  
PIPE GUIDE TOP  
PIPE GUIDE WELD

## SECTION

DATE	BY	CHKD	APP'D	REV	DESCRIPTION
10/1/50	J. H. H.	J. H. H.	J. H. H.	1	ISSUED FOR CONSTRUCTION
10/1/50	J. H. H.	J. H. H.	J. H. H.	2	REVISION
10/1/50	J. H. H.	J. H. H.	J. H. H.	3	REVISION
10/1/50	J. H. H.	J. H. H.	J. H. H.	4	REVISION
10/1/50	J. H. H.	J. H. H.	J. H. H.	5	REVISION
10/1/50	J. H. H.	J. H. H.	J. H. H.	6	REVISION
10/1/50	J. H. H.	J. H. H.	J. H. H.	7	REVISION
10/1/50	J. H. H.	J. H. H.	J. H. H.	8	REVISION
10/1/50	J. H. H.	J. H. H.	J. H. H.	9	REVISION
10/1/50	J. H. H.	J. H. H.	J. H. H.	10	REVISION

BOEING PLANT ENGINEERING

B



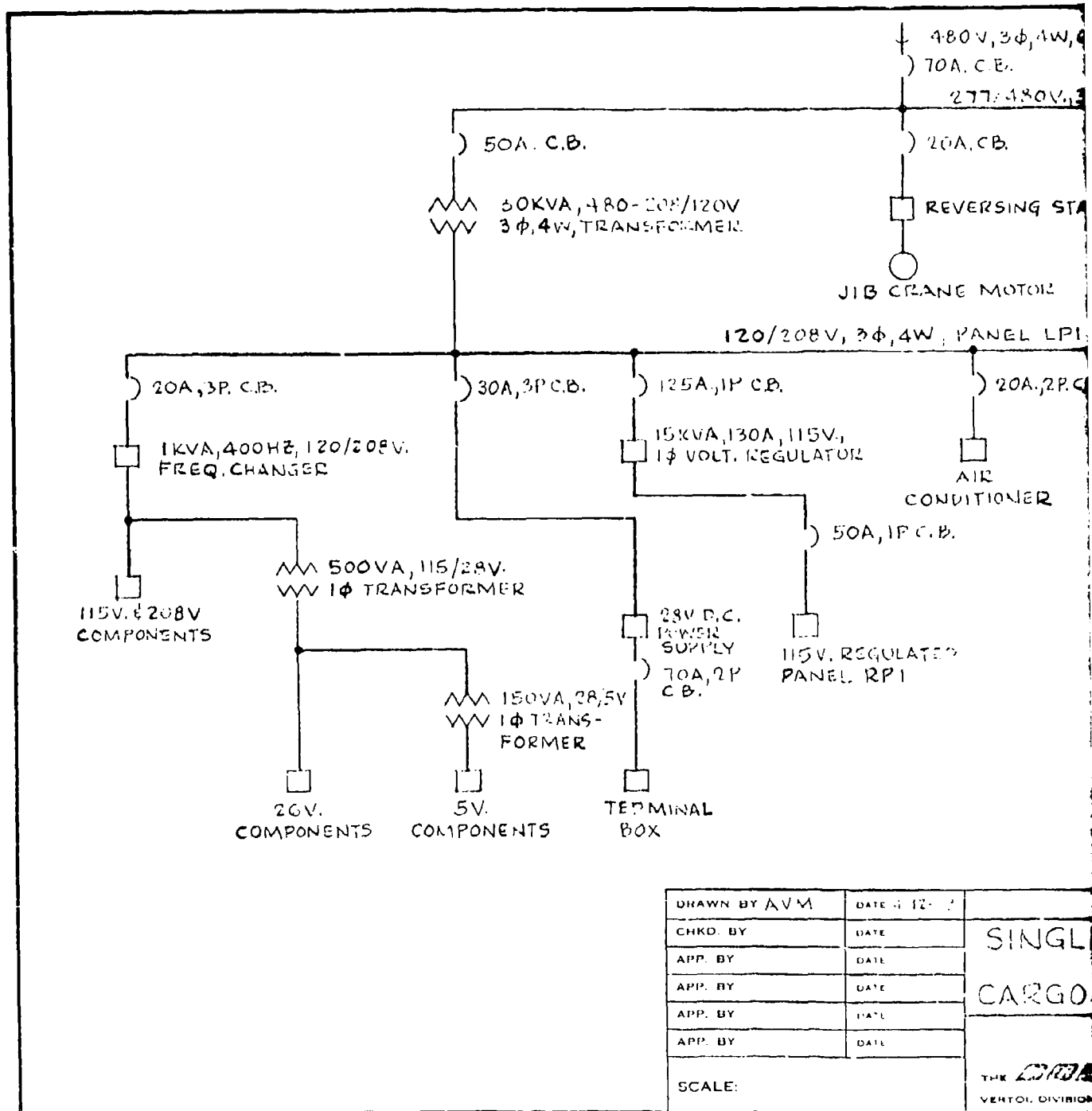


(NO SCALE)

PPG UNIT & FUEL PUMP BY OTHERS.

DRAWN BY		DATE		FIG. 22 AGG-0		SHEET 3 OF 3	
CHECKED BY		DATE		TITLE			
APP. BY		DATE		MATERIAL: CARBON STEEL, SAE 1020			
APP. BY		DATE		TOLERANCES: UNLESS OTHERWISE SPECIFIED			
APP. BY		DATE		FINISH: MILLING, 10-60			
APP. BY		DATE		PLANT ENGINEERING		REV. 1-1-68	
APP. BY		DATE		BOEING COMPANY		CWD NO. 2-1003-10-02	
SCALE: 1/2" = 1'-0"							

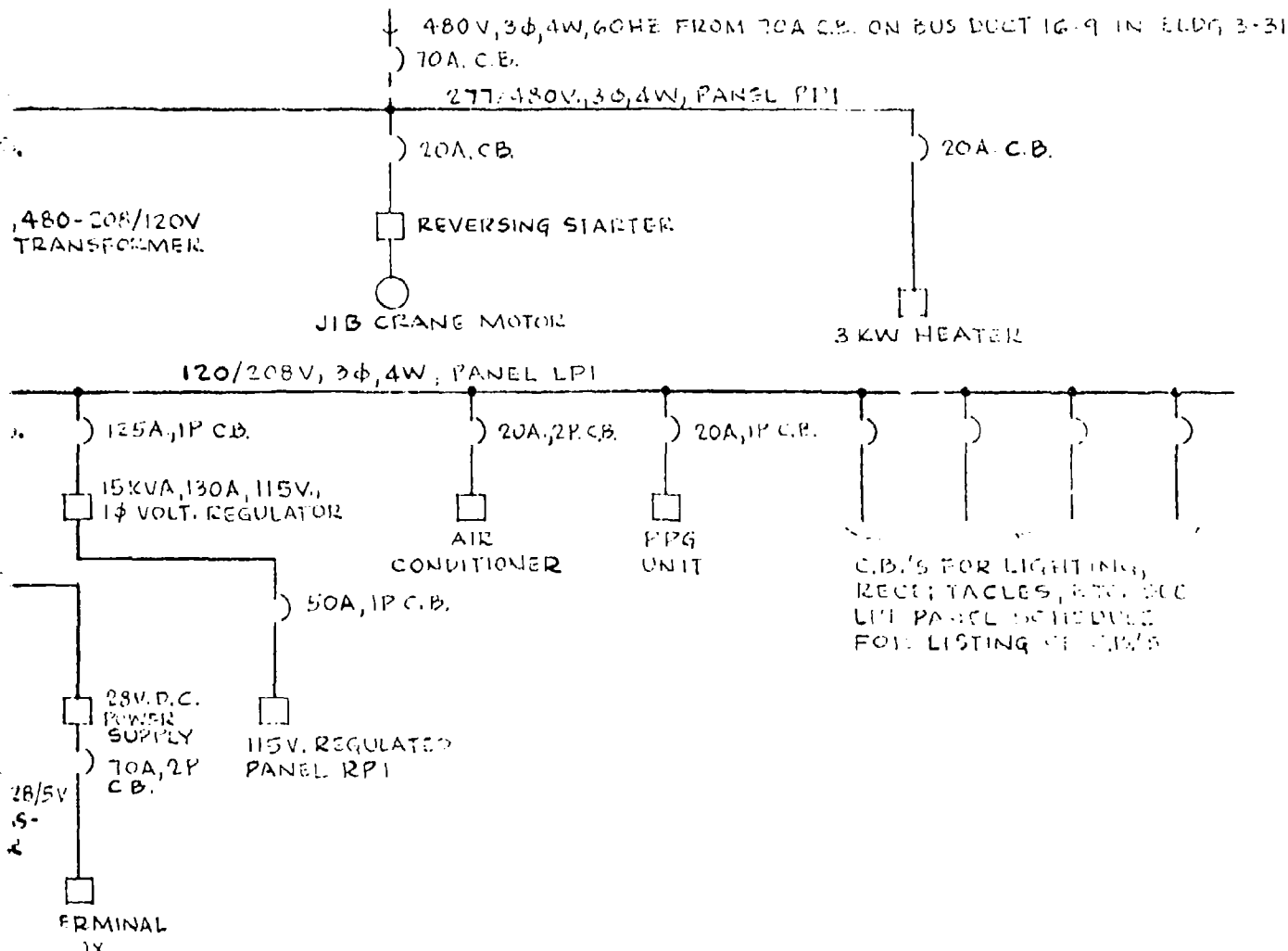
.t.



A

Figure 61. Electrical Single Line Diagram.

Preceding page blank



DRAWN BY AVM	DATE: 12-1-77	TITLE	
CHKD. BY	DATE	SINGLE LINE DIAGRAM	
APP. BY	DATE	HLH/ATC	
APP. BY	DATE	CARGO HANDLING TEST RIG	
APP. BY	DATE	PLANE ENGINEERING	
APP. BY	DATE	THE DRESSING ROOM	
SCALE:		VENTON DIVISION	



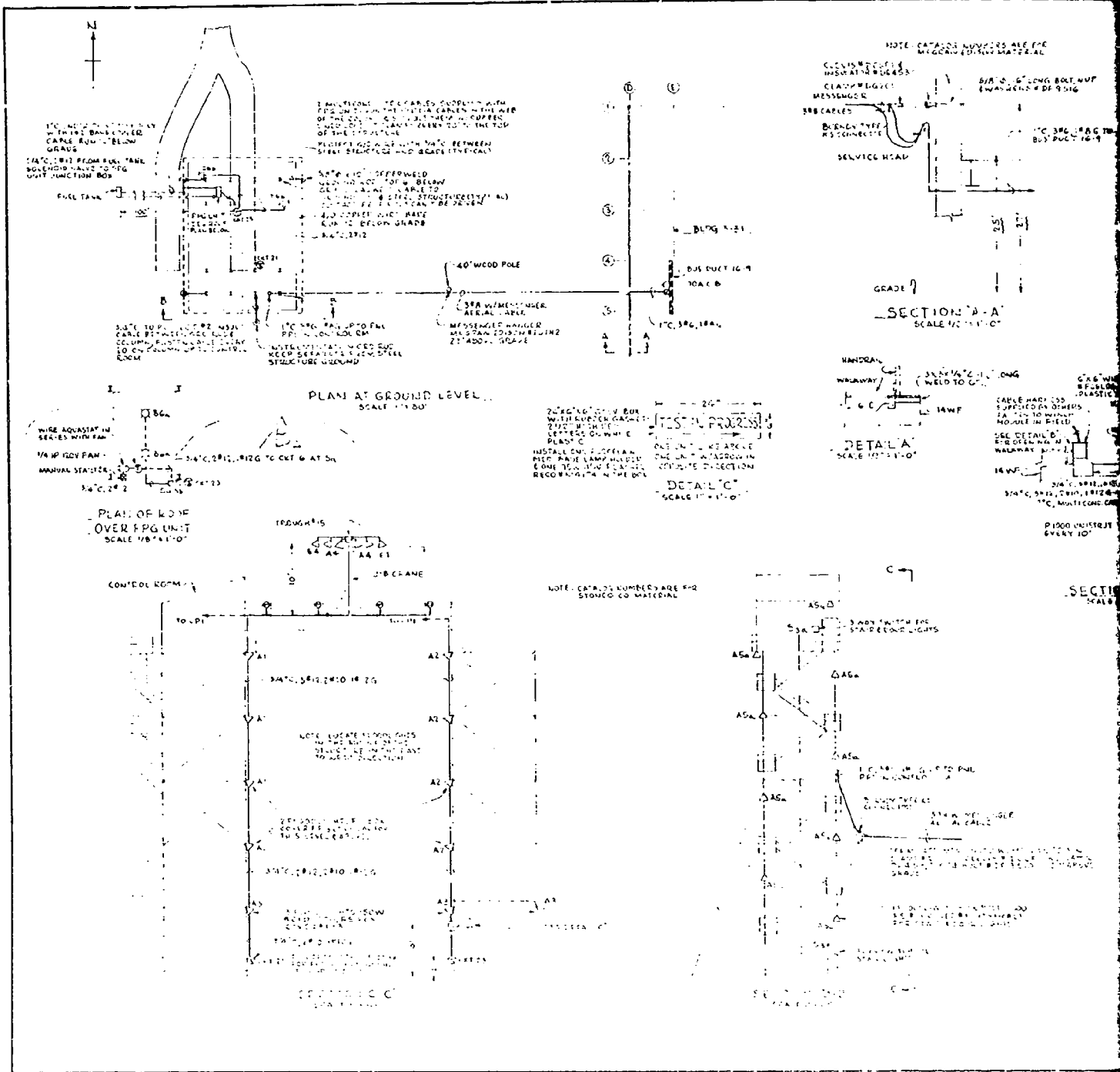
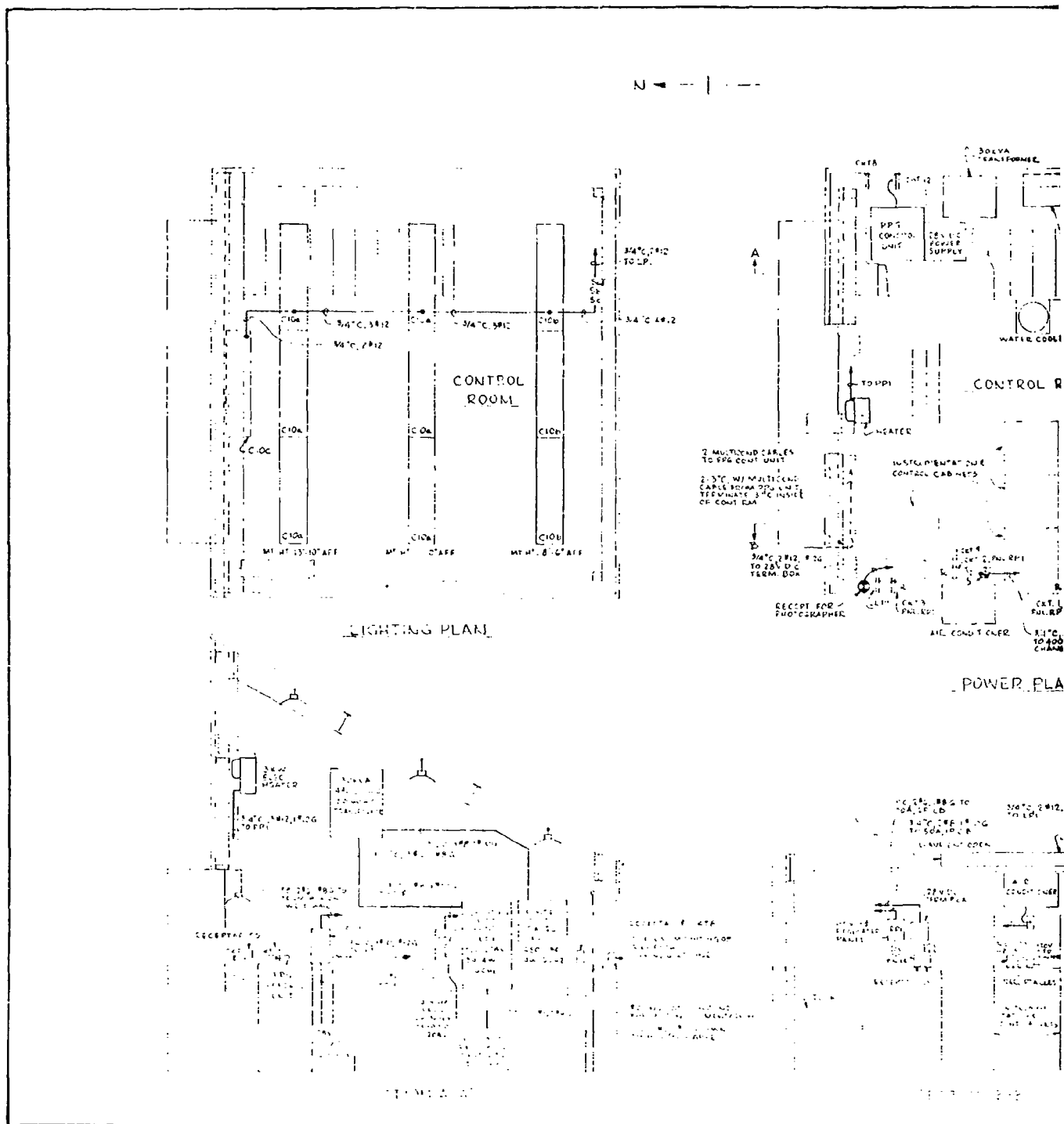


Figure 62. Electrical HLH/ATC Cargo Handling Test Rig.

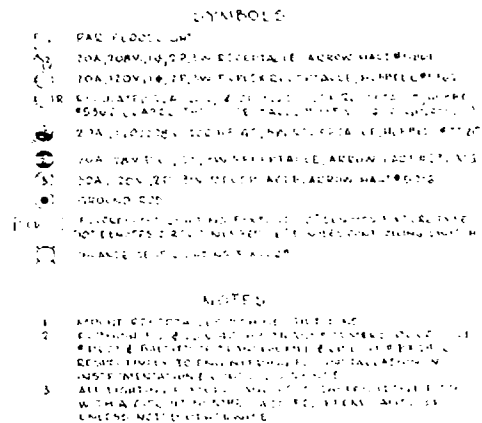




A

Figure 63. Control Room Electrical Layout.

Preceding page blank



DESIGN BY	DATE	TITLE	
DESIGNED BY			
APP. BY			
APP. BY			
APP. BY			
APP. BY			
APP. BY			
SCALE		PLANT ENGINEERING	
		<b>BOEING</b> COMPANY	
		10000 NORTH AVENUE, SEATTLE, WASH.	

11-12-71

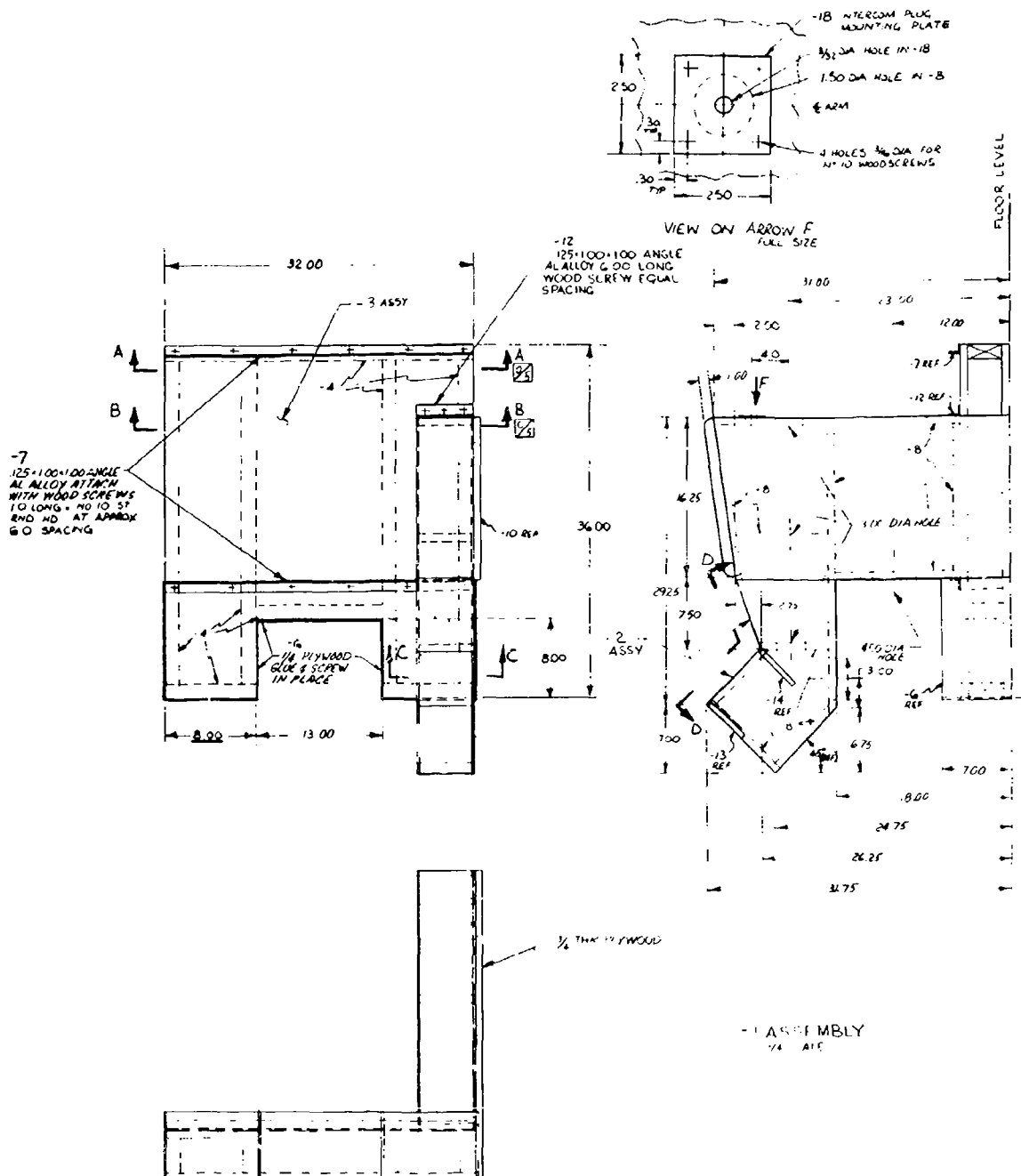
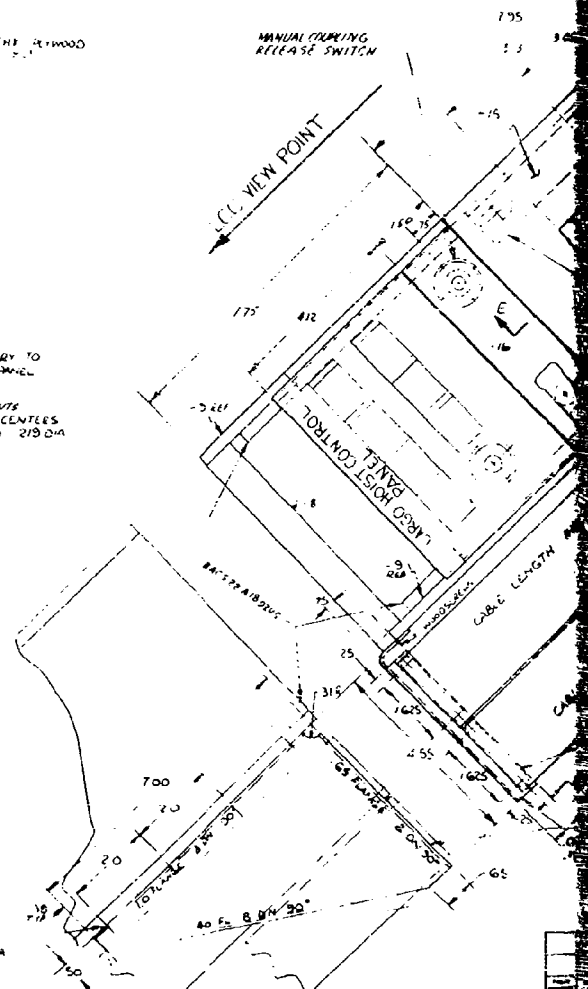
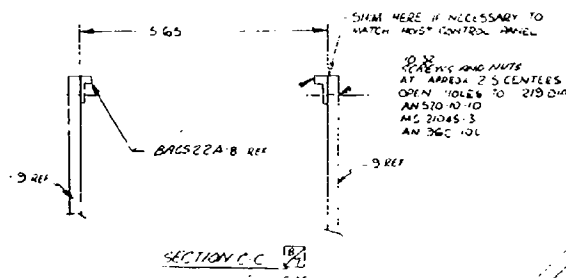
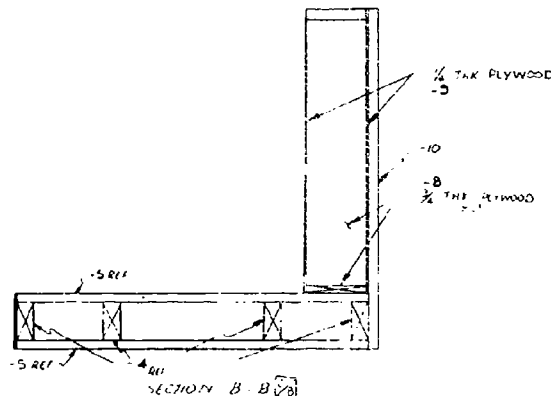
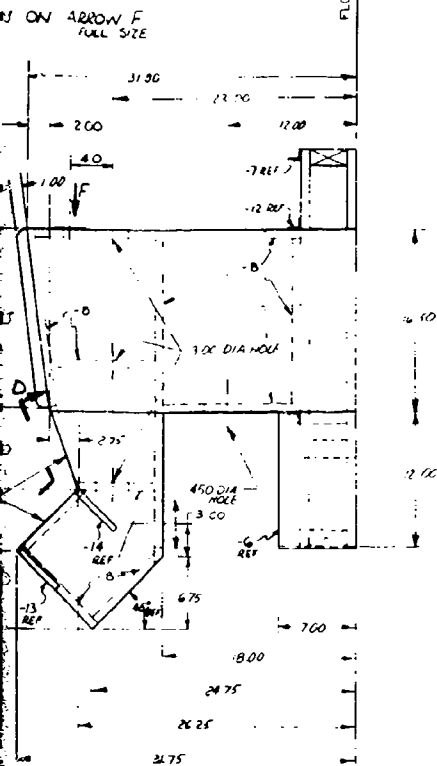
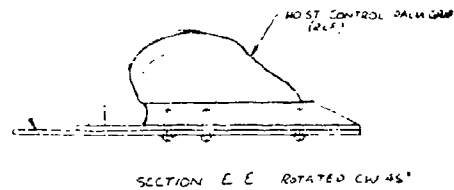
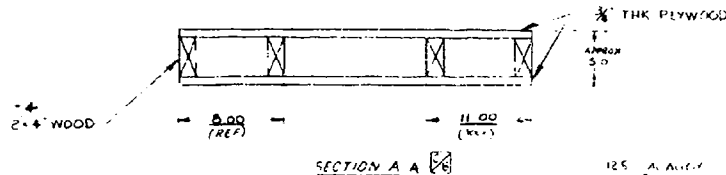
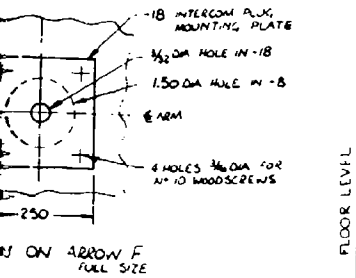


Figure 64. Load Controlling Crewman Platform - Integrated Test Rig.

Preceding page blank

A



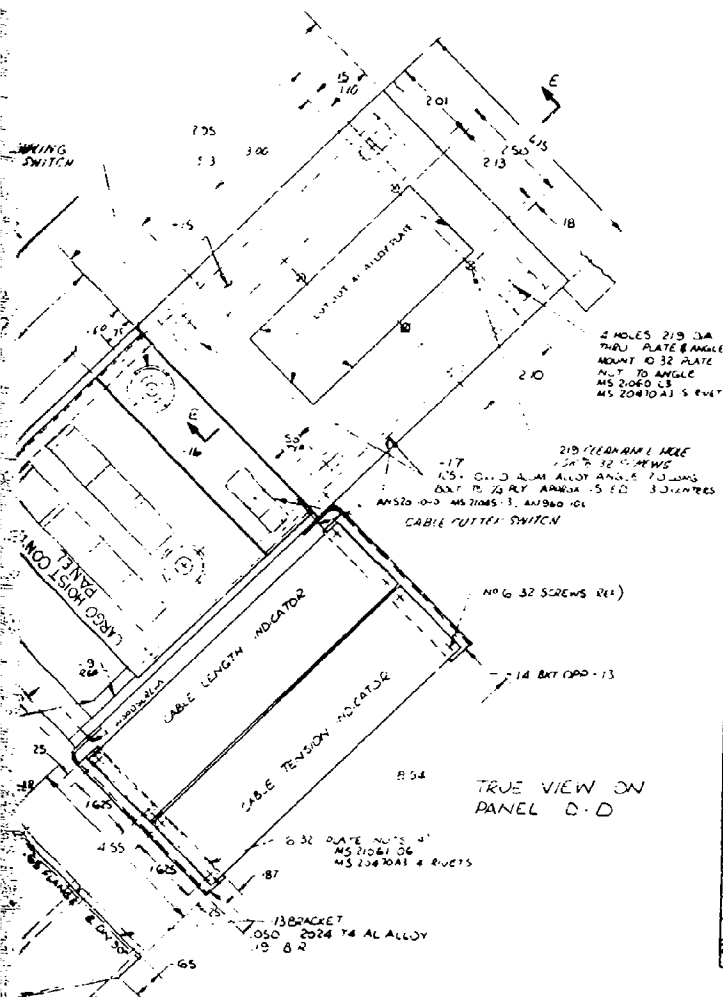
-1 ASSEMBLY  
 1/4 SCALE

SK 301-11564

4 HOLES 210 DIA

3

3 E-E ROTATED CW 45°



- 1 MATERIALS & HARDWARE MAY BE SUBSTITUTED  
TO SUIT AVAILABILITY
- 2 GIVE # SCREW - 2 ASSY AS NECESSARY
- 3 GIVE # SCREW - 3 ASSY AS NECESSARY
- 4 - 1 TO BE ASSEMBLED IN ITR CONTROL ROOMS
- 5 INDICATORS, SWITCHES, CONTROLLER & PANEL  
SHOWN FOR REF ONLY
- 6 PAINT 'LOCKBIT GREY'

[illegible]

Page	PLACEMENT	DATE	BY	INITIALS	TIME

[illegible]

5X 301-11564

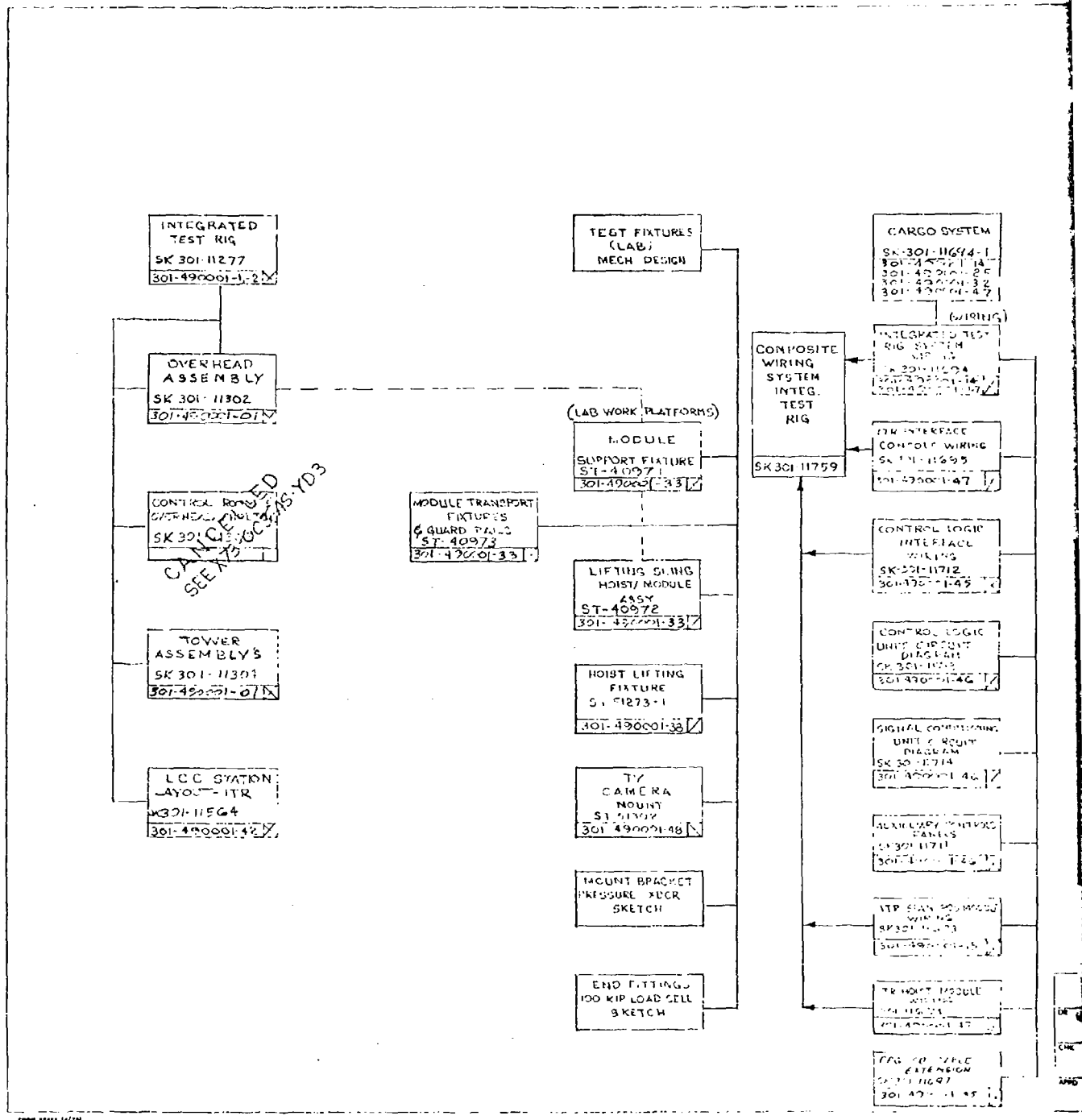


Figure 65. System Test - Drawing Tree.

Preceding page blank







124"

62'

1 1/2"

3/16"

20"

40"

1 1/2"

TIP GUSSET BRACES

-1 SLING ASSY

Hoist Module [REF]

60"

1 1/2"

3/16"

20"

40"

1 1/2"

5813-7/8 SHACKLE

8x115" FT CHANNEL

CROSBY-LAUGHLIN 5813-1/2 SHACKLE

AN17-25A BOLT

AN17-25A WASHER

MS21049N16 NUT

NOTE: PAINT TWO COATS STEEL PRIMER TWO COATS YELLOW EXTERIOR ENAMEL

REV. 1

REV. 2

REV. 3

REV. 4

REV. 5

REV. 6

REV. 7

REV. 8

REV. 9

REV. 10

REV. 11

REV. 12

REV. 13

REV. 14

REV. 15

REV. 16

REV. 17

REV. 18

REV. 19

REV. 20

REV. 21

REV. 22

REV. 23

REV. 24

REV. 25

REV. 26

REV. 27

REV. 28

REV. 29

REV. 30

REV. 31

REV. 32

REV. 33

REV. 34

REV. 35

REV. 36

REV. 37

REV. 38

REV. 39

REV. 40

REV. 41

REV. 42

REV. 43

REV. 44

REV. 45

REV. 46

REV. 47

REV. 48

REV. 49

REV. 50

REV. 51

REV. 52

REV. 53

REV. 54

REV. 55

REV. 56

REV. 57

REV. 58

REV. 59

REV. 60

REV. 61

REV. 62

REV. 63

REV. 64

REV. 65

REV. 66

REV. 67

REV. 68

REV. 69

REV. 70

REV. 71

REV. 72

REV. 73

REV. 74

REV. 75

REV. 76

REV. 77

REV. 78

REV. 79

REV. 80

REV. 81

REV. 82

REV. 83

REV. 84

REV. 85

REV. 86

REV. 87

REV. 88

REV. 89

REV. 90

REV. 91

REV. 92

REV. 93

REV. 94

REV. 95

REV. 96

REV. 97

REV. 98

REV. 99

REV. 100

QTY	REV	CODE	DESCRIPTION	MATERIAL AND SPECIFICATION	REV	DATE	BY	CHKD	APPV
4			3/16 DIA 121.3 1/2 IN 1020 STEEL WIRE ROPE - 60' LENGTH	ACCU WIRE ROPE CO. INTERMOUNTAIN TRADING CO. 211 S. HENNINGSON AVE. WARRICK, IN 46786					
2			5813-1/2 SHACKLE	CROSBY-LAUGHLIN CHESTER HARDWARE					
1			5813-7/8 SHACKLE	CROSBY-LAUGHLIN					
1			MS21049N16 NUT						
8			MS21049N16 NUT						
8			AN17-25A BOLT						
1			AN17-25A BOLT						
2			-7 SPACER	3/16 x 1 1/2 x 4 - STEEL PLATE - COMM'L	1020				
1			-6 CENTER PLATE	1 x 4 x 12 - STEEL PLATE - COMM'L	1020				
2			-5 SPACER	3/16 x 1 1/2 x 4 - STEEL PLATE - COMM'L	1020				
2			-4 LIFT PLATE	3/4 x 3 x 1 1/2 - STEEL PLATE - COMM'L	1020				
4			-3 SPACER	1/8 x 3 x 8 - STEEL PLATE - COMM'L	1020				
2			-2 CHANNEL	8 x 115 1/2 FT 104' LENGTH S100 CHANNEL	A-36				
1			-1 SLING ASSY						

3/16 STOCK

1 1/2

30

1 1/2

5813-1/2 SHACKLE

-B ENDS

3/16 DIA 121.3 1/2 IN 1020 STEEL WIRE ROPE - 60' LENGTH

ACCU WIRE ROPE CO. INTERMOUNTAIN TRADING CO. 211 S. HENNINGSON AVE. WARRICK, IN 46786

5813-1/2 SHACKLE

CROSBY-LAUGHLIN CHESTER HARDWARE

5813-7/8 SHACKLE

CROSBY-LAUGHLIN

MS21049N16 NUT

MS21049N16 NUT

AN17-25A BOLT

AN17-25A BOLT

-7 SPACER

3/16 x 1 1/2 x 4 - STEEL PLATE - COMM'L

-6 CENTER PLATE

1 x 4 x 12 - STEEL PLATE - COMM'L

-5 SPACER

3/16 x 1 1/2 x 4 - STEEL PLATE - COMM'L

-4 LIFT PLATE

3/4 x 3 x 1 1/2 - STEEL PLATE - COMM'L

-3 SPACER

1/8 x 3 x 8 - STEEL PLATE - COMM'L

-2 CHANNEL

8 x 115 1/2 FT 104' LENGTH S100 CHANNEL

-1 SLING ASSY

REV. 1

REV. 2

REV. 3

REV. 4

REV. 5

REV. 6

REV. 7

REV. 8

REV. 9

REV. 10

REV. 11

REV. 12

REV. 13

REV. 14

REV. 15

REV. 16

REV. 17

REV. 18

REV. 19

REV. 20

REV. 21

REV. 22

REV. 23

REV. 24

REV. 25

REV. 26

REV. 27

REV. 28

REV. 29

REV. 30

REV. 31

REV. 32

REV. 33

REV. 34

REV. 35

REV. 36

REV. 37

REV. 38

REV. 39

REV. 40

REV. 41

REV. 42

REV. 43

REV. 44

REV. 45

REV. 46

REV. 47

REV. 48

REV. 49

REV. 50

REV. 51

REV. 52

REV. 53

REV. 54

REV. 55

REV. 56

REV. 57

REV. 58

REV. 59

REV. 60

REV. 61

REV. 62

REV. 63

REV. 64

REV. 65

REV. 66

REV. 67

REV. 68

REV. 69

REV. 70

REV. 71

REV. 72

REV. 73

REV. 74

REV. 75

REV. 76

REV. 77

REV. 78

REV. 79

REV. 80

REV. 81

REV. 82

REV. 83

REV. 84

REV. 85

REV. 86

REV. 87

REV. 88

REV. 89

REV. 90

REV. 91

REV. 92

REV. 93

REV. 94

REV. 95

REV. 96

REV. 97

REV. 98

REV. 99

REV. 100

3/16 DIA 121.3 1/2 IN 1020 STEEL WIRE ROPE - 60' LENGTH

ACCU WIRE ROPE CO. INTERMOUNTAIN TRADING CO. 211 S. HENNINGSON AVE. WARRICK, IN 46786

5813-1/2 SHACKLE

CROSBY-LAUGHLIN CHESTER HARDWARE

5813-7/8 SHACKLE

CROSBY-LAUGHLIN

MS21049N16 NUT

MS21049N16 NUT

AN17-25A BOLT

AN17-25A BOLT

-7 SPACER

3/16 x 1 1/2 x 4 - STEEL PLATE - COMM'L

-6 CENTER PLATE

1 x 4 x 12 - STEEL PLATE - COMM'L

-5 SPACER

3/16 x 1 1/2 x 4 - STEEL PLATE - COMM'L

-4 LIFT PLATE

3/4 x 3 x 1 1/2 - STEEL PLATE - COMM'L

-3 SPACER

1/8 x 3 x 8 - STEEL PLATE - COMM'L

-2 CHANNEL

8 x 115 1/2 FT 104' LENGTH S100 CHANNEL

-1 SLING ASSY

REV. 1

REV. 2

REV. 3

REV. 4

REV. 5

REV. 6

REV. 7

REV. 8

REV. 9

REV. 10

REV. 11

REV. 12

REV. 13

REV. 14

REV. 15

REV. 16

REV. 17

REV. 18

REV. 19

REV. 20

REV. 21

REV. 22

REV. 23

REV. 24

REV. 25

REV. 26

REV. 27

REV. 28

REV. 29

REV. 30

REV. 31

REV. 32

REV. 33

REV. 34

REV. 35

REV. 36

REV. 37

REV. 38

REV. 39

REV. 40

REV. 41

REV. 42

REV. 43

REV. 44

REV. 45

REV. 46

REV. 47

REV. 48

REV. 49

REV. 50

REV. 51

REV. 52

REV. 53

REV. 54

REV. 55

REV. 56

REV. 57

REV. 58

REV. 59

REV. 60

REV. 61

REV. 62

REV. 63

REV. 64

REV. 65

REV. 66

REV. 67

REV. 68

REV. 69

REV. 70

REV. 71

REV. 72

REV. 73

REV. 74

REV. 75

REV. 76

REV. 77

REV. 78

REV. 79

REV. 80

REV. 81

REV. 82

REV. 83

REV. 84

REV. 85

REV. 86

REV. 87

REV. 88

REV. 89

REV. 90

REV. 91

REV. 92

REV. 93

REV. 94

REV. 95

REV. 96

REV. 97

REV. 98

REV. 99

REV. 100

REV. 1

REV. 2

REV. 3

REV. 4

REV. 5

REV. 6

REV. 7

REV. 8

REV. 9

REV. 10

REV. 11

REV. 12

REV. 13

REV. 14

REV. 15

REV. 16

REV. 17

REV. 18

REV. 19

REV. 20

REV. 21

REV. 22

REV. 23

REV. 24

REV. 25

REV. 26

REV. 27

REV. 28

REV. 29

REV. 30

REV. 31

REV. 32

REV. 33

REV. 34

REV. 35

REV. 36

REV. 37

REV. 38

REV. 39

REV. 40

REV. 41

REV. 42

REV. 43

REV. 44

REV. 45

REV. 46

REV. 47

REV. 48

REV. 49

REV. 50

REV. 51

REV. 52

REV. 53

REV. 54

REV. 55

REV. 56

REV. 57

REV. 58

REV. 59

REV. 60

REV. 61

REV. 62

REV. 63

REV. 64

REV. 65

REV. 66

REV. 67

REV. 68

REV. 69

REV. 70

REV. 71

REV. 72

REV. 73

REV. 74

REV. 75

REV. 76

REV. 77

REV. 78

REV. 79

REV. 80

REV. 81

REV. 82

REV. 83

REV. 84

REV. 85

REV. 86

REV. 87

REV. 88

REV. 89

REV. 90

REV. 91

REV. 92

REV. 93

REV. 94

REV. 95

REV. 96

REV. 97

REV. 98

REV. 99

REV. 100

Hoist/Module .

B

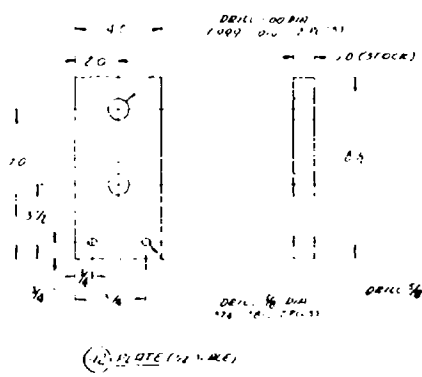
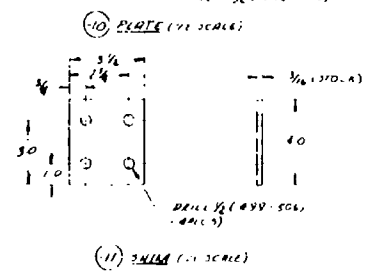
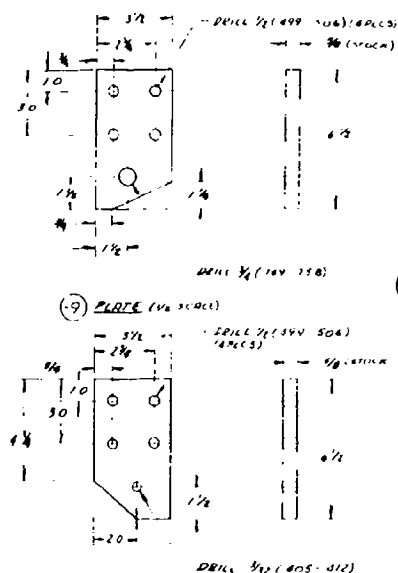
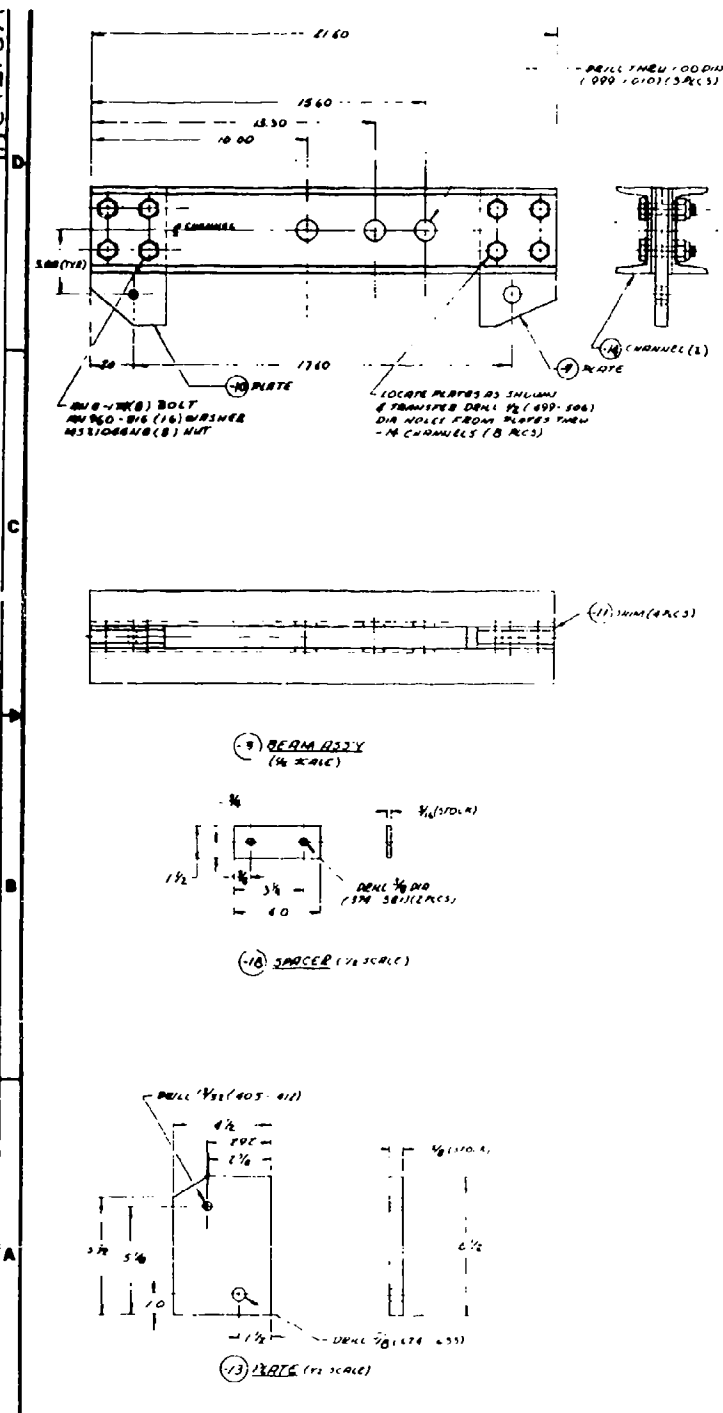
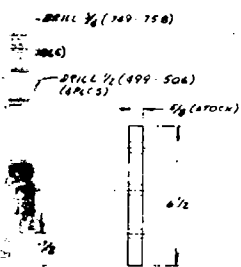
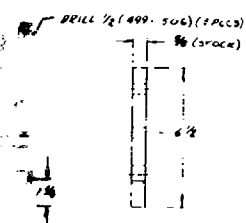
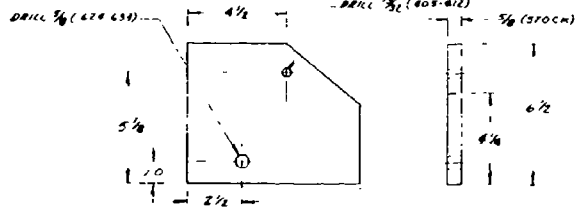
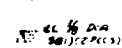
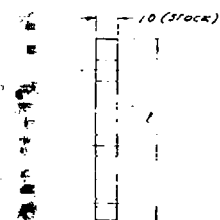
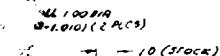
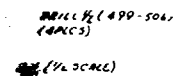
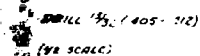


Figure 67. Hoist Lifting Fixture (Sheet 1).

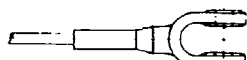
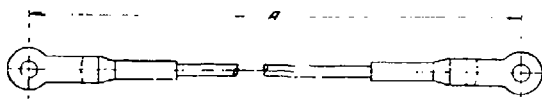
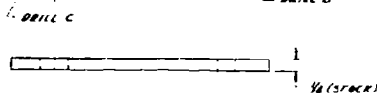


	-4	-5
A	22 3/4	10 3/8



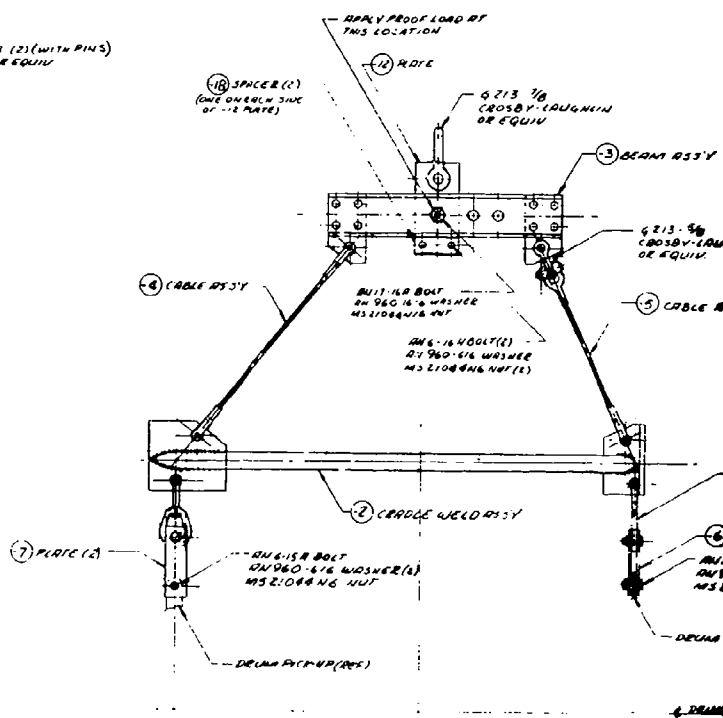
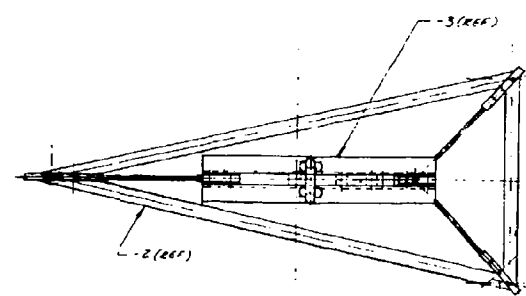
(4) & (7) PLATE (1/4 SCALE)

LENGTH A	DRILL B	DRILL C
-6	4.0	5/8
-7	4.6	5/8



ER-3222-H (2) (WITH PINS) TRU-LOC OR EQUIV

(4) & (5) CABLE ASSY (1/4 SCALE)



(1) SLING ASSY (1/4 SCALE)

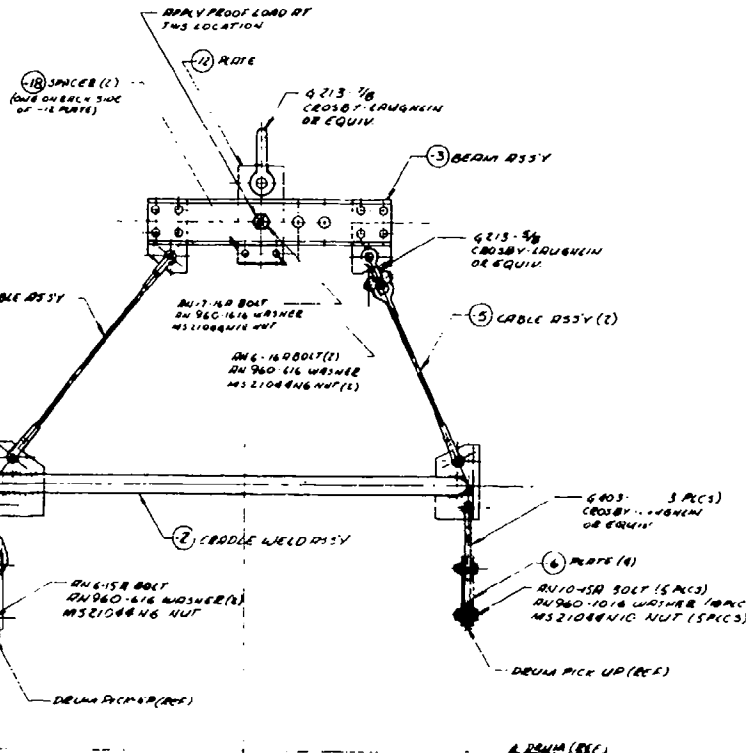
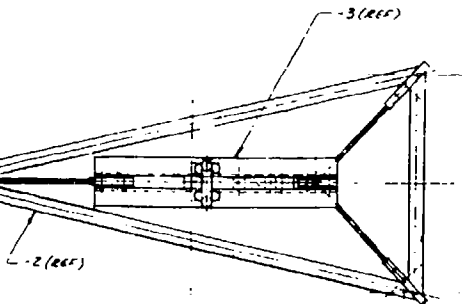
5751273

B

C

**Abstract**

- 2. PAINT - APPLY TWO COATS STEEL MEMBERS & TWO COATS OF YELLOW EPOXYE-OR ENAMEL.**



(-1) SLING ASSY  
MOIST ASSY, CARGO SYS  
(1/4 SCALE)

[illegible][illegible]



(3) BEAM 255 ✓

4213-5/8  
CROSS-EDU TAILIN  
OR EQUIV

-(5) CABLE ASSY (2)

- 6403-5/0 (3 PLS)  
CROSSY LAWRENCE  
OR EQUIV.

(6) 2076 (4)

AN10-45A BOLT (5 PCS)  
AN960-1016 WASHER (4 PCS)  
MS21086N10 NUT (5 PCS)

- DEWAS PICK UP (REK)

4344 (EE)

[illegible]

Page					

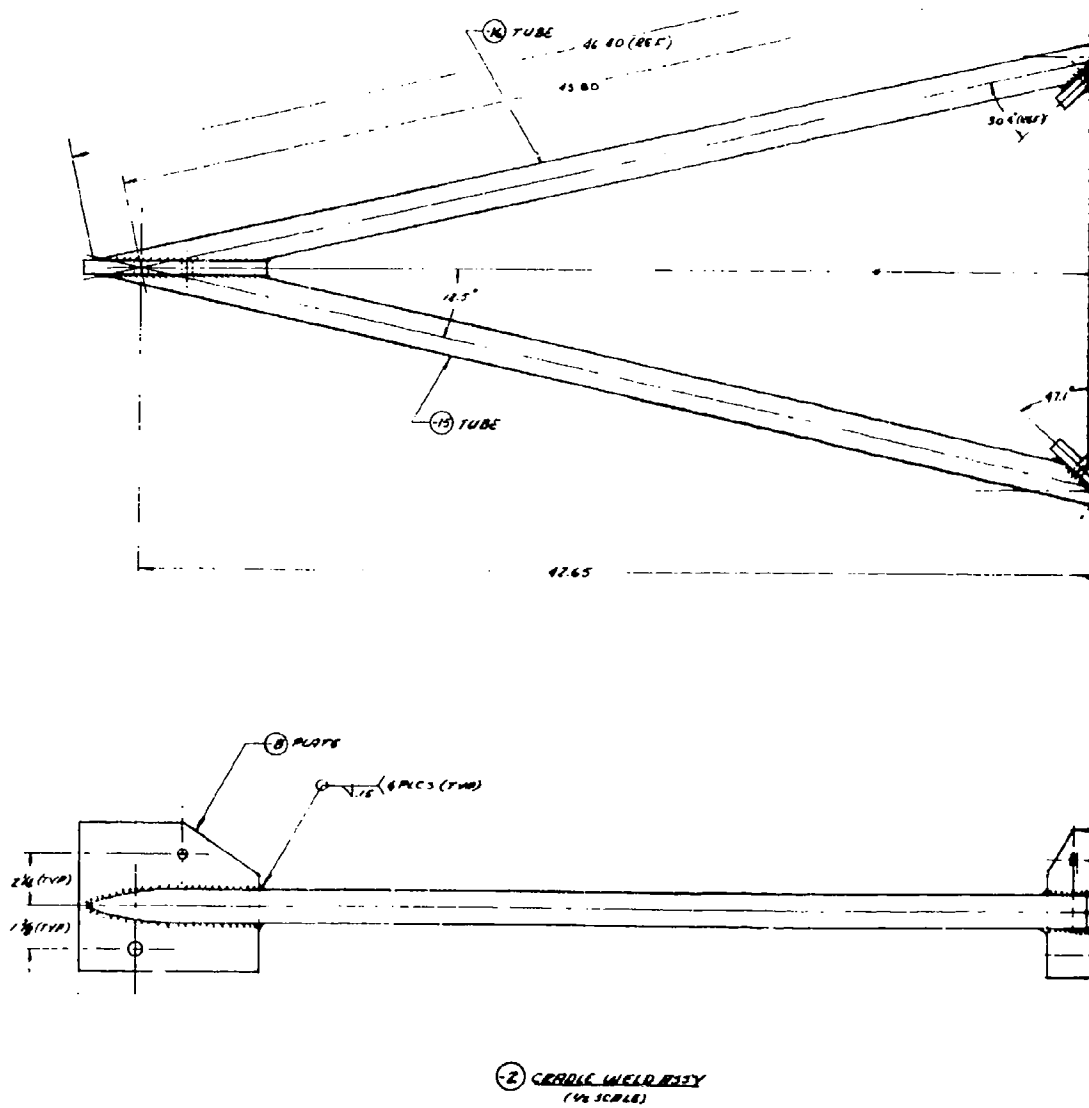

[illegible]

NAME	DATE
62-2	7/7
NAME	

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100
---	---	---	---	---	---	---	---	---	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	-----

SLING ASSY-  
HOIST ASSY, CARGO JYS.  
MLH  
J7772 5T51273

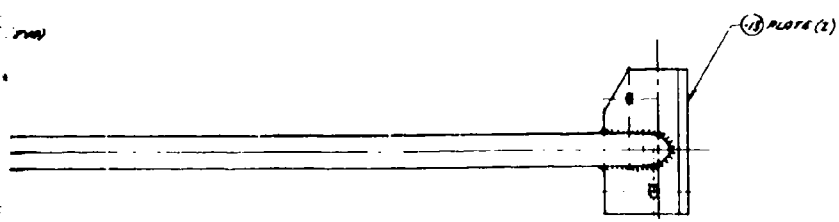
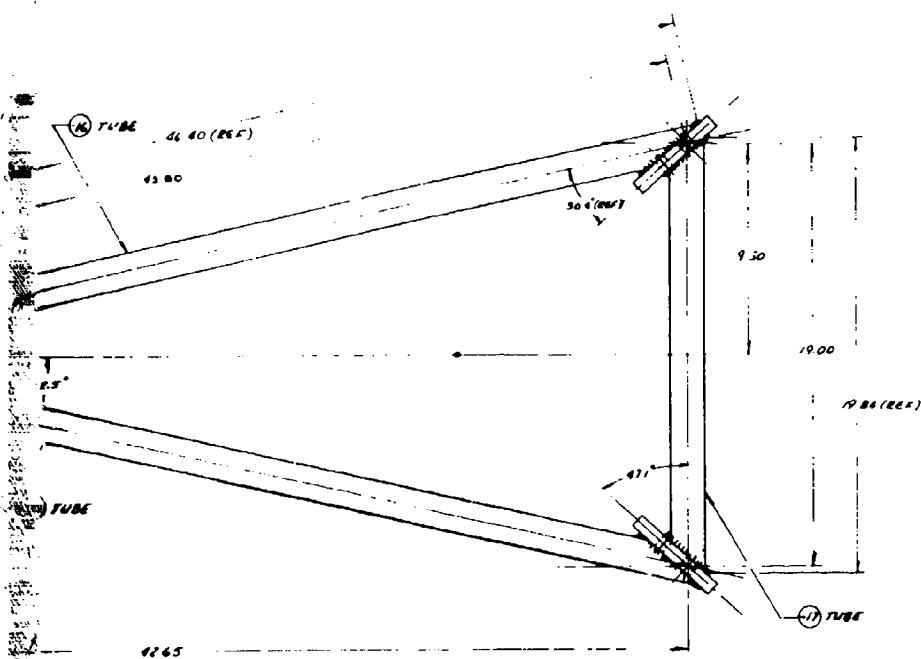
D  
C  
B  
A



A Figure 67. Hoist Lifting Fixture (Sheet 2).

Preceding page blank





(2) CIRCULAR WELD JOINT  
 (1/4" SCREW)

DATE		BY		CHECKED		APPROVED	
DATE	BY	DATE	BY	DATE	BY	DATE	BY
11/15/73	J. J. [signature]	11/15/73	J. J. [signature]	11/15/73	J. J. [signature]	11/15/73	J. J. [signature]
SLINK ASSY.				MOIST ASSY. CORE 40 SYS.			
N/A				N/A			
J. J. [signature]				J. J. [signature]			
J. J. [signature]				J. J. [signature]			

5751273

5751273

B

Sheet 2).

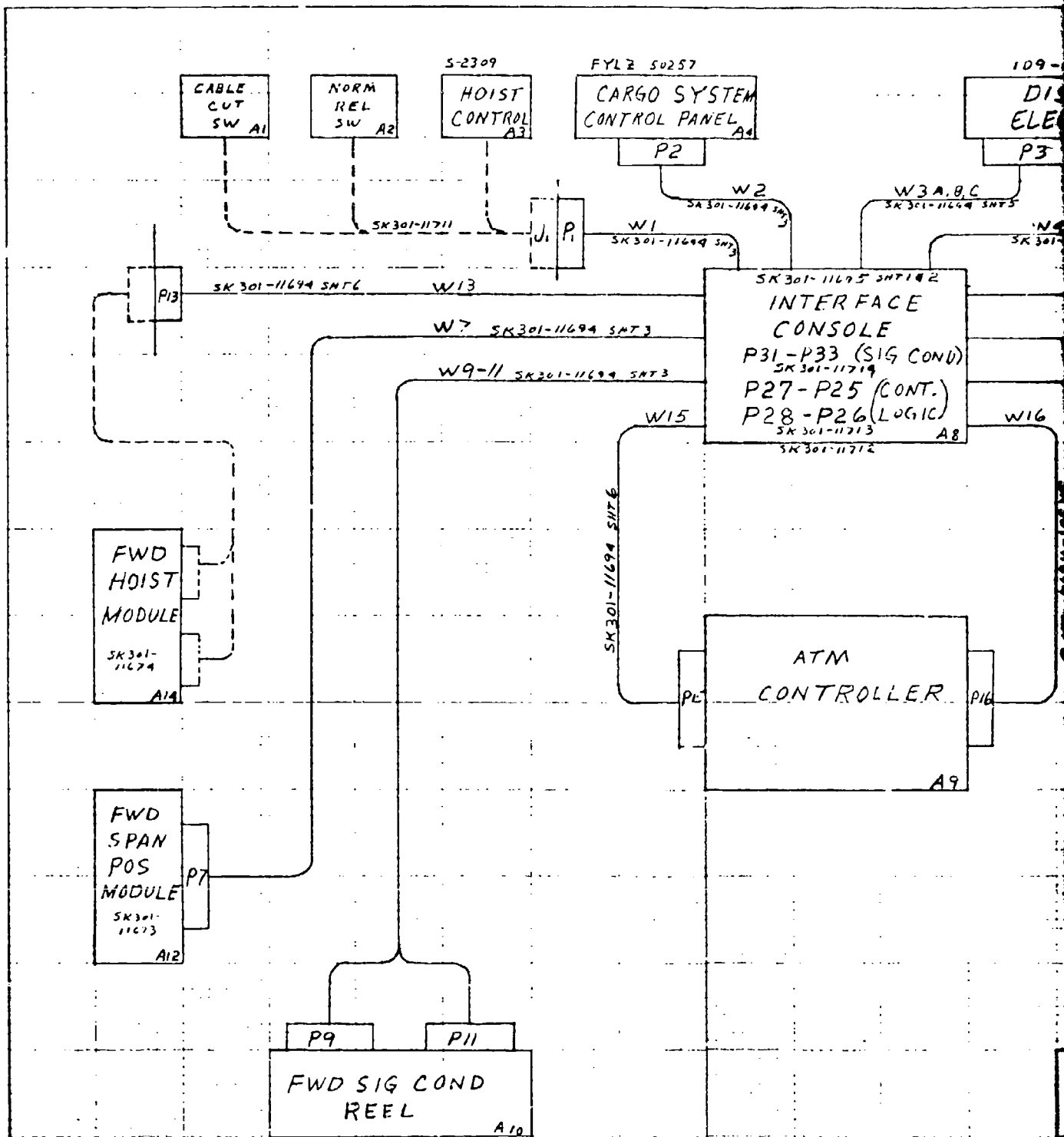


Figure 68. Integrated Test Rig - System Wiring.

STEM  
EL  
A6

109-172-P3

DISPLAY IEU  
ELECTRONICS  
P3 P4 A5

109-170  
CABLE  
TENSION  
IND A6  
P5

109-171  
CABLE  
LENGTH  
IND A7  
P6

REVISIONS			
LTR	DESCRIPTION	DATE	APPROVAL
A	COMPLETE REVISION	8/8/13	

W2  
01-11699 SMT3

W3A,B,C  
SK 301-11699 SMT5

W4  
SK 301-11699 SMT4

W5  
SK 301-11699 SMT7

SK 301-11695 SMT1 & 2  
INTERFACE  
CONSOLE  
P31-P33 (SIG COND)  
SK 301-11714  
P27-P25 (CONT.)  
P28-P26 (LOGIC)  
SK 301-11713  
SK 301-11712  
A8

W14

SK 301-11699 SMT6

W8

SK 301-11699 SMT3

W10-12

W16

SK 301-11699 SMT6

SK 301-11699 SMT3

AFT  
HOIST  
MODULE  
SK 301-11699  
A15

AFT  
SPAN  
POS  
MODULE  
SK 301-11693  
A13

ATM  
CONTROLLER  
P16  
A9

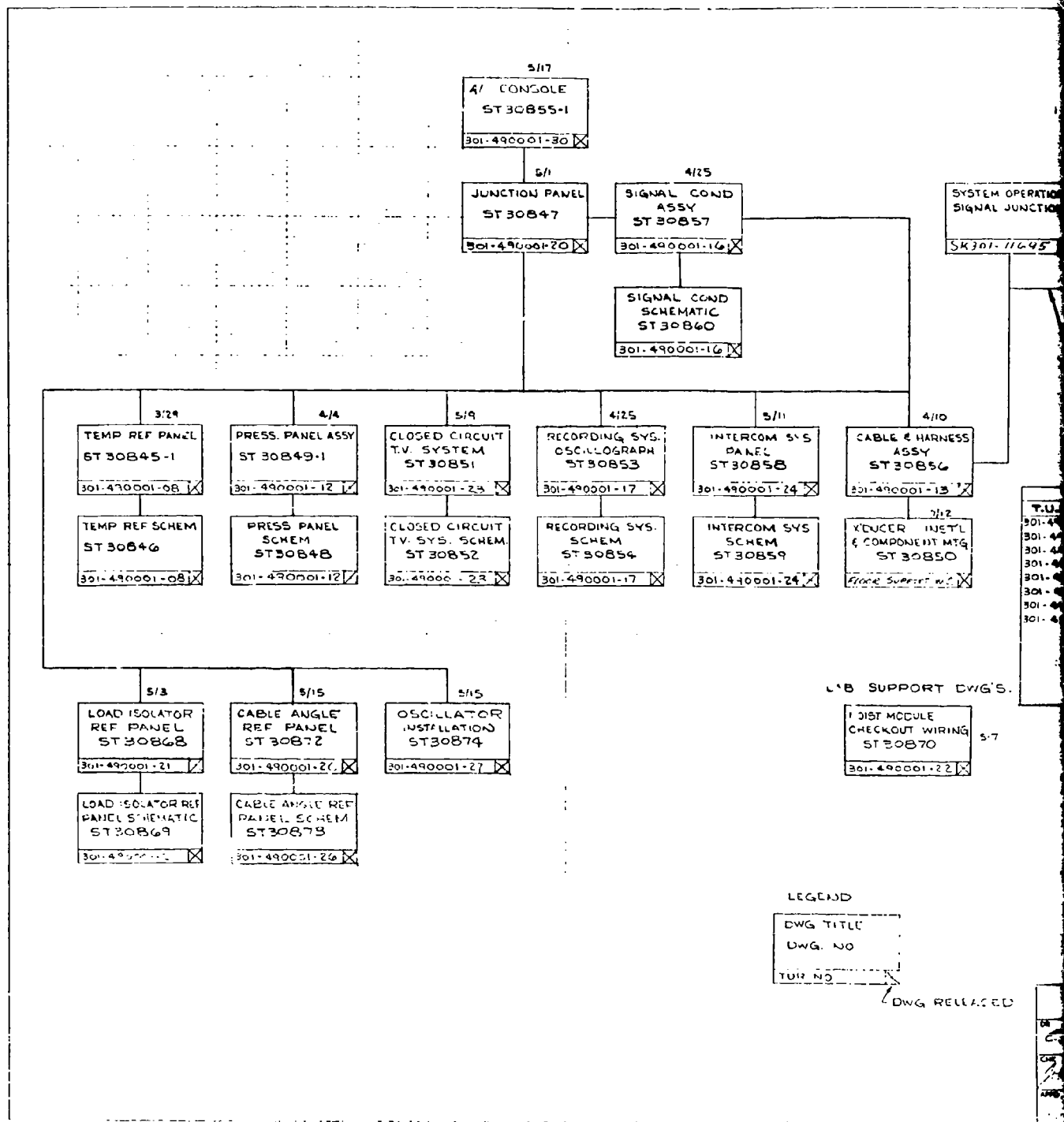
P12

P10

AFT SIG COND  
REEL  
A11

Wiring.

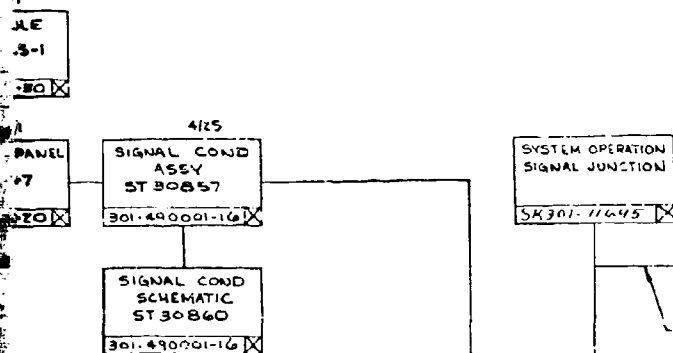
B



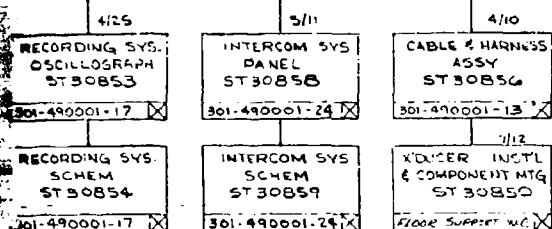
A  
Figure 69. Instrumentation Drawing Tree.

Preceding page blank

REVISIONS			
LTB	DESCRIPTION	DATE	APPROVAL
A	RELEASED THRU INSTE ST30850	7/26	
B	REVISED TO SP. SCHEM ST30852	7/27	



CABLES TO OPERATORS CONSOLE, TEST ARTICLE AND POWER SEE SK301-11679



ADDITIONAL T.U.R. RELEASE		
T.U.R. NO.	EFFECTIVITY	DATE
301-490001-11	REV A DWG ST30855 & ST30846	4/16/73
301-490001-18	REV A DWG ST30856 FAB - 5 ASSY	4/20/73
301-490001-19	ST30851 PURCHASE SYSTEM	4/16/73
301-490001-28	ST30846 REV B PURCHASE SWITCH	5/21/73
301-490001-29	ST30848 REV A	5/21/73
301-490001-31	TEST RIG ASSEMBLY INSTRUCTIONS	5/25/73
301-490001-34	FABRICATION OF ALL COMPLETE PARTS	5/25/73
301-490001-41	REVISE TO SYS SCHEMATIC ST30852 REV A	5/21/73

LAB SUPPORT DWGS.

HOIST MODULE  
CHECKOUT WIRING  
ST30870  
301-490001-22

LEGEND

DWG TITLE  
DWG NO  
TUR NO

DWG RELEASED

CONTRACT NO. 62-01-1-100-0000 (P. 40)  
DRAWN BY: CITY MAIL SIZE & DESCRIPTION  
BOEING TESTOL COMPANY  
PH. 602-241-1000  
PH. 602-241-1000

INSTRUMENTATION DRAWING TREE  
CARGO HANDLING SYSTEM TEST  
NO. 101-11645  
C 17772  
ST 30851

B

APPENDIX III  
PNEUMATIC POWER GENERATOR - STARTING AND OPERATING PROCEDURE

PRERUN CHECKOUT

1. Remove all covers from unit:
  - a. Unit covers
  - b. Exhaust cover
  - c. Bleed valve wooden plug
2. Turn on water for oil cooler.
3. Turn on hand shutoff valve in fuel line at 275-gal. tank.
4. Check all switches on control panel for "OFF" position.
5. Attach all electrical connectors:
  - a. 2 control lines at engine
  - b. 3 thermo couple leads at engine
  - c. 2 control lines at console
  - d. 3 thermo couple leads at console
  - e. 110-VAC line at console
  - f. 14-VDC connector from batteries to APU
  - g. 28-VDC connectors from batteries to APU

STARTING PROCEDURE - CONTROL CONSOLE

1. Turn power "ON".
2. Turn fuel valve "ON".
3. Turn fuel pump "ON" (requires 10 seconds to pressurize line).
4. Turn starter and ignition "ON".
5. When gas producer speed reaches 13-15%, increase throttle control with momentary jog. Observe engine starting limits.
6. When gas producer speed reaches 52-58%, turn "OFF" starter and ignition. Allow engine to warm up at idle for 1 minute.
7. Increase throttle control to "full speed" with momentary jog. Observe engine continuous run limits.

**Preceding page blank**

#### STOP PROCEDURE

1. Decrease throttle control to idle position with momentary jog. Run for 2 minutes at "idle" position.
2. Decrease throttle control to "stop" position.
3. When gas producer speed is less than 15%, turn "OFF" fuel pump and fuel valve.
4. Turn "OFF" power.

#### POST-RUN SHUTDOWN

1. Turn "OFF" water to oil cooler.
2. Turn "OFF" hand fuel valve at 275-gal. tank.
3. Disconnect electrical connectors:
  - a. 110-VAC at console
  - b. 14-VDC from batteries to APU
  - c. 28-VDC from batteries to APU
4. Install covers after unit has cooled:
  - a. Exhaust covers
  - b. Bleed valve wooden plug
  - c. Unit cover

TABLE XII. NORMAL PPG OPERATING LIMITS.		
Parameter	Run Condition	Range or Limit*
T <sub>T5</sub>	GI	700°-900°F
T <sub>T5</sub>	Max Continuous	1430°F
N1	Start	12%-15%
N1	GI	59%-65%
N1	Max Continuous	104%
N2	GI	74.8%-104.7%
N2	Max Continuous	103.8%
P Turb.GB	GI	50-130 psi
P Turb.GB	Max Continuous	115-130 psi
T Turb.GB	GI-Max Continuous	130-225°F
P Compressor GB	GI	90 psi
P Compressor GB	Max Continuous	110 psi
T Compressor GB	Max Continuous	225°F
*Values are for S.L. Standard operation. For information on transient limitations, consult the GMC Allison Div. 250-C20 Engine Operation and Maintenance Manual.		



# APPENDIX IV INSTRUMENTATION CALIBRATION PROCEDURE

The following represents the basic calibration steps carried out before and after daily hoist system operation.

## TEMPERATURES

<u>Operating Mode</u>	<u>Power Source</u>	<u>Osc A</u>	<u>Chan1. Selection</u>
Cal.	R.C.	.25 ips	1 thru 7; then zero
Normal	On	.25 ips	Zero

## PRESSURES

<u>Operating Mode</u>	<u>Input</u>	<u>Osc A</u>	<u>Cal. Pushbutton</u>
Cal.	Depress Cal.	.25 ips	1-5 depress each
Normal	--	--	---

## LOAD ISOLATORS

<u>Operating Mode</u>	<u>Input</u>	<u>Indication</u>	<u>Adjust</u>
Cal.	Depress sense	30.0	Adjust each pot

## CABLE ANGLE

<u>Operating Mode</u>	<u>Input</u>	<u>Read Voltage At</u>	<u>Adjust</u>
Normal	.25V/DEC Fwd/Pitch	CTB4 Pins 1&2	Adj. each pot for panel reading in accordance with CTB4 voltage reading
	Fwd/Roll	CTB4 Pins 4&5	
	Aft/Pitch	CTB4 Pins 7&8	
	Aft/Roll	CTB4 Pins 10&11	

#### CABLE PAYOUT SPEED

<u>Operating Mode</u>	<u>Input</u>	<u>Apply To</u>	<u>Adjust</u>
Normal	5.0VDC=120 FPM	CTB3 Pins 1&2 CTB3 Pins 4&5	Adj.A1 gain for 2.0" trace deflection Adj.A2 gain for 2.0" trace deflection

#### CABLE PAYOUT LENGTH

<u>Operating Mode</u>	<u>Input</u>	<u>Apply To</u>	<u>Adjust</u>
Normal	5.0VDC=100 Ft	CTB4 Pins 14&15 CTB4 Pins 18&19	Adj.A5 gain for 2.0" deflection Adj.A6 gain for 2.0" deflection

#### SPEED COMMAND CAL.

<u>Operating Mode</u>	<u>Input</u>	<u>Indication</u>	<u>Adjust</u>
Normal	6.0V=100%	2.0" trace deflec.	Press fwd then aft control grip thumb switches.

#### MOD. VALVE CURRENT CAL.

<u>Operating Mode</u>	<u>Input</u>	<u>Apply At</u>	<u>Adjust</u>
Normal	1.5VDC-100%	CTB3 Pins 24&25 CTB3 Pins 9&10	A3 gain for deflection A7 gain for deflection